











**GENERAL FLOOR PLAN NOTES**

- WHERE PARTITIONS OF VARIOUS THICKNESSES MEET, MAINTAIN A FLUSH SURFACE ON THE SIDE WHERE FACES ARE STRAIGHT AND CONTIGUOUS, UNLESS NOTED OTHERWISE.
- ALL DOOR OPENINGS SHALL BE 4" FROM A PERPENDICULAR PARTITION TO THE OUTSIDE FACE OF FRAME UNLESS NOTED OTHERWISE.
- SPRINKLER CONTRACTOR SHALL COORDINATE SPRINKLER LOCATIONS WITH WALL HUNG EQUIPMENT TO PROVIDE PROPER COVERAGE.
- TYPICAL GYPSUM BOARD PARTITION TYPE IS TYPE GS. ALL NEW GYPSUM BOARD PARTITIONS SHALL BE TYPE GS UNLESS NOTED OTHERWISE. PARTITIONS NOT DESIGNATED WITH A PARTITION TYPE SYMBOL SHALL BE TYPE GS.

**FLOOR PLAN LEGEND**

- NEW DOOR & FRAME
- STOREFRONT DOOR & FRAME
- NEW WALL
- EXISTING WALL
- INTERIOR ELEVATION TAG
- EXTERIOR ELEVATION TAG
- CALLOUT TAG
- SECTION: BUILDING SECTION
- SECTION: WALL SECTION
- SECTION: DETAIL SECTION
- COLUMN GRID LINE: NEW
- COLUMN GRID LINE: EXISTING
- EQUIPMENT TAG
- FURNITURE TAG
- DOOR & BORROWED LIGHT TAG
- DOOR & BORROWED LIGHT TAG (DETENTION)
- EXTERIOR WINDOW TAG
- PARTITION TYPE TAG
- CURTAIN WALL / STOREFRONT TAG
- ROOM NAME & NUMBER TAG
- ALIGN FACE OF MATERIALS SYMBOL
- CARD READER
- DOOR OPERATOR / TOUCHLESS ACTUATOR
- FLOOR PLAN / FINISH PLAN KEYNOTE
- DURESS BUTTON

**FLOOR PLAN KEYNOTES**

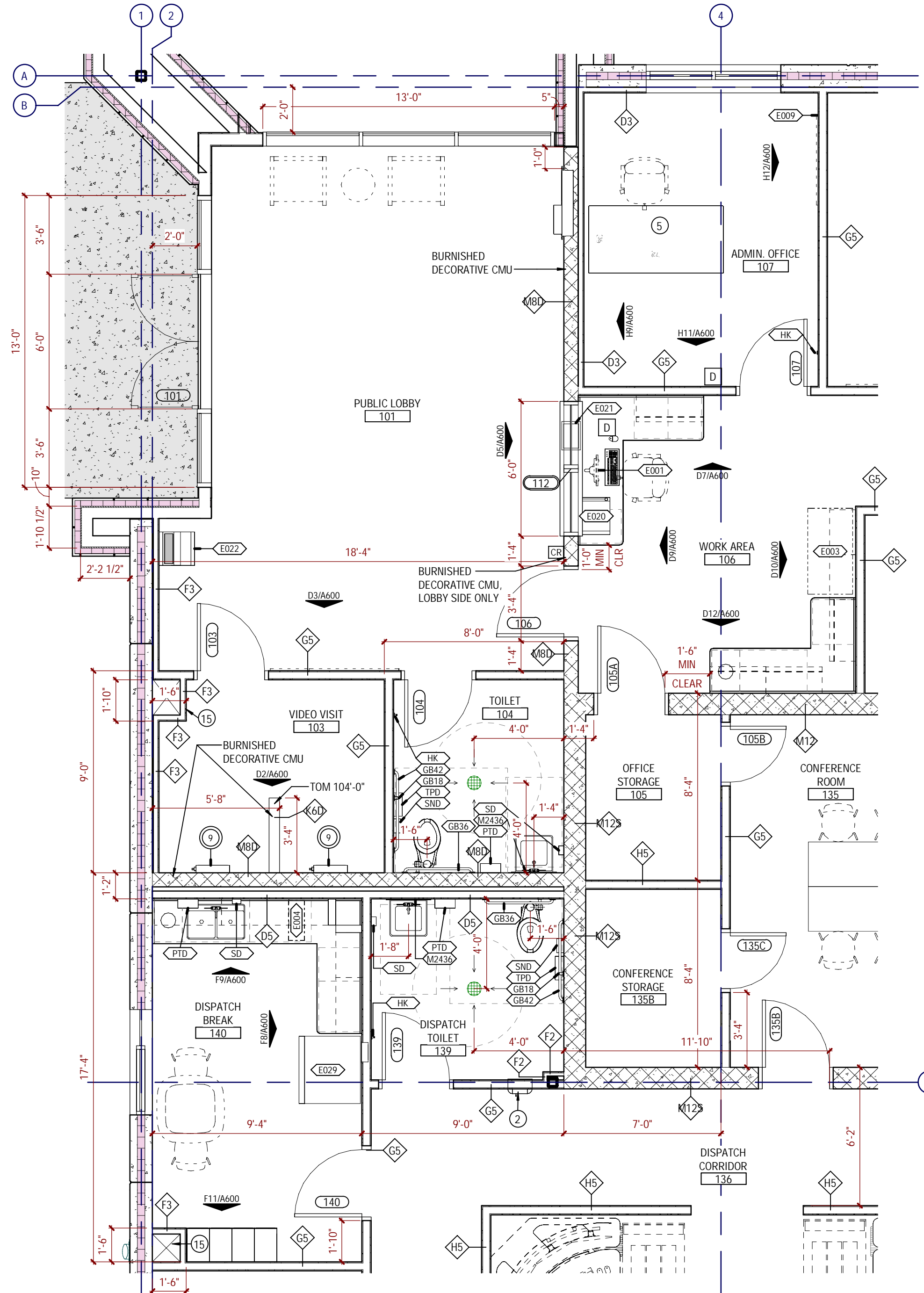
- 4" WIDE PAINTED LINE ON THE FLOOR
- SEMI-RECESSED FIRE EXTINGUISHER CABINET
- SURFACE MOUNTED FIRE EXTINGUISHER CABINET
- ACCESS LADDER, RE: STRUCTURAL
- OFFICE FURNITURE (OFO)
- PROVIDE 3/4" FIRE-TREATED PLYWOOD PANELS TO FACE OF GYP BD AS REQUIRED FOR FIRE ALARM CONTROL PANEL. DO NOT PAINT ENTIRE PLYWOOD PANEL. LEAVE AT LEAST ONE FIRE-TREATED LABEL VISIBLE ON EACH PANEL.
- MUGSHOT PHOTO BACKDROP TO BE ALIGNED WITH MUGSHOT CAMERA AT BOOKING DESK
- MONITOR MOUNTED ON WALL AT 72" A.F.F. TO CENTERLINE
- 4" DIA. CONCRETE BOLLARD TO CAUTION OVERHEAD CLEARANCE, RE: DETAIL ABAS31 PANT PER FINISH PLAN
- 1" EXPANSION JOINT AT CMU PARTITION, RE: STRUCTURAL
- EXTERIOR BOLLARDS, RE: CIVIL FOR DETAILS
- CARD READER PEDESTAL, RE: SECURITY/ELECTRONICS
- RECESS MOUNT KNOXBOX, MOUNTED @ 48" TO CENTER OF BOX
- FUTURE MANUAL ROLLING STORAGE BY OTHERS, FLOOR TRACKS TO BE MOUNTED ON THE CONCRETE SLAB.
- PIPE CHASE FOR ROOF STORM DRAINS
- PICK-PROOF SEALANT AT EACH SIDE OF COLUMN, TYPICAL
- FLUSHING FLOOR DRAIN VALVE COVER, RE: MEP

**DETENTION EQUIPMENT**

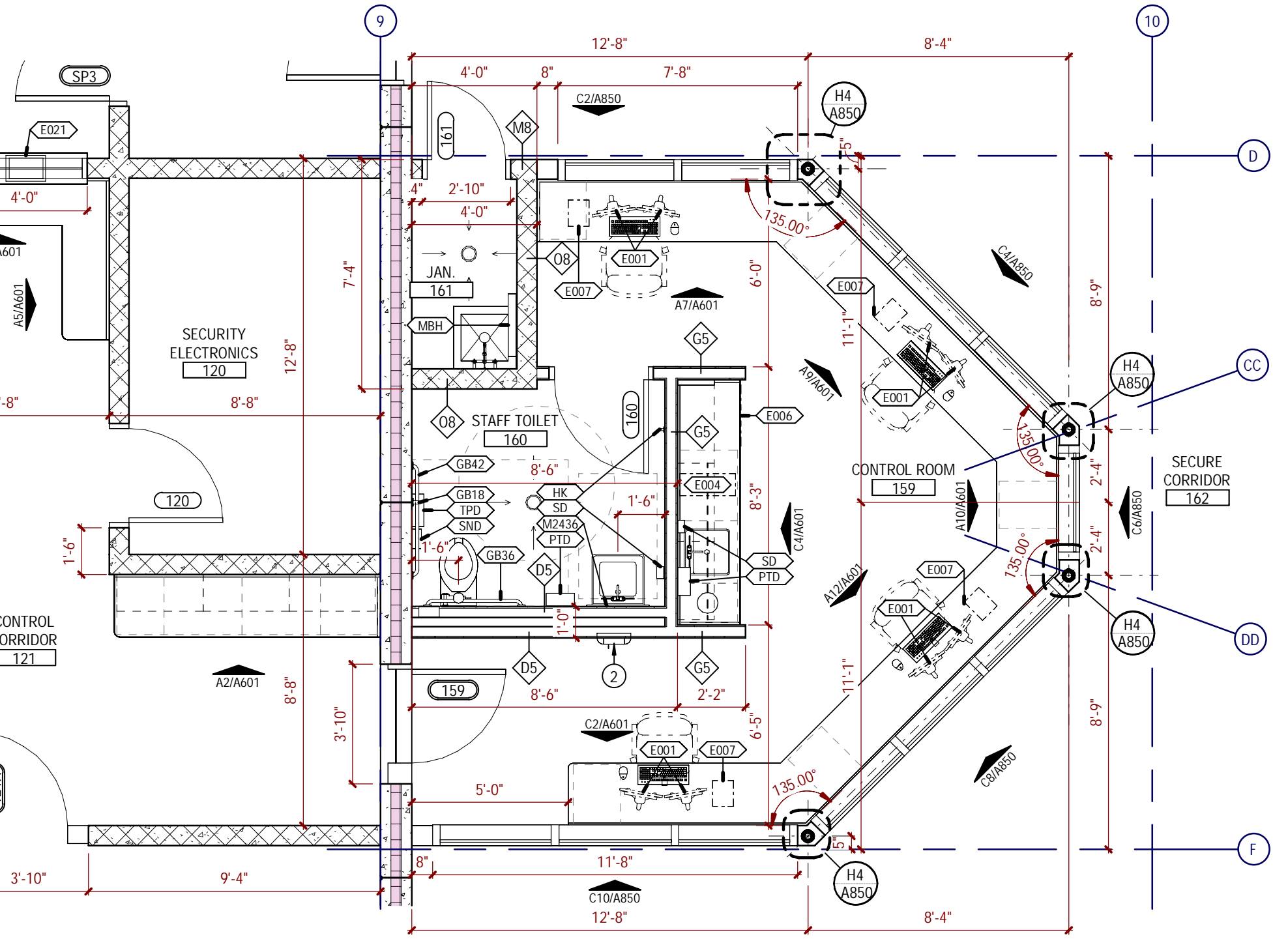
CODE	ITEM DESCRIPTION	SPEC SECTION	PROVIDED BY
1	DETENTION MIRROR	10 2813.63	CFCI
2	SHELF WITH HOOK	10 2813.63	CFCI
4	WALL MOUNTED DOUBLE DETENTION DESK AND SEATS	12 5500	CFCI
5	FLOOR MOUNTED DOUBLE BUNK	12 5500	CFCI
6	FLOOR MOUNTED FIXED STOOL	12 5500	CFCI
7	EVIDENCE LOCKER (PASS-THRU) - 6 COMPARTMENTS	11 1916	CFCI
8	PISTOL LOCKER (RECESSED) - 6 COMPARTMENTS	<varies>	<varies>
9	VIDEO VISITATION MONITOR	...	OKFI
10	INMATE PHONE	...	OKFI
11	PISTOL LOCKER (SURFACE MOUNTED) - 6 COMPARTMENTS	11 1916	CFCI
12	DETENTION GRAB BAR (42")	10 2813.63	CFCI
13	4-MAN TABLE	12 5500	CFCI
15	8-MAN TABLE	12 5500	CFCI
20	RECESSED CLIFF RING	10 2813.63	CFCI
21	SECURITY INTERCOM	RE: MEP	CFCI

**EQUIPMENT SCHEDULE**

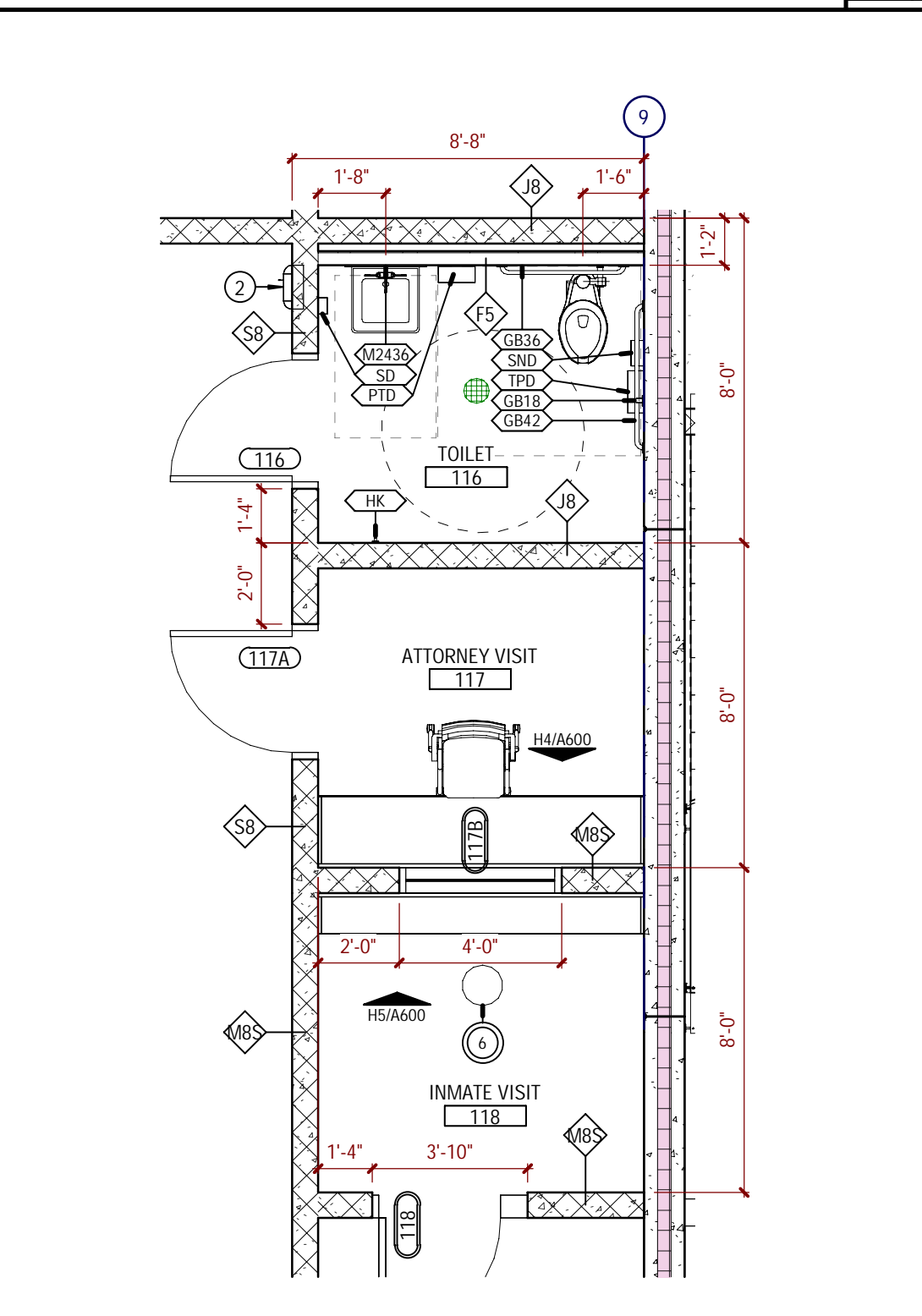
CODE	ITEM DESCRIPTION	RESPONSIBILITY	COMMENTS
E001	Computer Monitor (Desktop)	X	
E002	Exam Table	X	
E003	Printer/Copy/Fax (Floor)	X	
E004	Coffee Maker	X	
E005	Microwave	X	
E006	DC Refrigerator	X	
E007	PHONE (DESKTOP)	X	
E008	Breathalyzer (Counter-top)	X	
E009	Tack Board	X	
E010	TRASH BIN (24 GAL)	X	
E011	Commercial Washer	X	SEE SECTION 11 1100
E012	Commercial Dryer (Gas)	X	SEE SECTION 11 1100
E013	AUTOMATED CONTROLLED SUBSTANCE DISPENSER	X	
E014	PAY STATION CLOCK	X	
E015	TV MONITOR (52")	X	CONTRACTOR TO PROVIDE BLOCKING
E016	32" ENCARTELE MESSAGE MONITOR	X	CONTRACTOR TO PROVIDE BLOCKING
E017	Refrigerator (full size w/ ice maker)	X	
E018	Marker Board	X	
E019	Folding Table	X	
E020	Package Pass	X	
E021	Drop in Dish Tray	X	SEE SECTION 10 2813.63 - 7 5/8" WALL
E022	COMMISSARY KIOSK	X	
E023	RED BAG TRASH (24 GAL)	X	
E024	Mugshot Camera	X	
E025	Mugshot Photo Backdrop	X	
E026	Laundry Cart	X	
E027	32" ENCARTELE MESSAGE MONITOR	X	CONTRACTOR TO PROVIDE BLOCKING
E028	BROCHURE RACK	X	
E029	Refrigerator/Freezer (full-size, w/ ice maker)	X	CONTRACTOR TO PROVIDE WATER LINE
E030	First Aid Box (wall-mounted)	X	
E031	Body Scanner	X	
E032	Wall-mounted Coat Rack (24" W)	X	



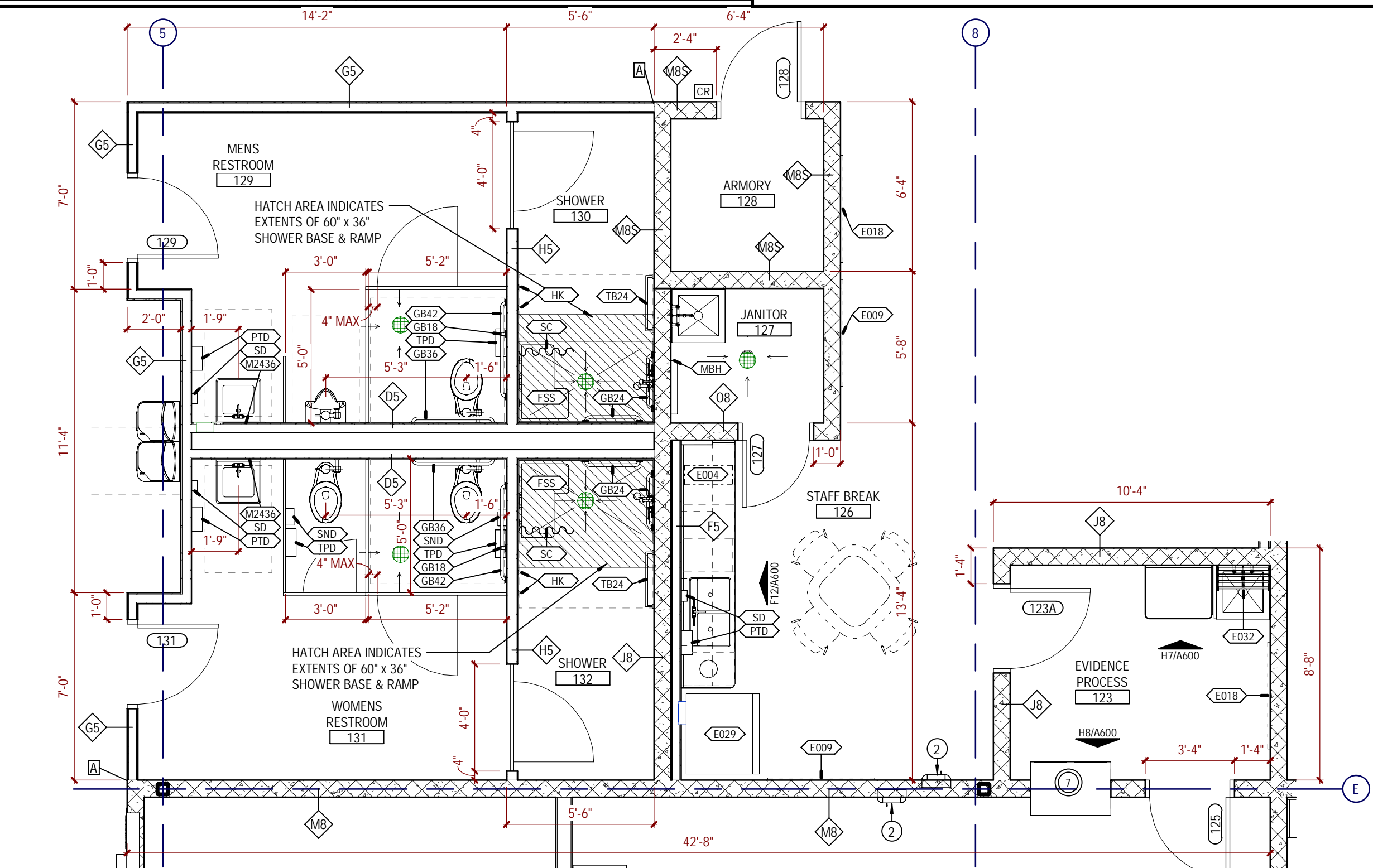
**C1101 - DISPATCH BREAK, TOILETS, VIDEO VISIT**  
1/4" = 1'-0" (ASA10)



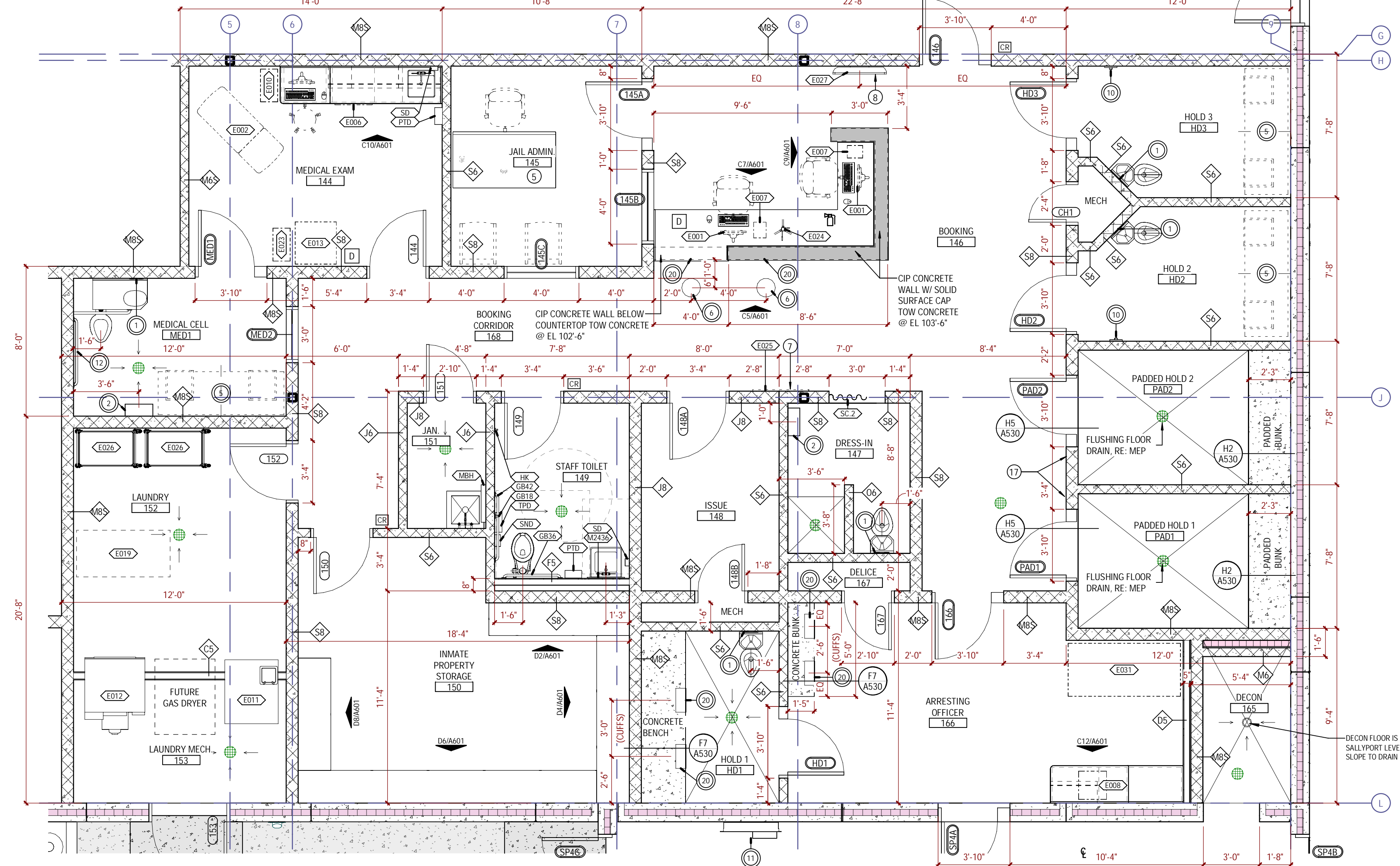
**A10 CONTROL ROOM**  
1/4" = 1'-0" (ASA10)



**E801 - ATTORNEY VISIT AND TOILET**  
1/4" = 1'-0" (ASA10)



**E601 - STAFF BREAK, EVIDENCE PROCESS**  
1/4" = 1'-0" (ASA10)



**A801 - BOOKING, MEDICAL, HOLDING**  
1/4" = 1'-0" (ASA10)



























**PRECAST PANEL KEYNOTES**

#	DESCRIPTION
E1A	RECESSED BOX & CONDUIT FOR RECEPTAL
E1B	RECESSED BOX & CONDUIT FOR RECEPTAL (OTHER SIDE)
E2A	RECESSED BOX & CONDUIT FOR SWITCH
E2B	RECESSED BOX & CONDUIT FOR SWITCH (OTHERSIDE)
E3A	RECESSED BOX & CONDUIT FOR LIGHT FIXTURE
E4A	RECESSED BOX & CONDUIT FOR THERMOSTAT
E4B	RECESSED BOX & CONDUIT FOR THERMOSTAT
E5A	TECH CONDUITS
E6B	RECESSED BOX & CONDUIT FOR DISCONNECT SWITCH
FP1A	RECESSED BOX & CONDUIT FOR PULL STATION
FP1B	RECESSED BOX & CONDUIT FOR PULL STATION (OTHER SIDE)
FP2B	RECESSED BOX & CONDUIT FOR EXT SIGN (OTHER SIDE)
FP3A	RECESSED BOX & CONDUIT FOR HORNSTROBE
FP3B	RECESSED BOX & CONDUIT FOR HORNSTROBE (OTHER SIDE)
FP4A	3" OPENING FOR SIEMENS FDC
PP5A	RECESSED BOX & CONDUIT FOR ELECTRIC BELL
MTA	OPENING FOR DUCT
MZA	LOUVER OPENING
M3A	OPENING FOR NATURAL GAS LINE
M4A	OPENING FOR SMOKE DAMPER
PIA	RECESSED BOX FOR HYDRANT
PA	3" OPENING FOR PLUMBING
PSA	6" OPENING FOR PLUMBING
P4A	6" OPENING FOR DOWNSPOUT
PSA	2" OPENING FOR PLUMBING
PA	4" OPENING FOR STORM WATER
SE1A	RECESSED BOX & CONDUIT FOR SWITCH
SE2A	RECESSED BOX & CONDUIT FOR CAMERA
SE2B	RECESSED BOX & CONDUIT FOR CAMERA (OTHER SIDE)
SE3A	RECESSED BOX & CONDUIT FOR CARD READER
SE4A	RECESSED BOX & CONDUIT FOR INTERCOM
SE5A	RECESSED BOX FOR PAGING SPEAKER
TN1A	RECESSED BOX & CONDUIT FOR DATA RECEPTAL
TN1B	RECESSED BOX & CONDUIT FOR DATA RECEPTAL (OTHER SIDE)

**PRECAST KEYNOTE LEGEND**

**X # Y X = DISCIPLINE  
# = ITEM  
Y = FACE OF PRECAST**

**PRECAST FINISH LEGEND:** SEE SPEC 03 45 00 FOR ELASTOMERIC FORM LINERS & ARCHITECTURAL FINISHES

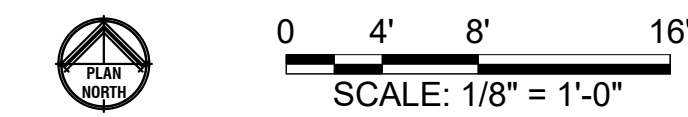
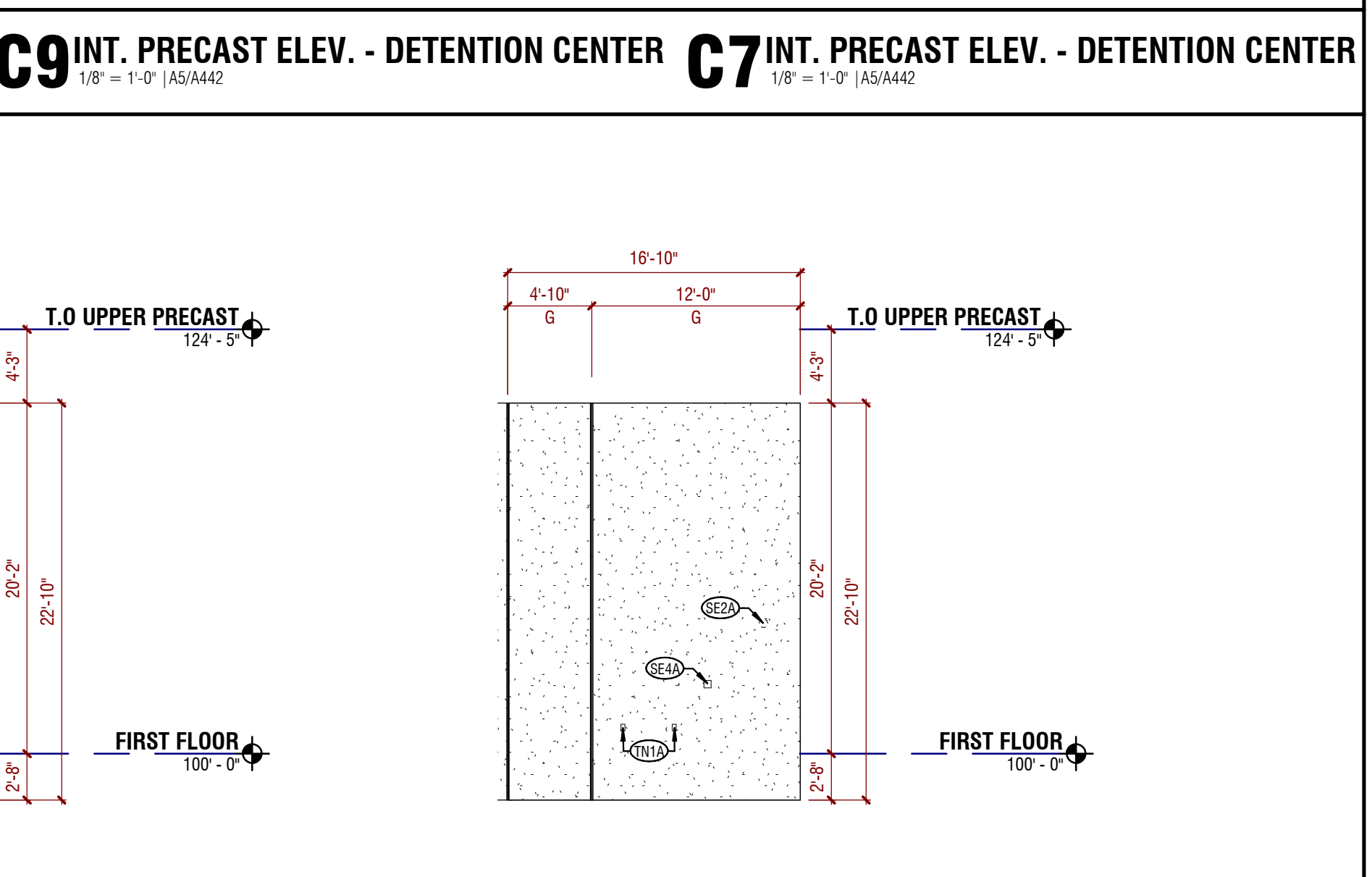
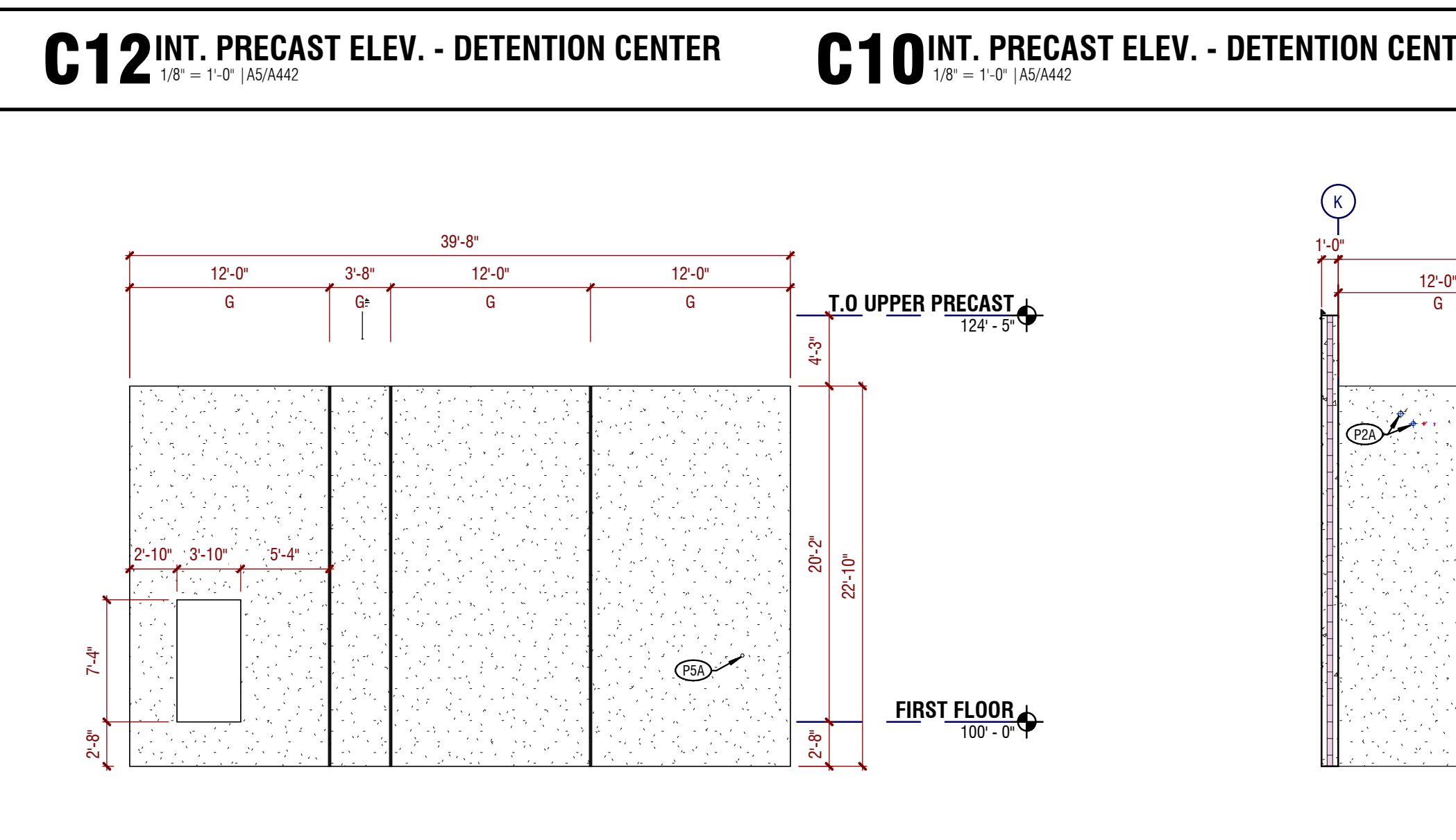
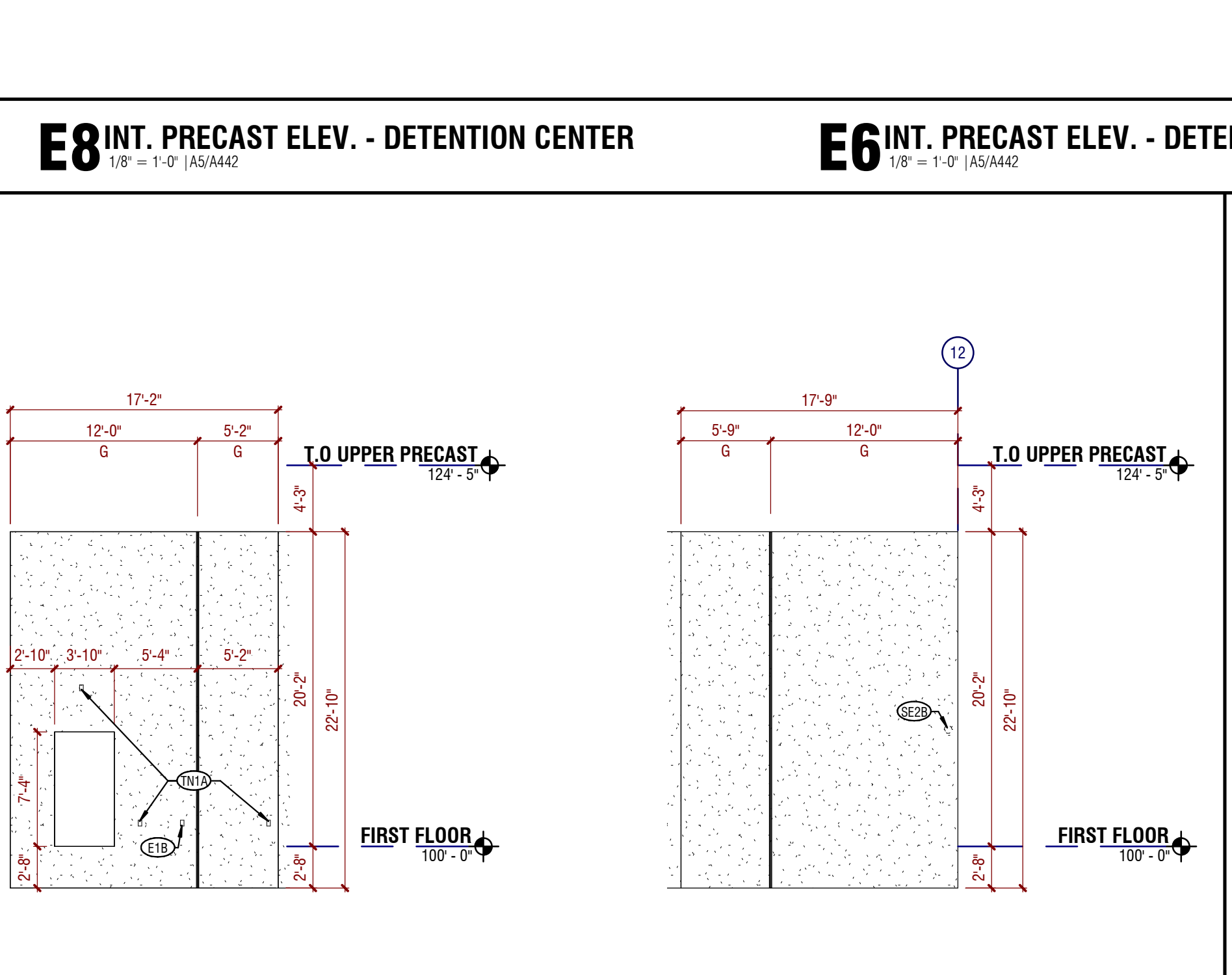
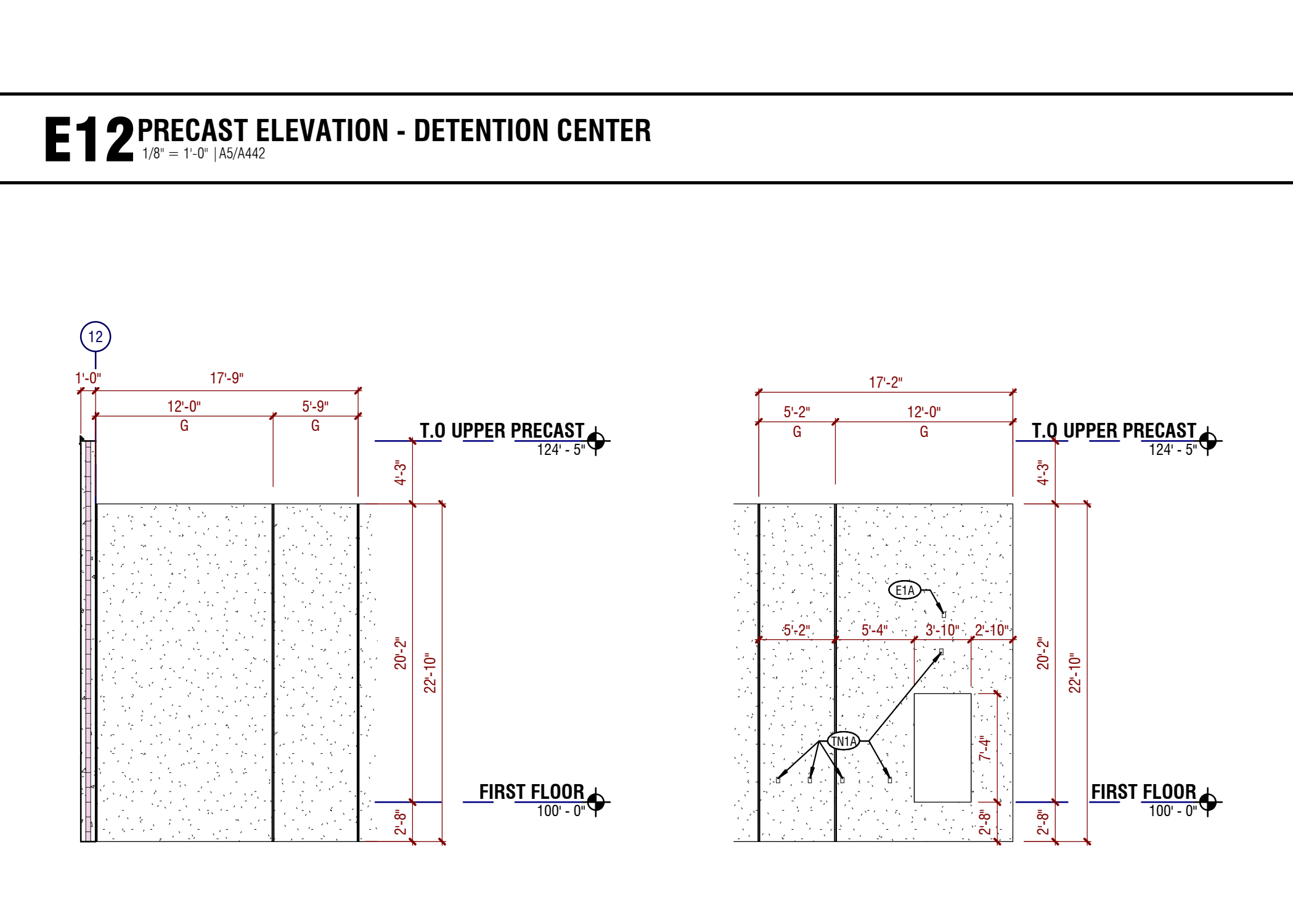
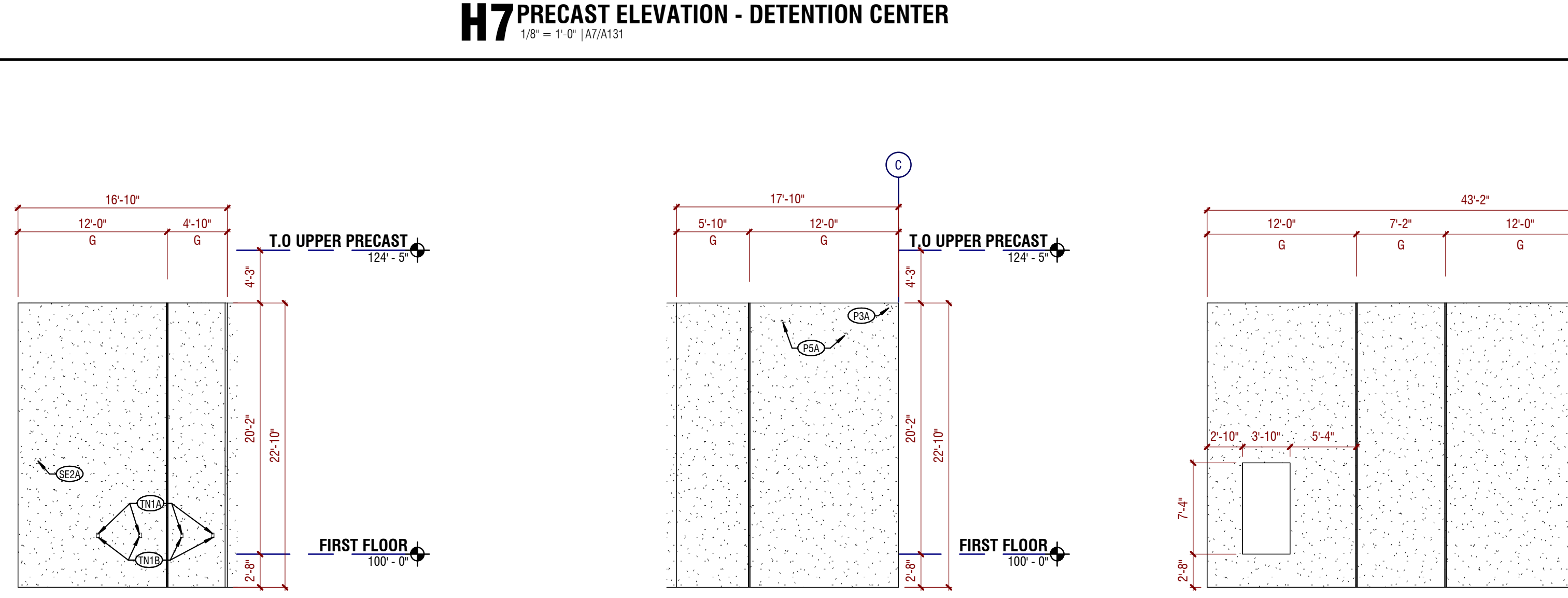
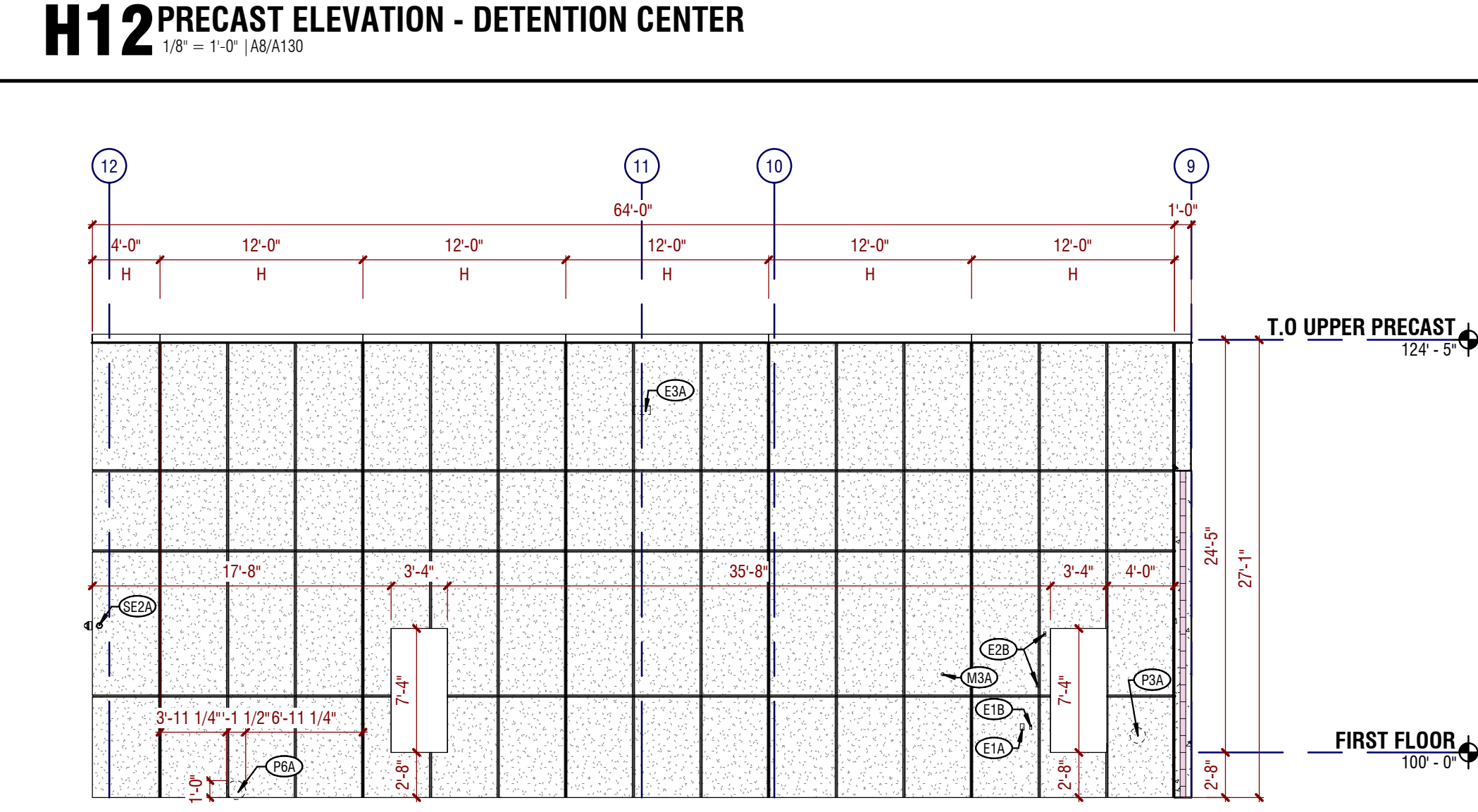
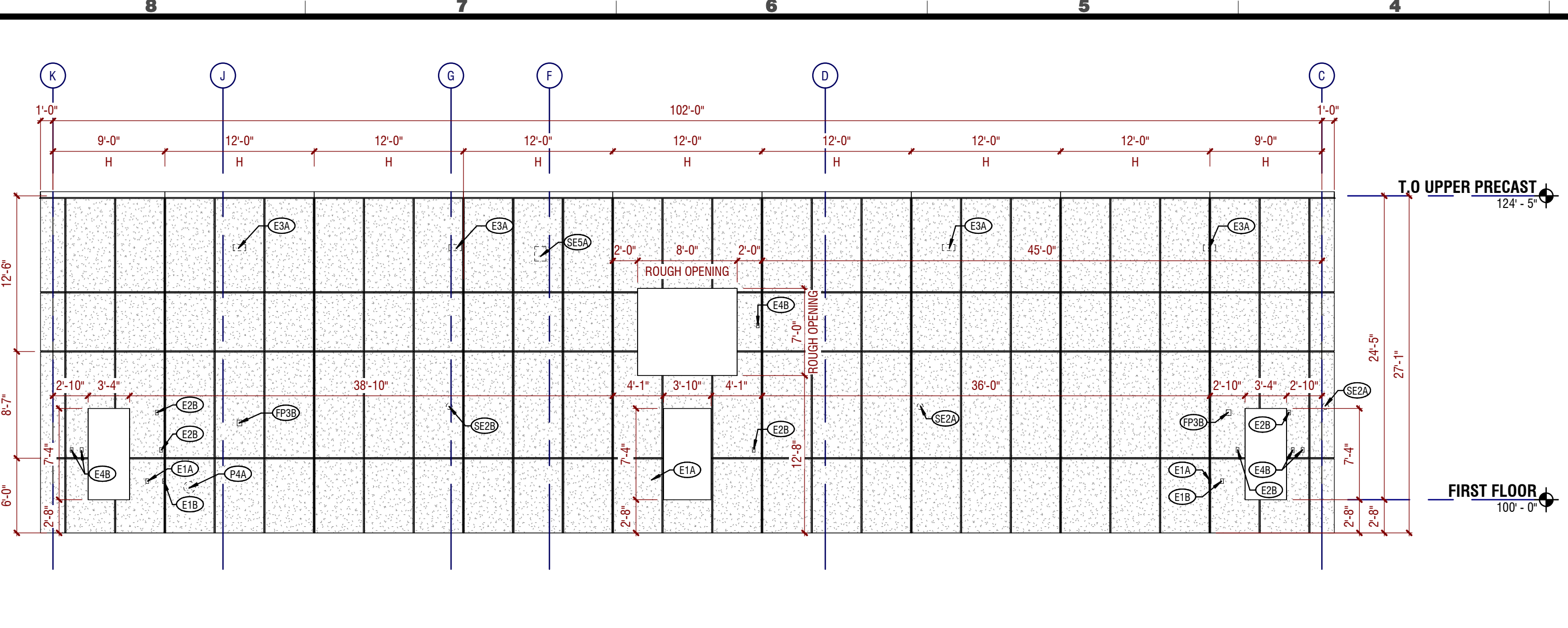
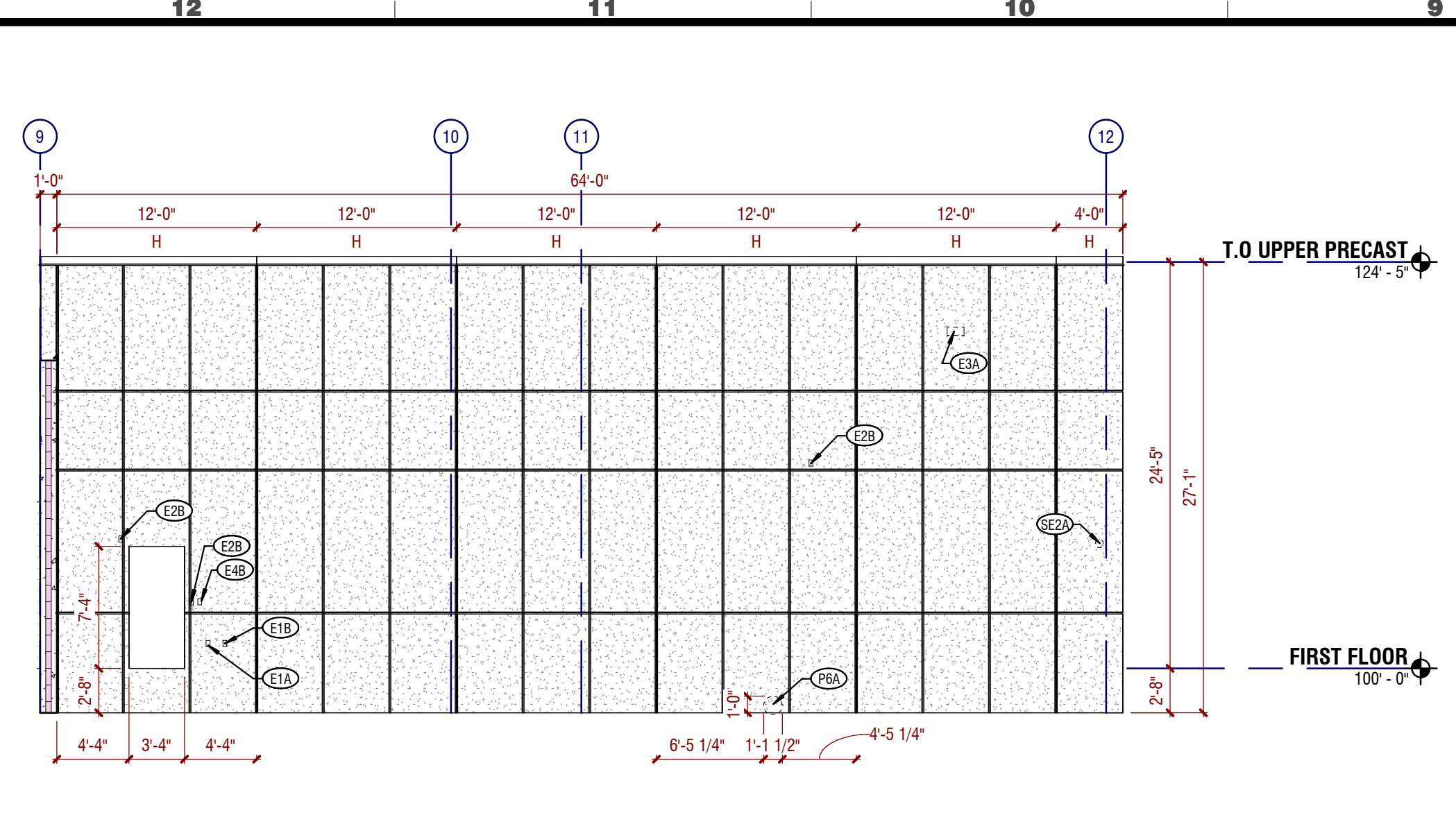
[Pattern]	F-1 LITE SANDBLAST FINISH
[Pattern]	F-2 GREY STEEL FORM FINISH
[Pattern]	F-3 STEEL TROWEL FINISH, SEE NOTE 5.
[Pattern]	F-4 FORMLINER FINISH

**PRECAST ELEVATION LEGEND:**



**PRECAST GENERAL NOTES:**

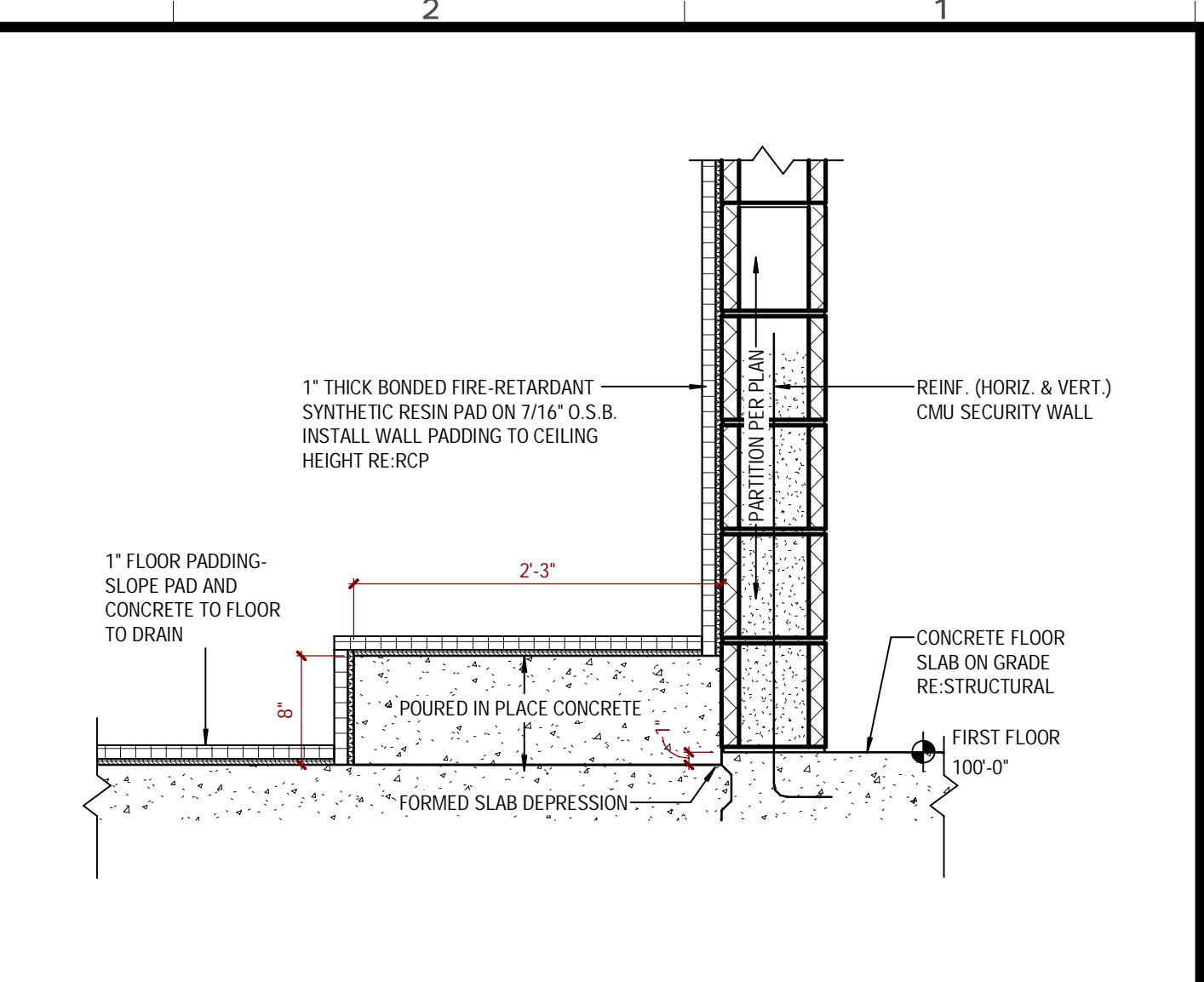
1. BOTTOM OF PRECAST WALL PANELS TO BE MAXIMUM 1-1/2" ABOVE TOP OF FOOTING FOR SHIM SPACE. SEE STRUCTURAL SHEETS.
2. SEE PRECAST DETAILS ON SHEET A443 FOR PANEL JOINT PROFILE, INSULATION, AND FIRE STOP INFORMATION.
3. PRECAST SILLS AT ALL WINDOW AND LOUVER OPENINGS TO SLOPE. SEE WINDOW SHEETS A800-A801.
4. PRECAST WALL PANEL TO BE PAINTED AS NOTED IN FINISH SCHEDULE ON SHEET A800.
5. COORDINATE ALL RECEPT OPENINGS WITH MEPP SHEETS.
6. THE INFORMATION SHOWN ON THE PRECAST ELEVATIONS REPRESENTS THE INTENT OF THE ARCHITECT AND ENGINEER TO DOCUMENT PENETRATIONS, OPENINGS, JOINTS, AND RECESSED DEVICES WITHIN THE PRECAST CONCRETE PANELS & AMONGST ALL OF THE TRADES. THE INFORMATION SHOWN SHALL NOT BE CONSIDERED FINAL OR ALL INCLUSIVE. THE INFORMATION SHOWN DOES NOT RELIEVE THE CONTRACTOR OF THEIR RESPONSIBILITY TO THOROUGHLY CHECK AND CROSS COORDINATE ALL WORK DURING SHOP DRAWINGS. INCLUDING ANYTHING NOT SHOWN ON THESE PRECAST DRAWINGS. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO MAKE SURE ANY TRADE ADJUSTMENTS MADE DURING SHOP DRAWING REVIEW ARE ALSO COORDINATED WITH THE PRECAST CONCRETE PANELS.
7. ALL PRECAST WALLS THAT RECEIVE A RUBBER WALL BASE SHALL HAVE ADDITIONAL SEALANT APPLIED WITHIN THE INTERIOR CHAMBER JOINT TO BE FLUSH WITH FACE OF WALL TO THE RUBBER BASE HAS A CONTINUOUS WALL SURFACE FOR ATTACHMENT.



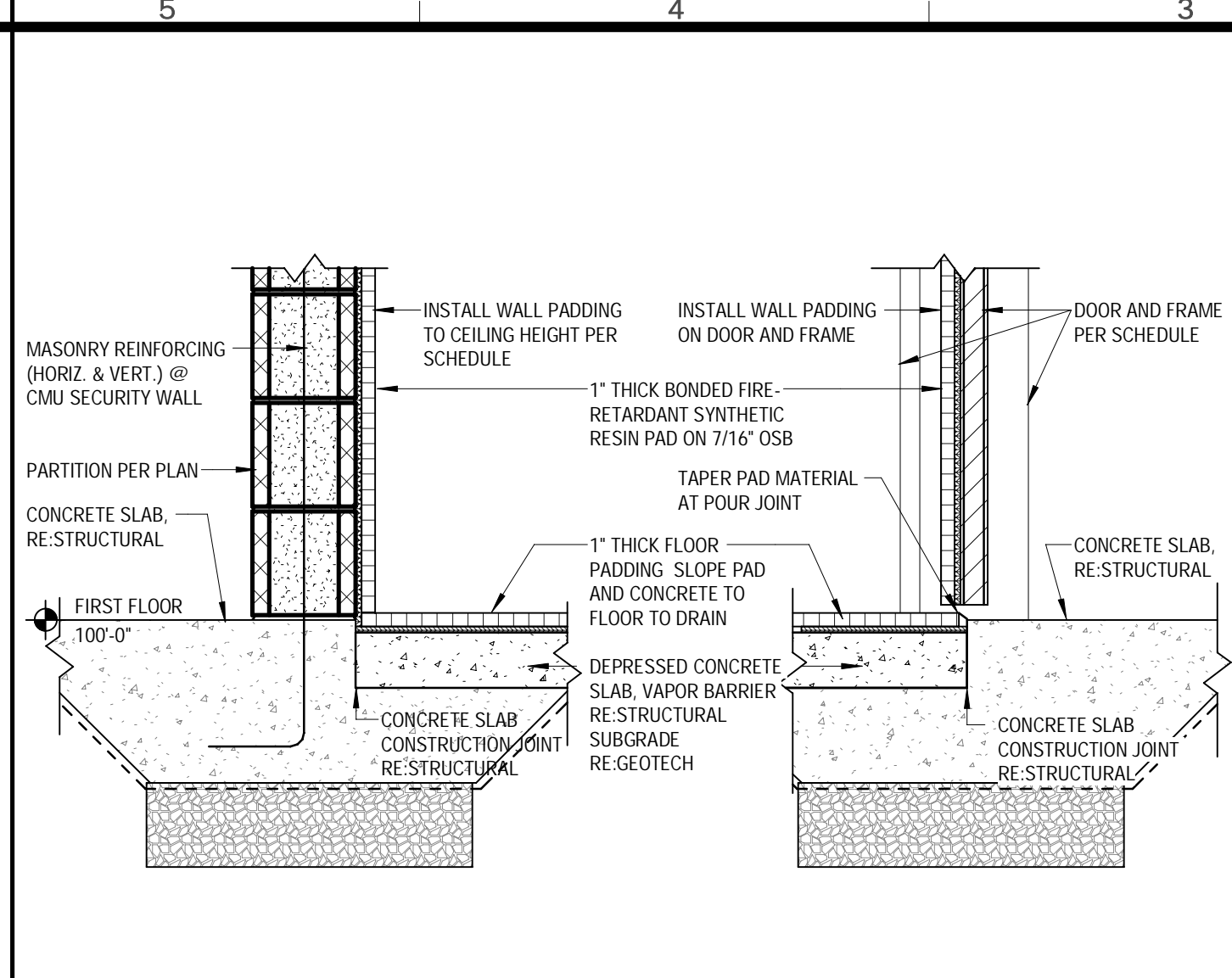




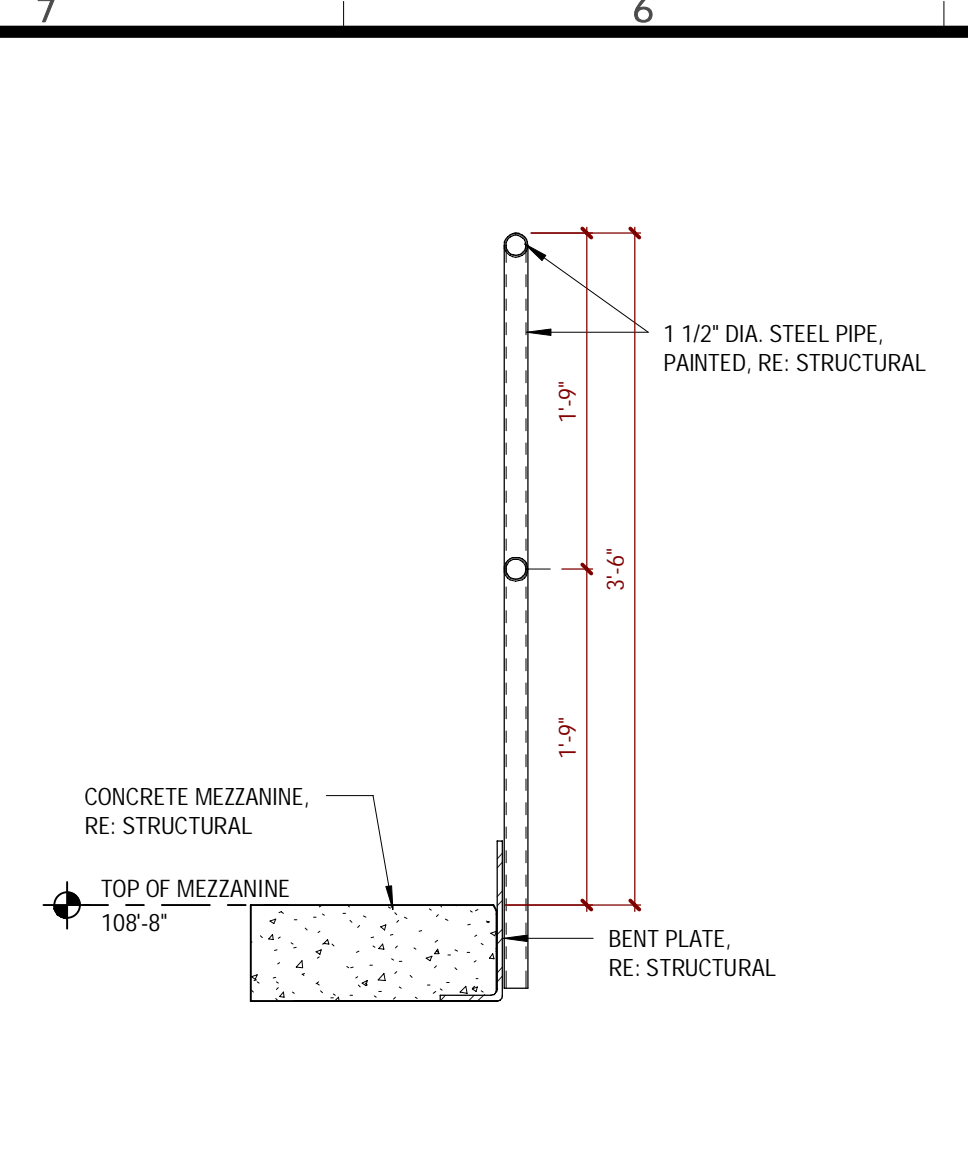
Issue	Date



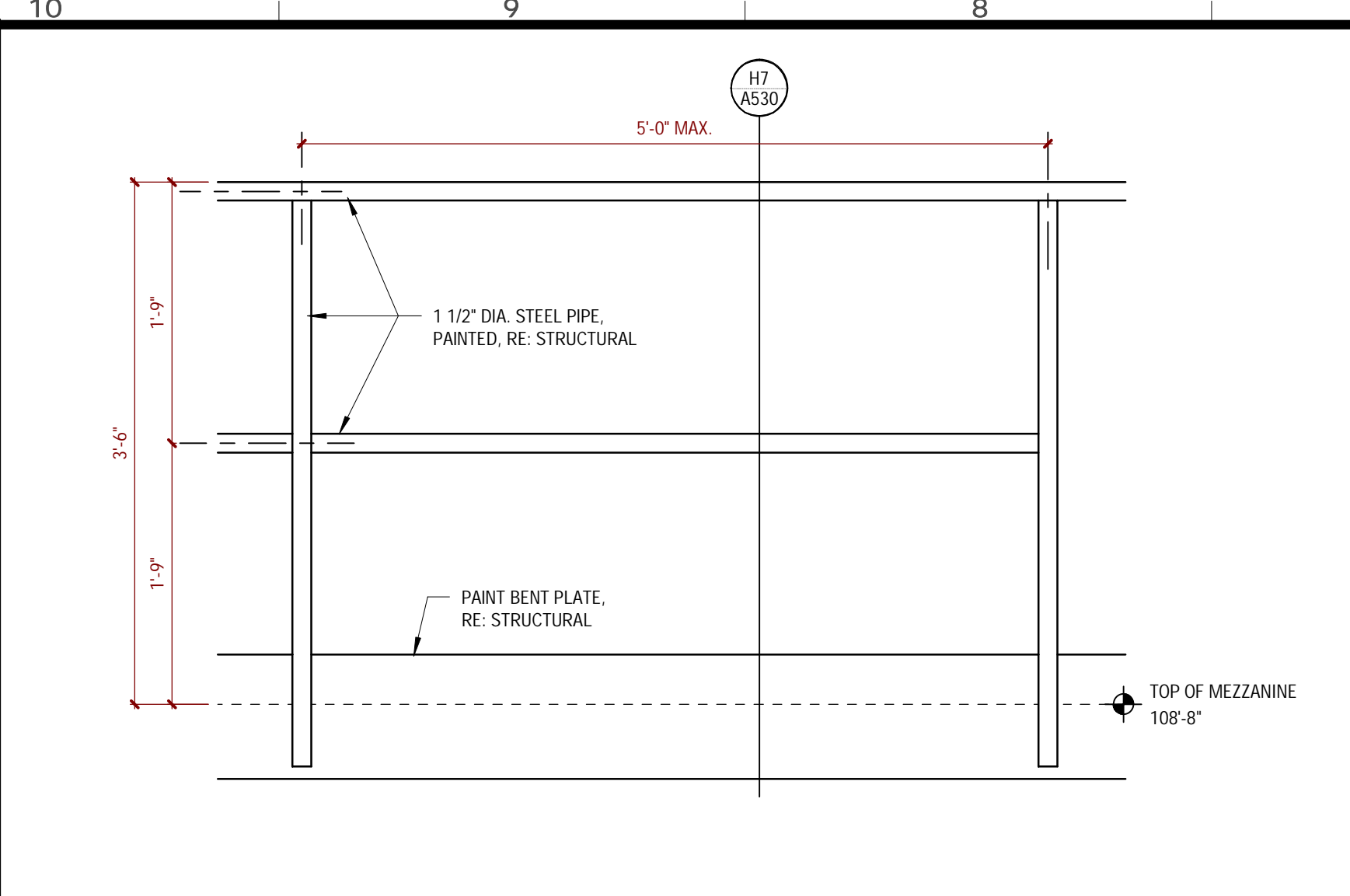
**H2 PADDED CELL BUNK DETAIL**  
1" = 1'-0" | A8/A130



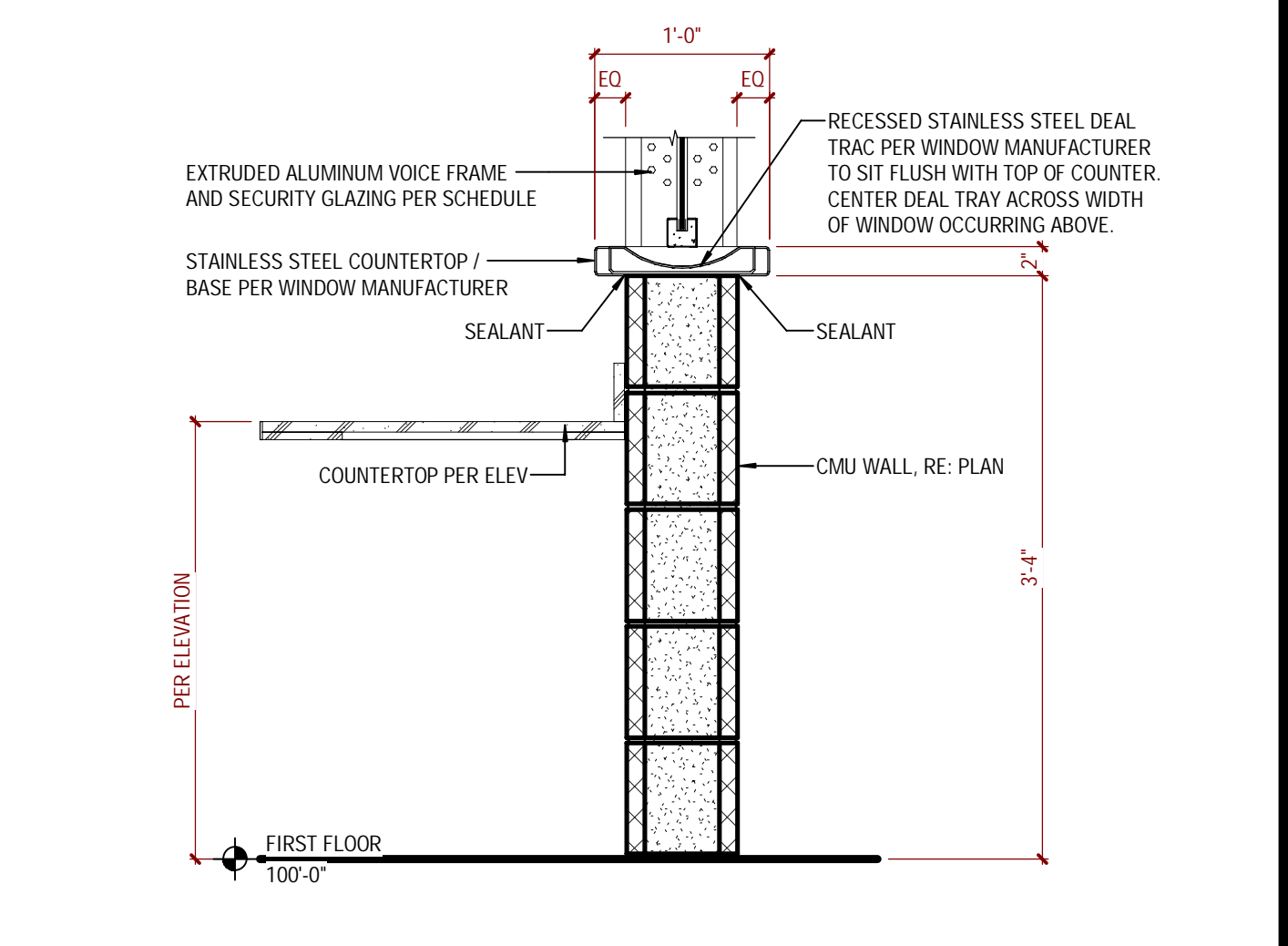
**H5 PADDED CELL WALL-FLOOR-DOOR**  
1" = 1'-0" | A8/A130



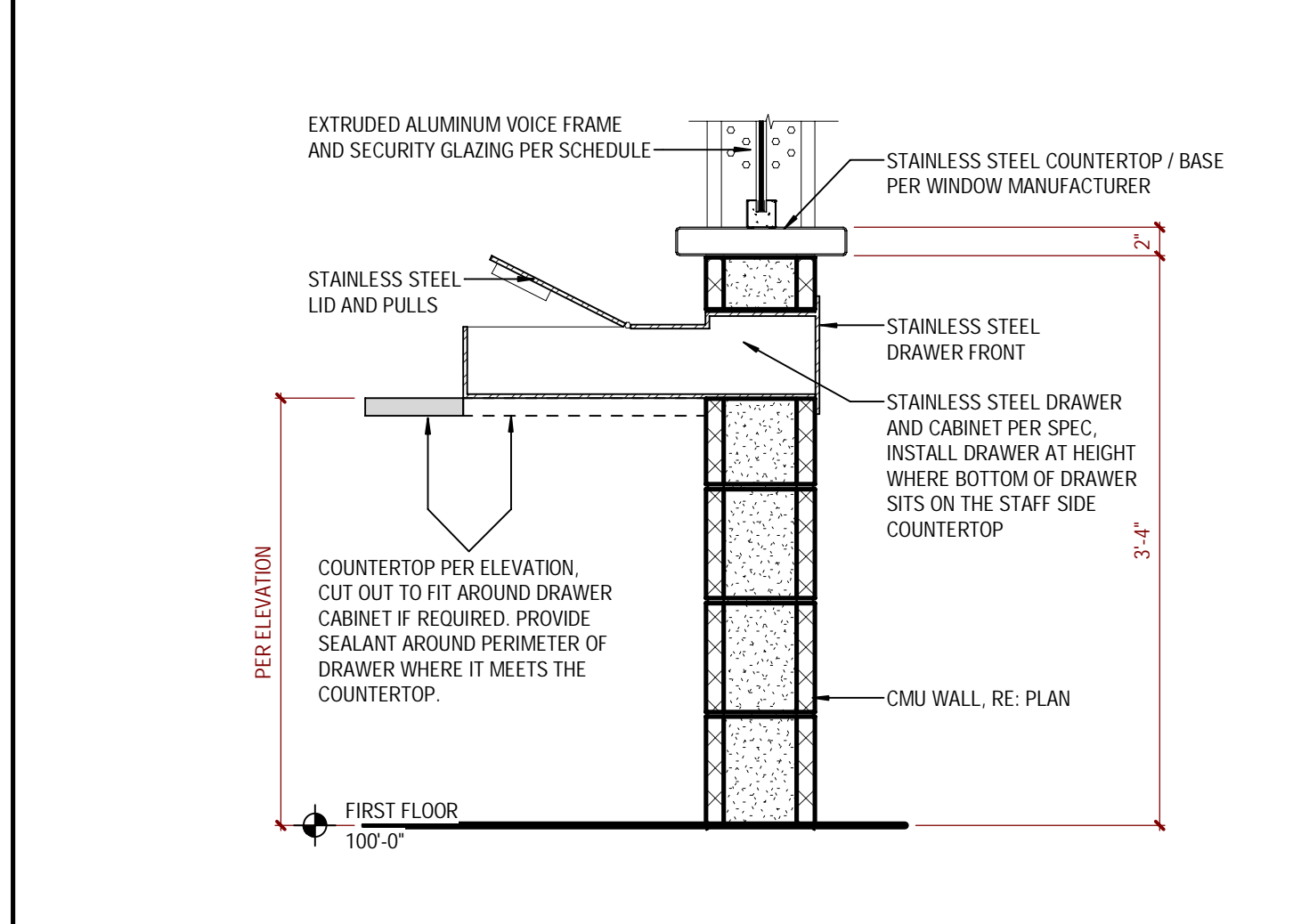
**H7 MEZZANINE GUARDRAIL SECTION**  
1" = 1'-0" | H10/A530



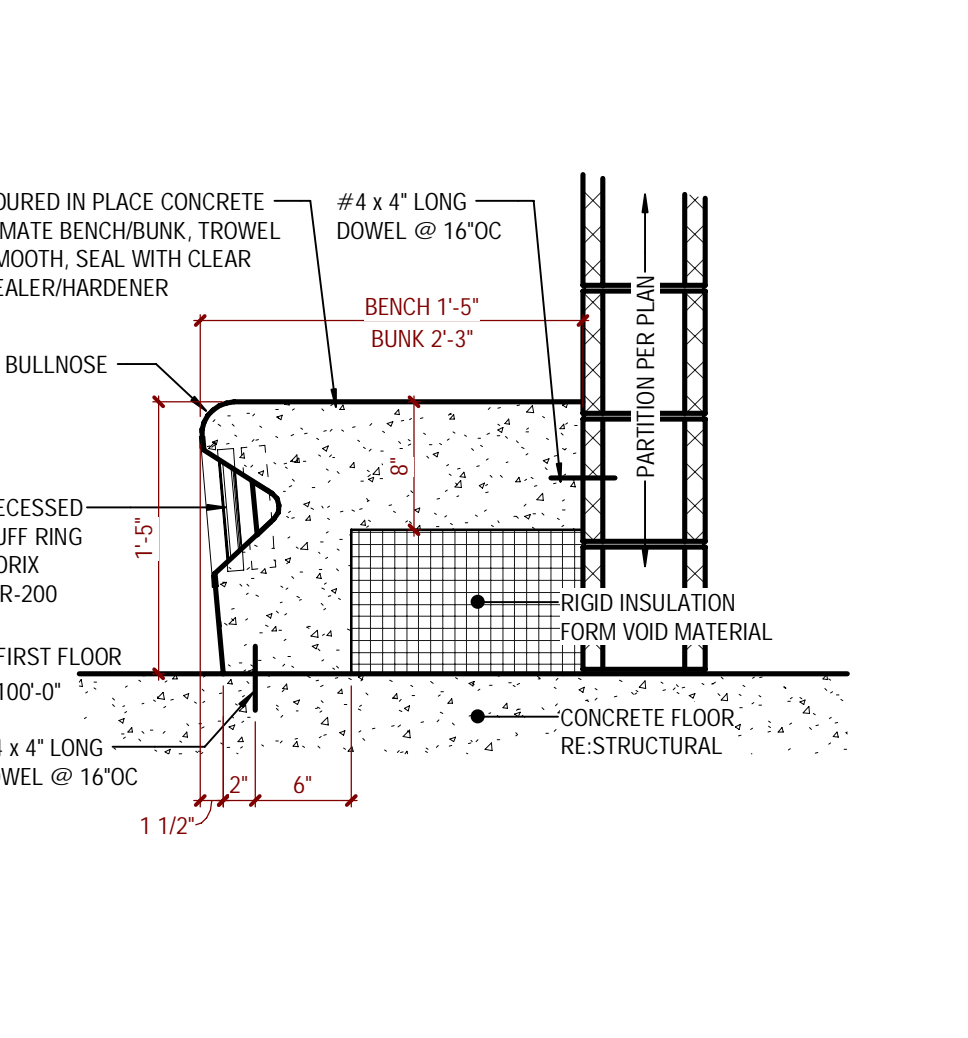
**H10 MEZZANINE GUARDRAIL ELEVATION**  
1" = 1'-0" |



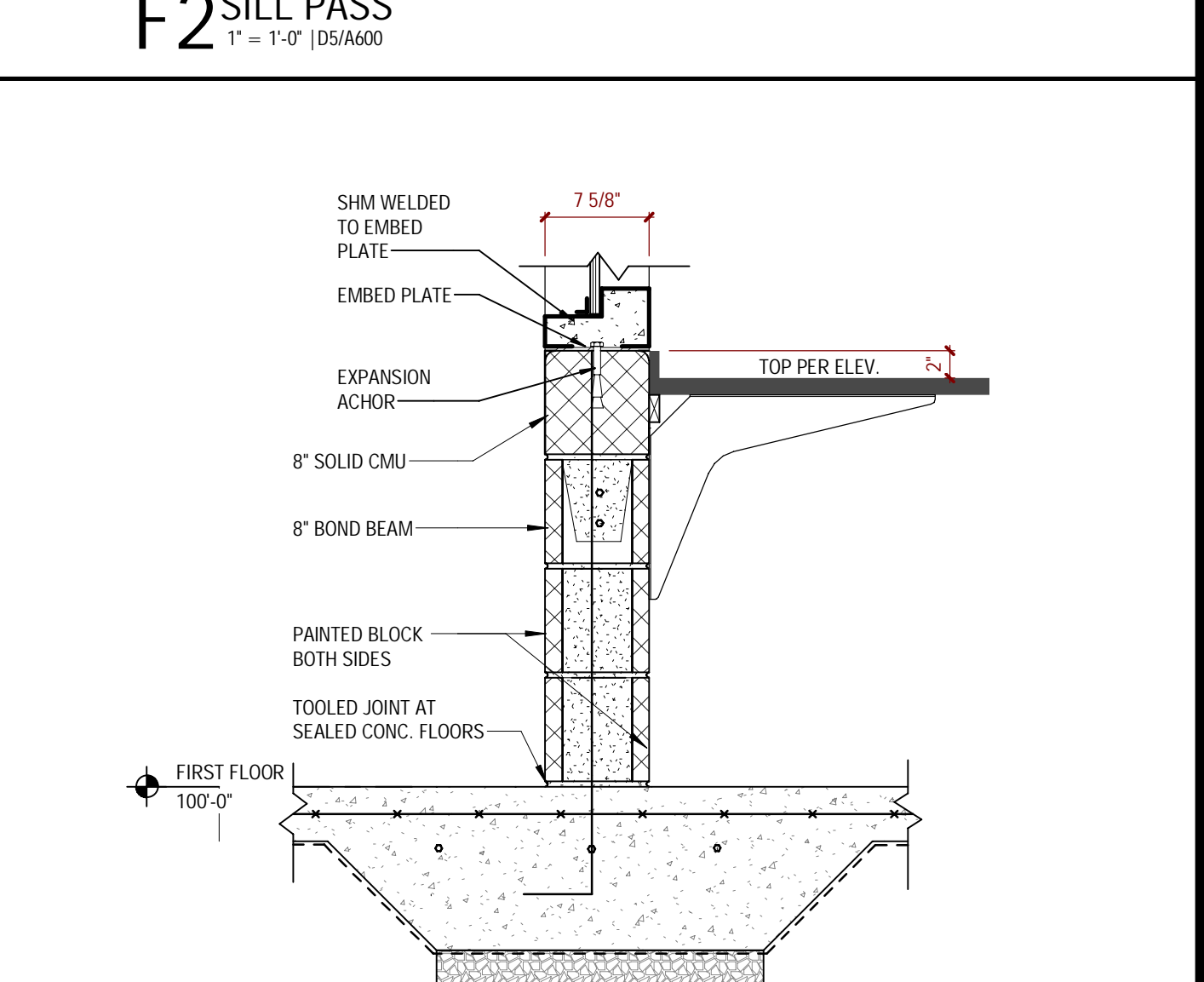
**F2 SILL PASS**  
1" = 1'-0" | D5/A600



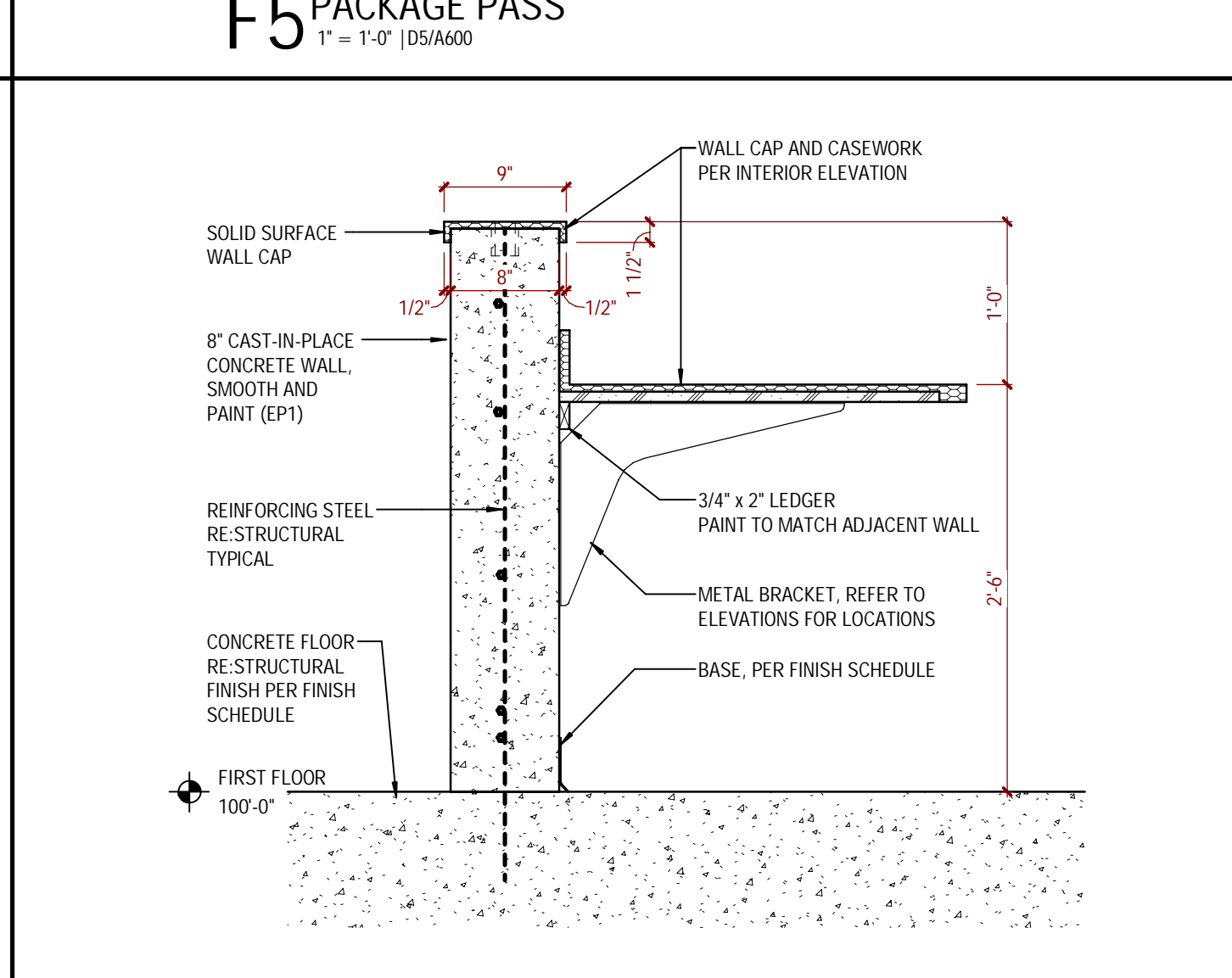
**F5 PACKAGE PASS**  
1" = 1'-0" | D5/A600



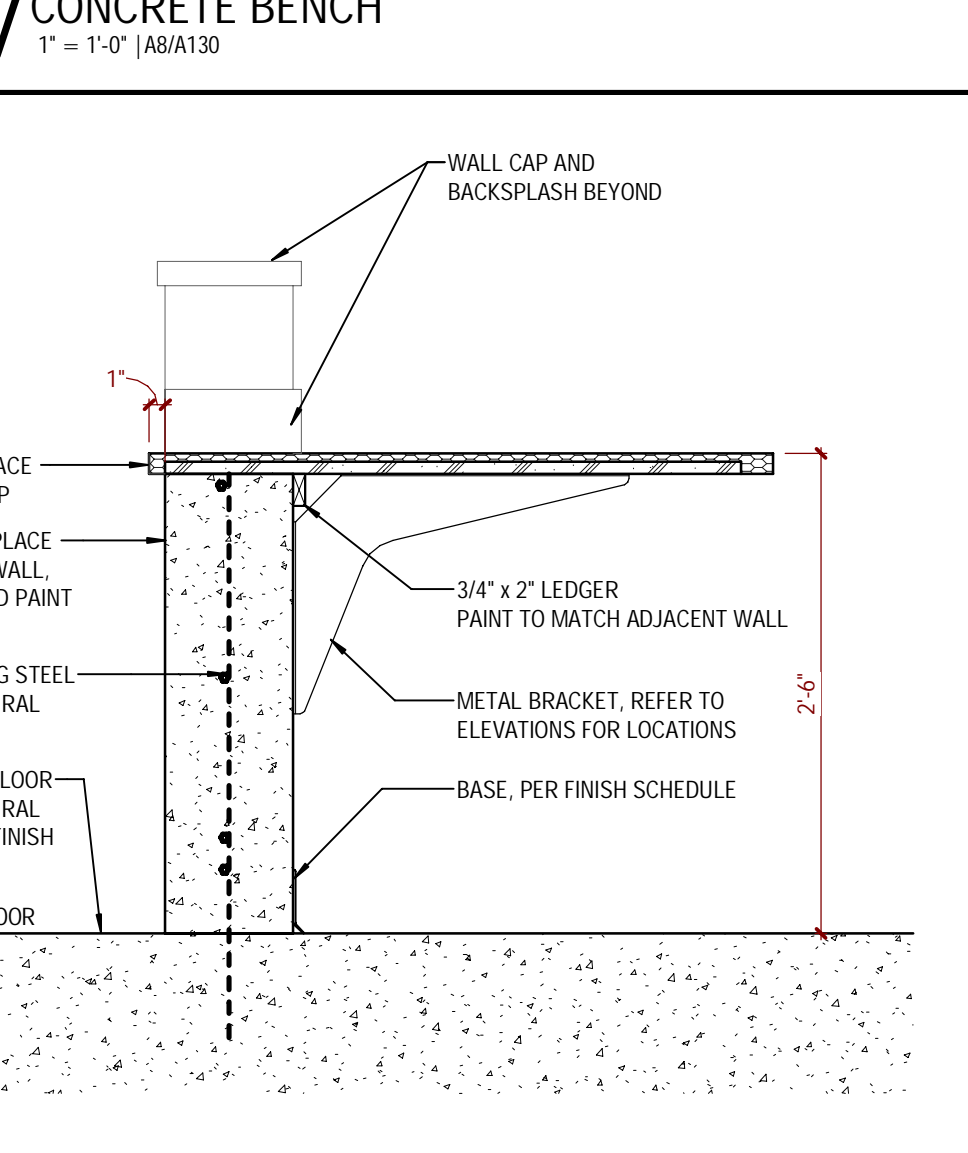
**F7 CONCRETE BENCH**  
1" = 1'-0" | A8/A130



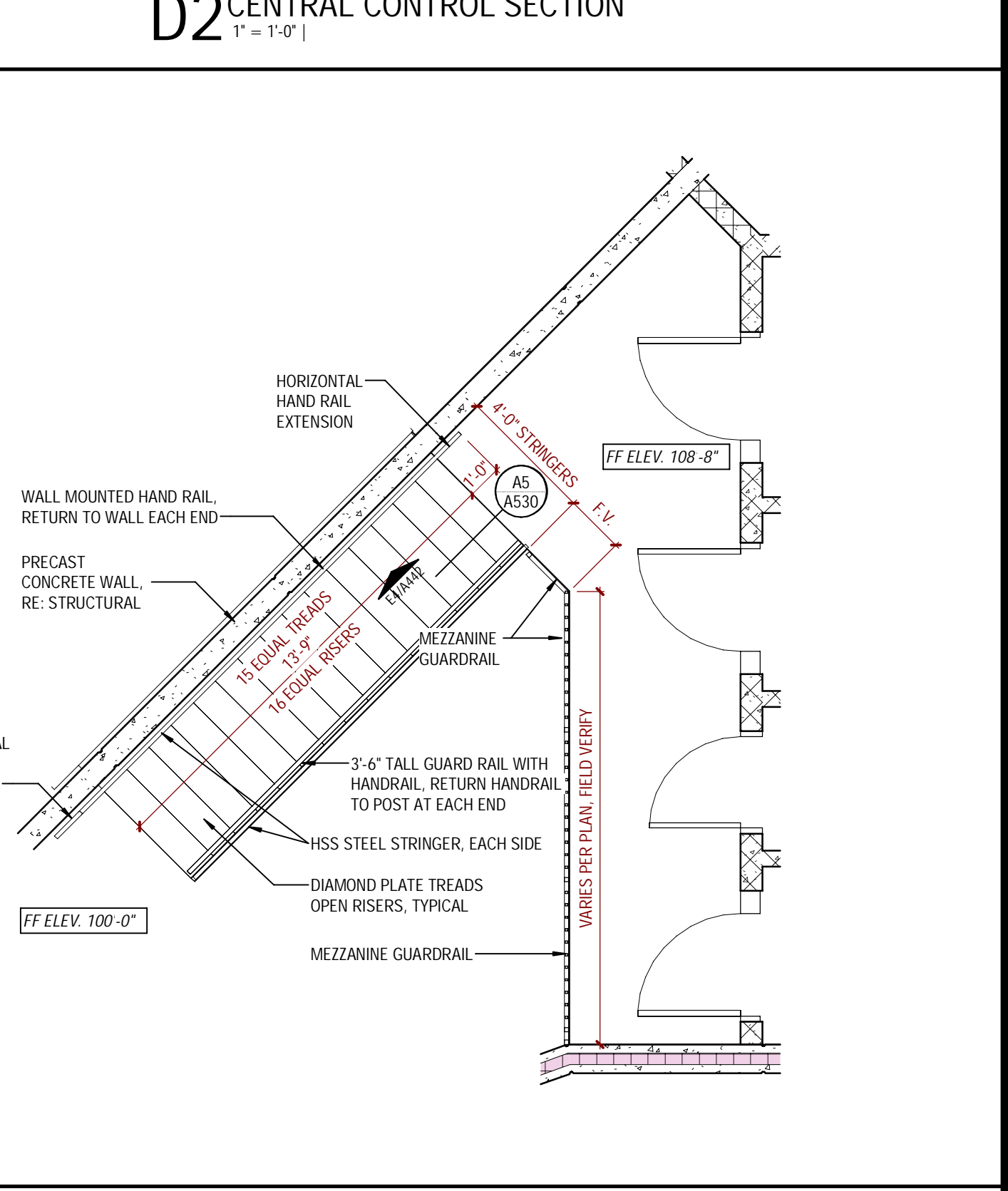
**D2 CENTRAL CONTROL SECTION**  
1" = 1'-0" |



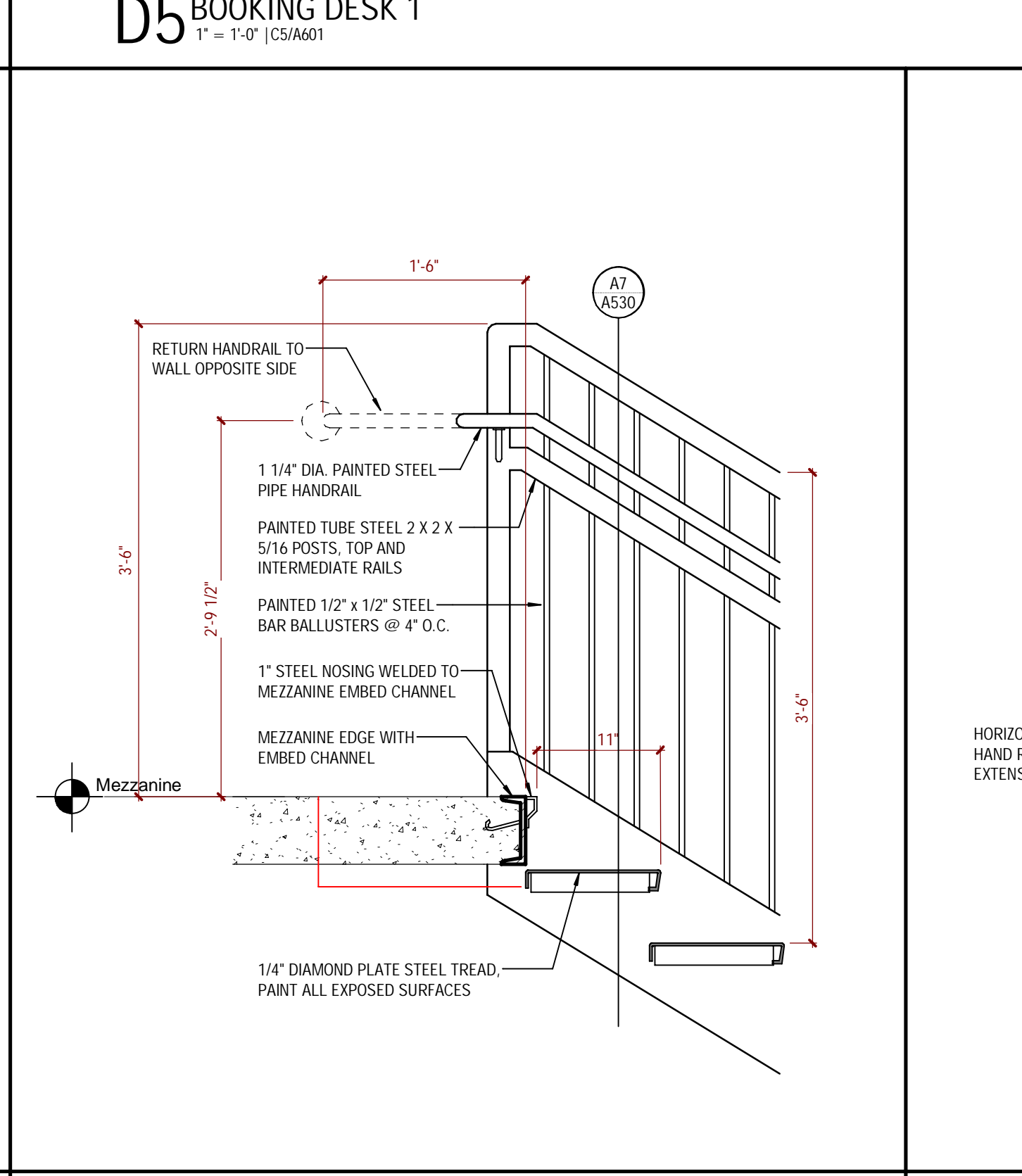
**D5 BOOKING DESK 1**  
1" = 1'-0" | C5/A601



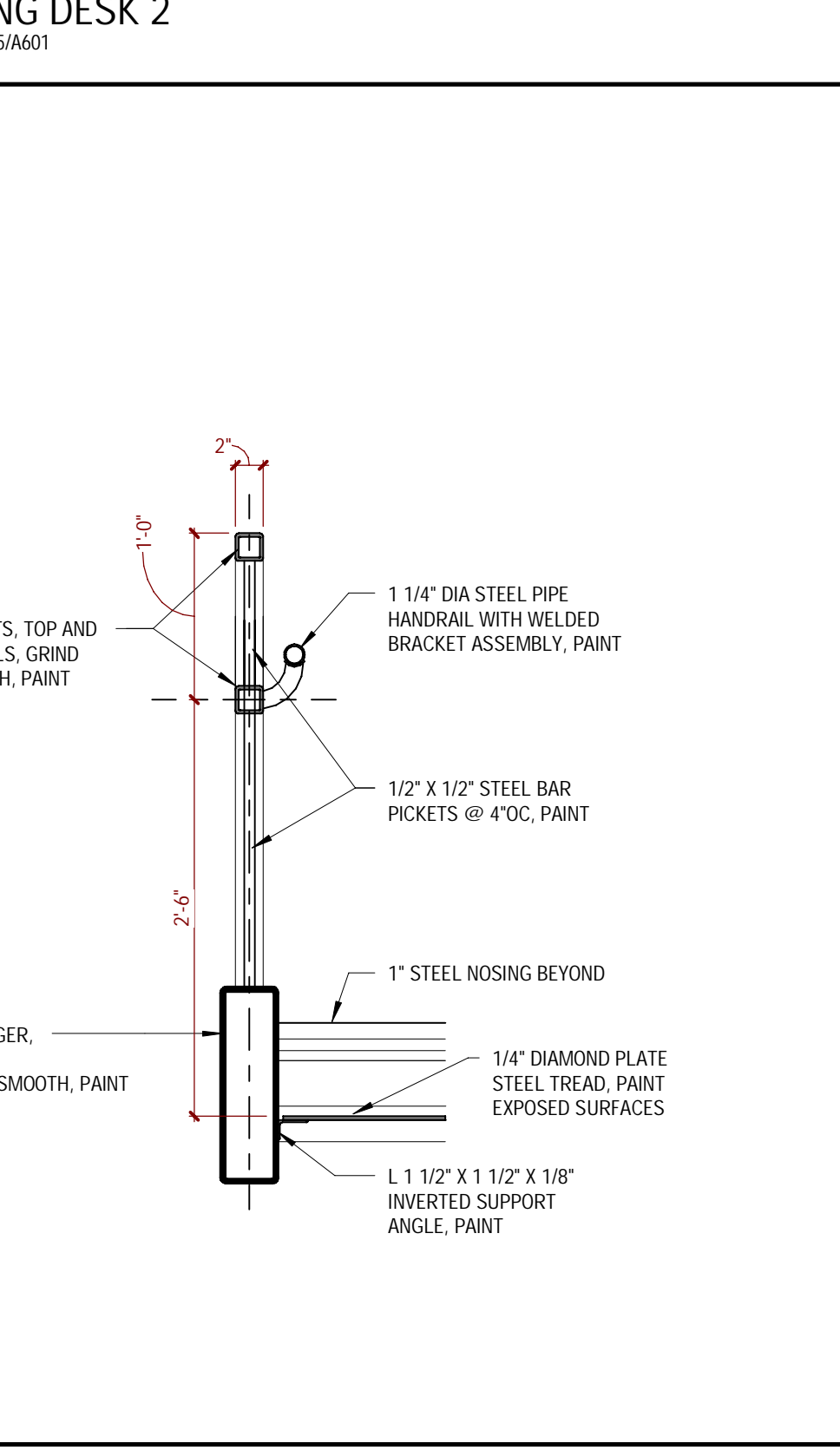
**D7 BOOKING DESK 2**  
1" = 1'-0" | C5/A601



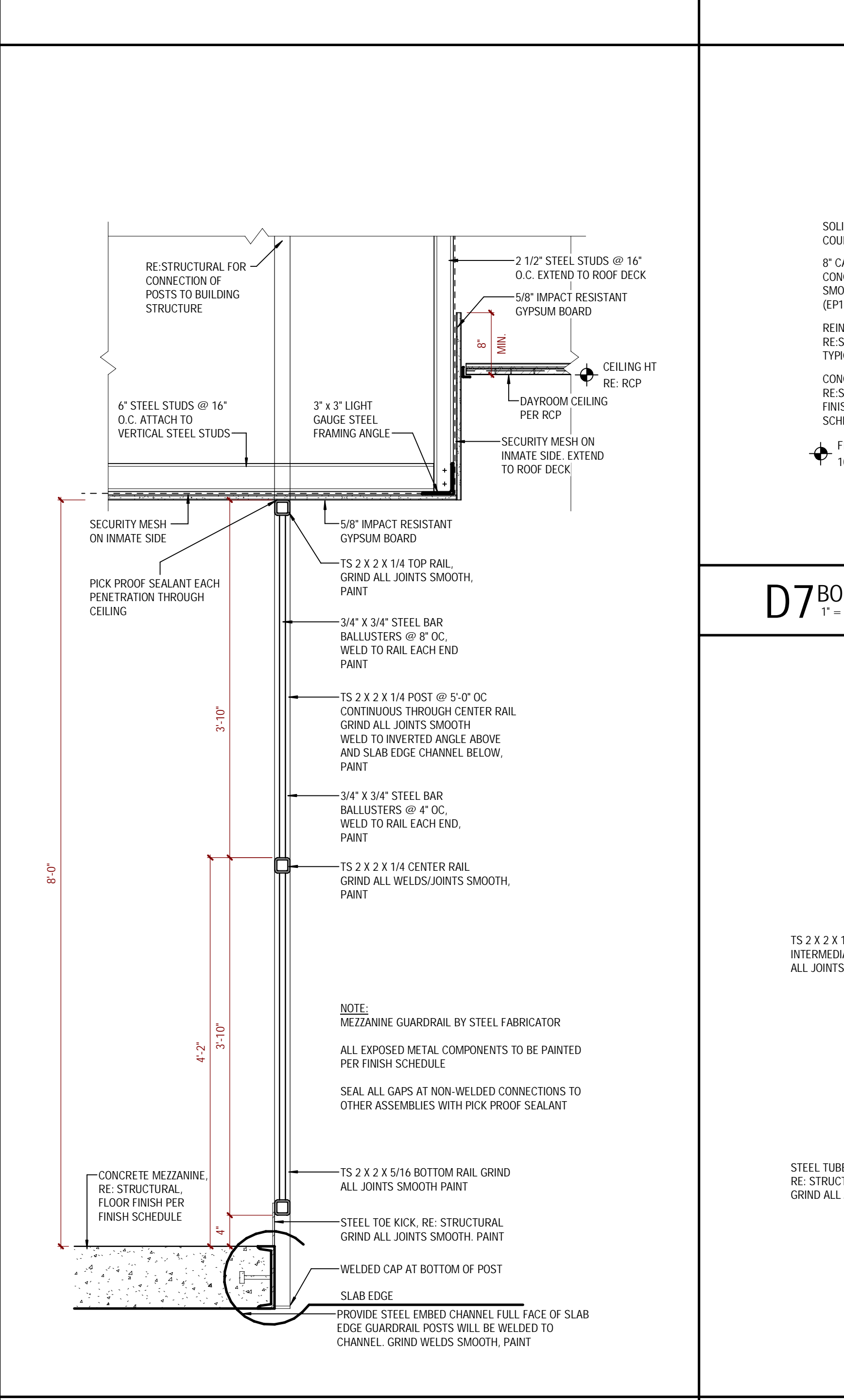
**A302 - MEZZANINE STAIR PLAN - TYPICAL**  
1/4" = 1'-0" | A5/A120



**A5 MEZZANINE STAIR LANDING**  
1" = 1'-0" | A3/A530



**A7 STAIR HANDRAIL**  
1" = 1'-0" | A5/A530



**A10 MEZZANINE GUARDRAIL SECTION 1**  
1" = 1'-0" | A6/A431



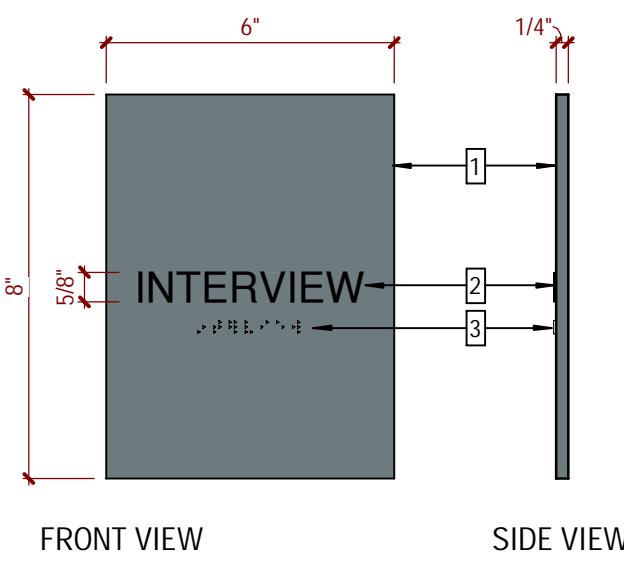
**INTERIOR SIGNAGE SCHEDULE** CONFIRM SIGNAGE QUANTITIES WITH EDGAR COUNTY STAFF

Number	Room Name (Per Plan)	Sign Text	Type	Notes
101	PUBLIC LOBBY	B - STAFF ONLY BEYOND THIS POINT; C - "EDGAR COUNTY"	B/C	B - ABOVE DOOR FROM LOBBY TO WORK AREA 106; C - CENTERED ON WALL ABOVE WINDOW TO WORK AREA 106 (REF. ELEVATION ON A600)
103	VIDEO VISIT	C = "V1", "V2"	A1/C	A1 = ON WALL NEXT TO DOOR FROM LOBBY; C = CENTERED ON WALL ABOVE VIDEO VISIT MONITORS (REF. ELEVATION ON A600)
104	TOILET		A3	
105	OFFICE STORAGE		F	
107	ADMIN. OFFICE		A1	
108	SHERIFF OFFICE		A1	
109	CHIEF DEPUTY OFFICE		A1	
110	DETECTIVE'S OFFICE		A1	
111	MECHANICAL		F	
113	ROAD PATROL/JAIL STAFF		A1	
115	INTERVIEW		A1	
116	TOILET		A3	
117	ATTORNEY VISIT		A1	
123	EVIDENCE PROCESS		A1	
127	JANITOR		F	
128	ARMSORY		A1	
129	MENS RESTROOM		A2	
131	WOMENS RESTROOM		A2	
134	SERVER ROOM		F	
135	CONFERENCE ROOM		A1	
135B	CONFERENCE STORAGE		F	
137	DISPATCH SUPERVISOR		A1	
138	DISPATCH		A1	
139	DISPATCH TOILET		A3	
140	DISPATCH BREAK		A1	
141	DISPATCH SERVER		F	
171	EMERGENCY ELECTRICAL MEZZ MECHANICAL MEZZ		F	

INFORMATION IN THIS COLUMN TO BE CONFIRMED WITH CLIENT DURING SUBMITTAL REVIEW

**PANEL SIGNS - INTERIOR ONLY** SCALE: 3" = 1'-0"

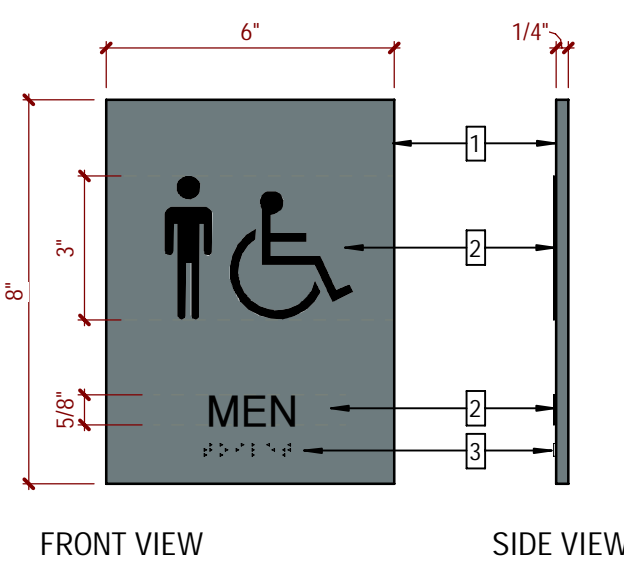
**SIGN TYPE A1**



QUANTITY / LOCATION: 13 - SEE SIGNAGE SCHEDULE

- MATERIAL:** 25" Acrylic (Square edge) Pantone Match to Sherwin Williams Foggy Day SW6235
- TEXT / SYMBOL:** Applied Raised Profile (Tactile) SansSerif Black (Matte)
- BRAILLE:** 1/32" Raised Braille Pantone Match to Sherwin Williams Gossamer Veil SW9165
- ATTACHMENT:** Vinyl Foam Tape

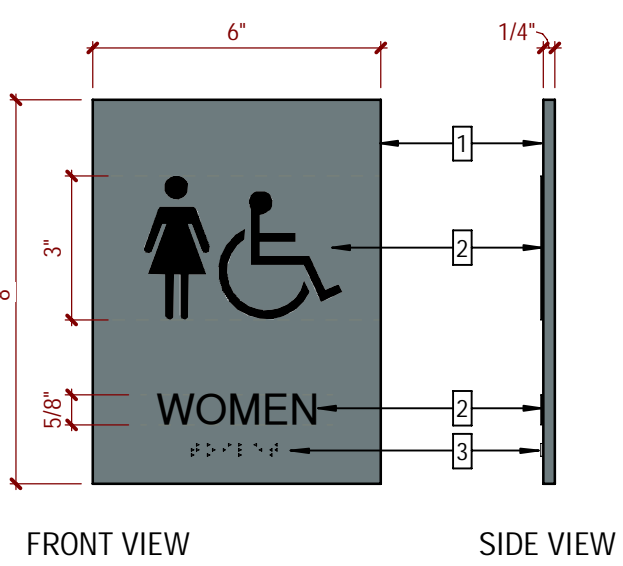
**SIGN TYPE A2 (MULTIUSER - MEN)**



QUANTITY / LOCATION: 1 - SEE SIGNAGE SCHEDULE

- MATERIAL:** 25" Acrylic (Square edge) Pantone Match to Sherwin Williams Foggy Day SW6235
- TEXT / SYMBOL:** Applied Raised Profile (Tactile) SansSerif Black (Matte)
- BRAILLE:** 1/32" Raised Braille Pantone Match to Sherwin Williams Gossamer Veil SW9165
- ATTACHMENT:** Vinyl Foam Tape

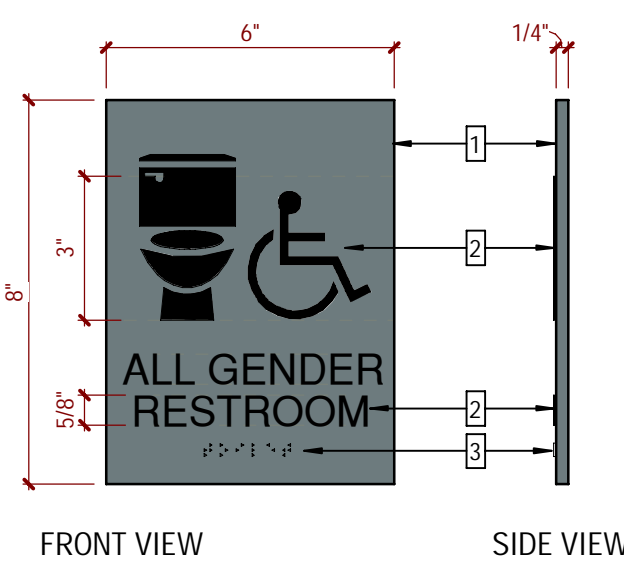
**SIGN TYPE A2 (MULTIUSER - WOMEN)**



QUANTITY / LOCATION: 1 - SEE SIGNAGE SCHEDULE

- MATERIAL:** 25" Acrylic (Square edge) Pantone Match to Sherwin Williams Foggy Day SW6235
- TEXT / SYMBOL:** Applied Raised Profile (Tactile) SansSerif Black (Matte)
- BRAILLE:** 1/32" Raised Braille Pantone Match to Sherwin Williams Gossamer Veil SW9165
- ATTACHMENT:** Vinyl Foam Tape

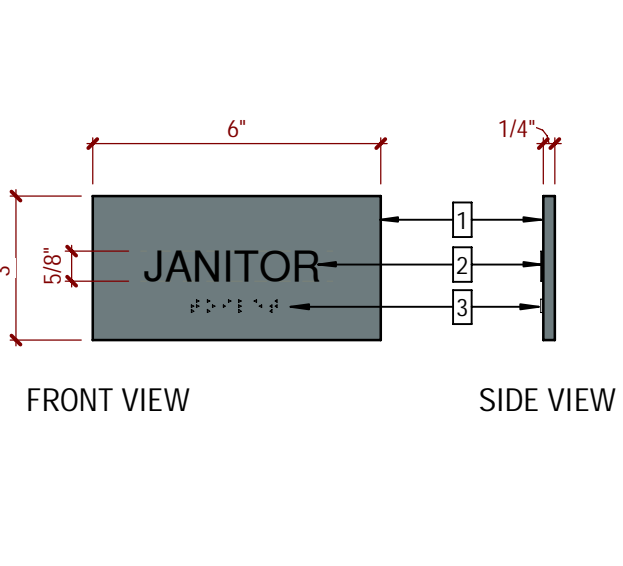
**SIGN TYPE A3 (SINGLE USER - ALL GENDER)**



QUANTITY / LOCATION: 3 - SEE SIGNAGE SCHEDULE

- MATERIAL:** 25" Acrylic (Square edge) Pantone Match to Sherwin Williams Foggy Day SW6235
- TEXT / SYMBOL:** Applied Raised Profile (Tactile) SansSerif Black (Matte)
- BRAILLE:** 1/32" Raised Braille Pantone Match to Sherwin Williams Gossamer Veil SW9165
- ATTACHMENT:** Vinyl Foam Tape

**SIGN TYPE F**



QUANTITY / LOCATION: 6 - SEE SIGNAGE SCHEDULE

- MATERIAL:** 25" Acrylic (Square edge) Pantone Match to Sherwin Williams Foggy Day SW6235
- TEXT / SYMBOL:** Applied Raised Profile (Tactile) SansSerif Black (Matte)
- BRAILLE:** 1/32" Raised Braille Pantone Match to Sherwin Williams Gossamer Veil SW9165
- ATTACHMENT:** Vinyl Foam Tape

FOR STORM SHELTER SIGNS, SEE SHEET G101

**DIMENSIONAL CHARACTERS** SCALE: 3" = 1'-0"

**SIGN TYPE B (INTERIOR)**



- QUANTITY:** 24 Characters - See Signage Schedule
- MATERIAL:** 1/2" Gal Aluminum Painted Black (Gloss)
- TEXT:** Bahnschrift Semibold
- ATTACHMENT:** Concealed Pin Mount
- NOTES:** Interior For Locations & Mounting Heights, Ref. Interior Elevations D4/A600

**SIGN TYPE C (INTERIOR)**



- QUANTITY:** 17 Characters - See Signage Schedule
- MATERIAL:** 1/2" Gal Aluminum Painted Black (Gloss)
- TEXT:** Bahnschrift Semibold
- ATTACHMENT:** Concealed Pin Mount
- NOTES:** Interior For Locations & Mounting Heights, Ref. Interior Elevations

**SIGN TYPE D** EXTERIOR SIGN ON NORTH AND WEST FACADES

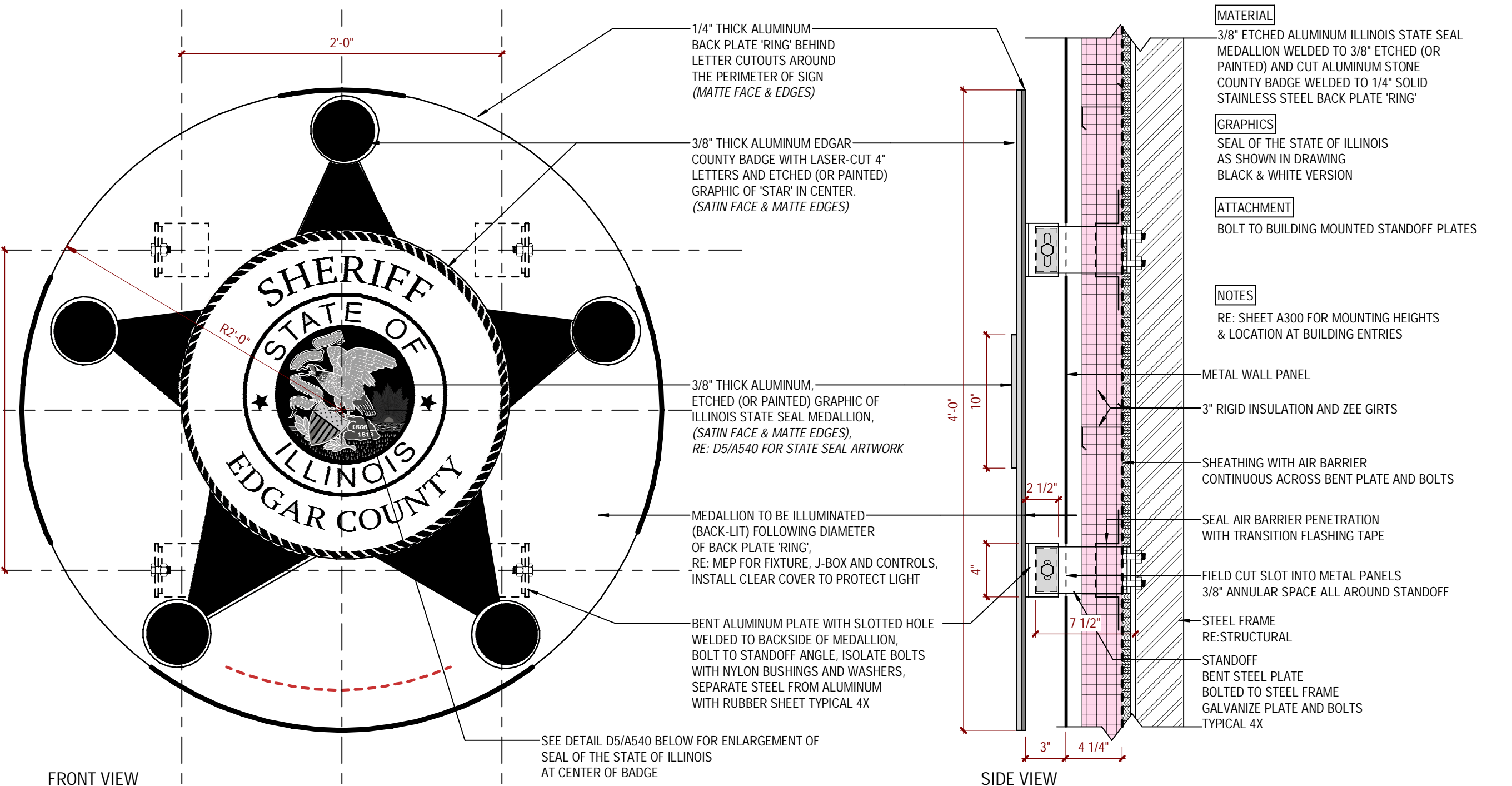


- QUANTITY:** 28 Characters
- MATERIAL:** 1" Gal Aluminum Brushed Aluminum Finish
- TEXT:** Bahnschrift Semibold
- ATTACHMENT:** Concealed Pin Mount
- NOTES:** Exterior For Locations & Mounting Heights, Ref. Exterior Elevations on sheet A300

NOTE: THIS SHEET IS ONLY VALID IF PRINTED IN COLOR

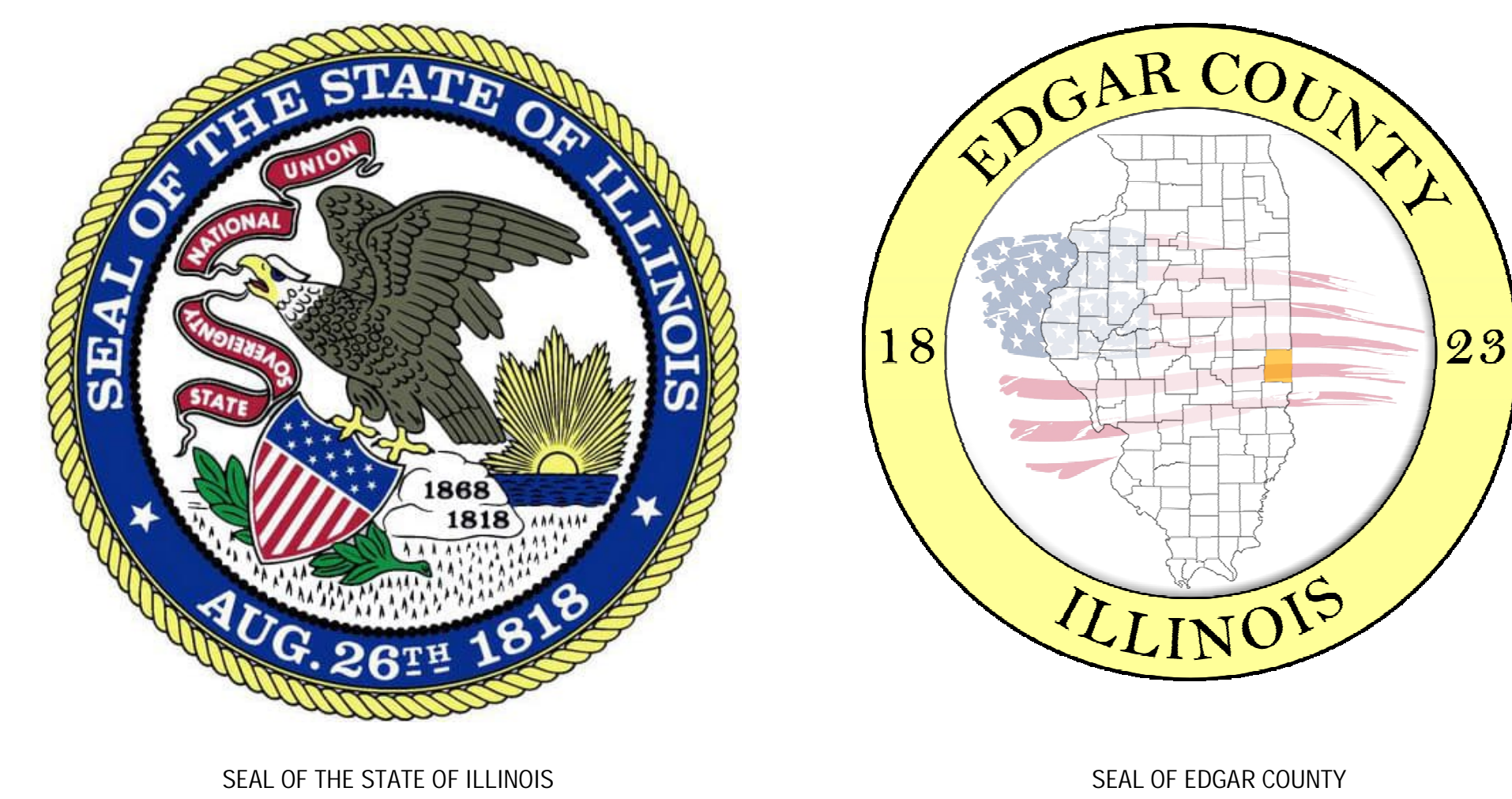
**MEDALLION SIGN - EXTERIOR ONLY**

**SIGN TYPE E**



- QUANTITY:** 2
- MATERIAL:** 3/8" ETCHED ALUMINUM ILLINOIS STATE SEAL MEDALLION WELDED TO 3/8" ETCHED (OR PAINTED) AND CUT ALUMINUM STONE COUNTY BADGE WELDED TO 1/4" SOLID STAINLESS STEEL BACK PLATE RING
- GRAPHIC:** SEAL OF THE STATE OF ILLINOIS AS SHOWN IN DRAWING BLACK & WHITE VERSION
- ATTACHMENT:** BOLT TO BUILDING MOUNTED STANDOFF PLATES
- NOTES:** RE: SHEET A300 FOR MOUNTING HEIGHTS & LOCATION AT BUILDING ENTRIES

**F5 EXTERIOR MEDALLION SIGN** 1 1/2" - 1 1/4" (W/A300)

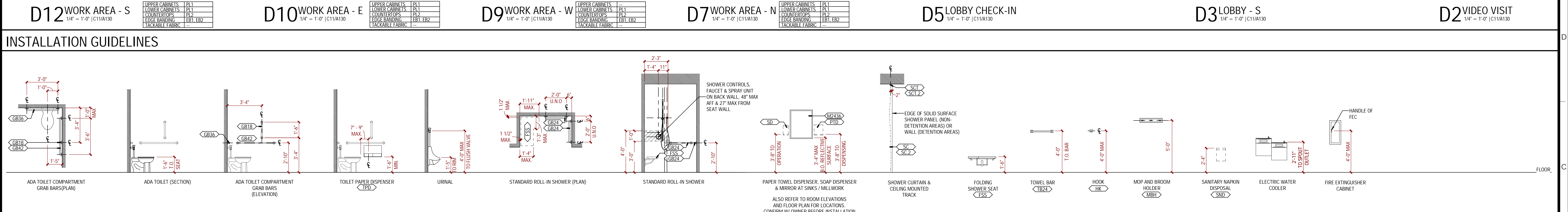
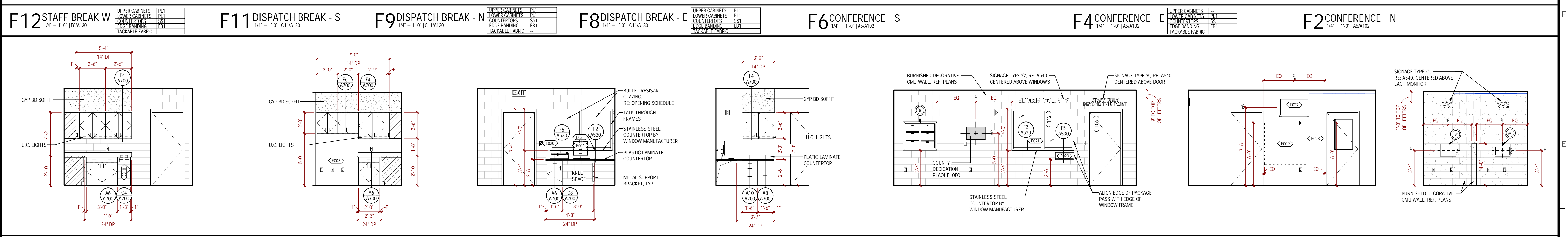
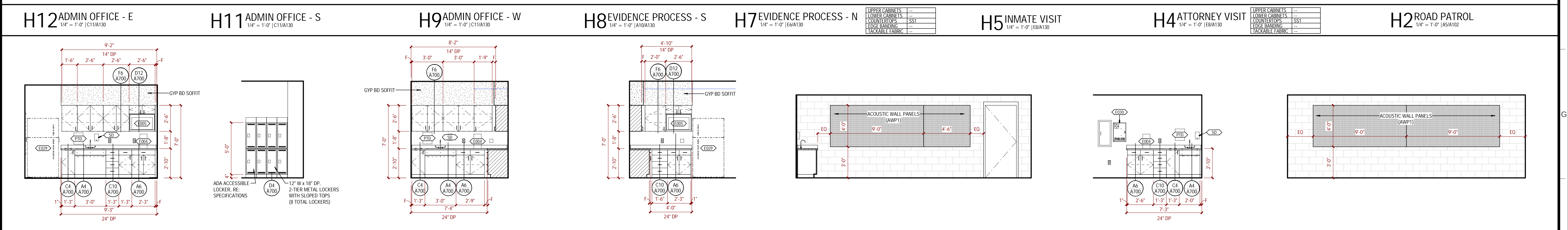
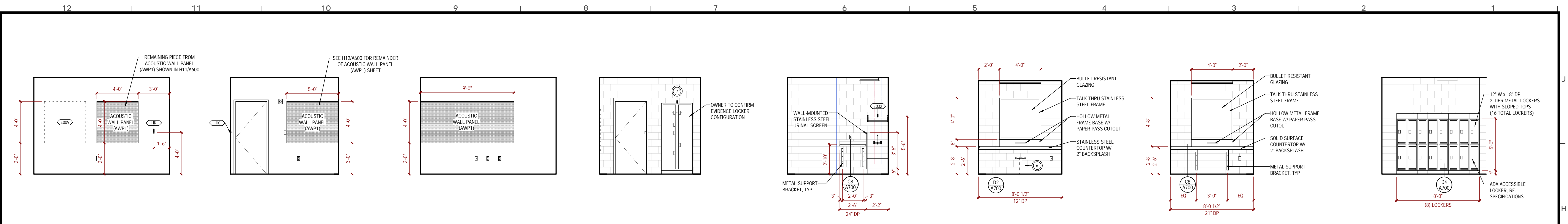


SEAL OF THE STATE OF ILLINOIS

SEAL OF EDGAR COUNTY

**D5 STATE AND COUNTY SEALS** N.T.S.

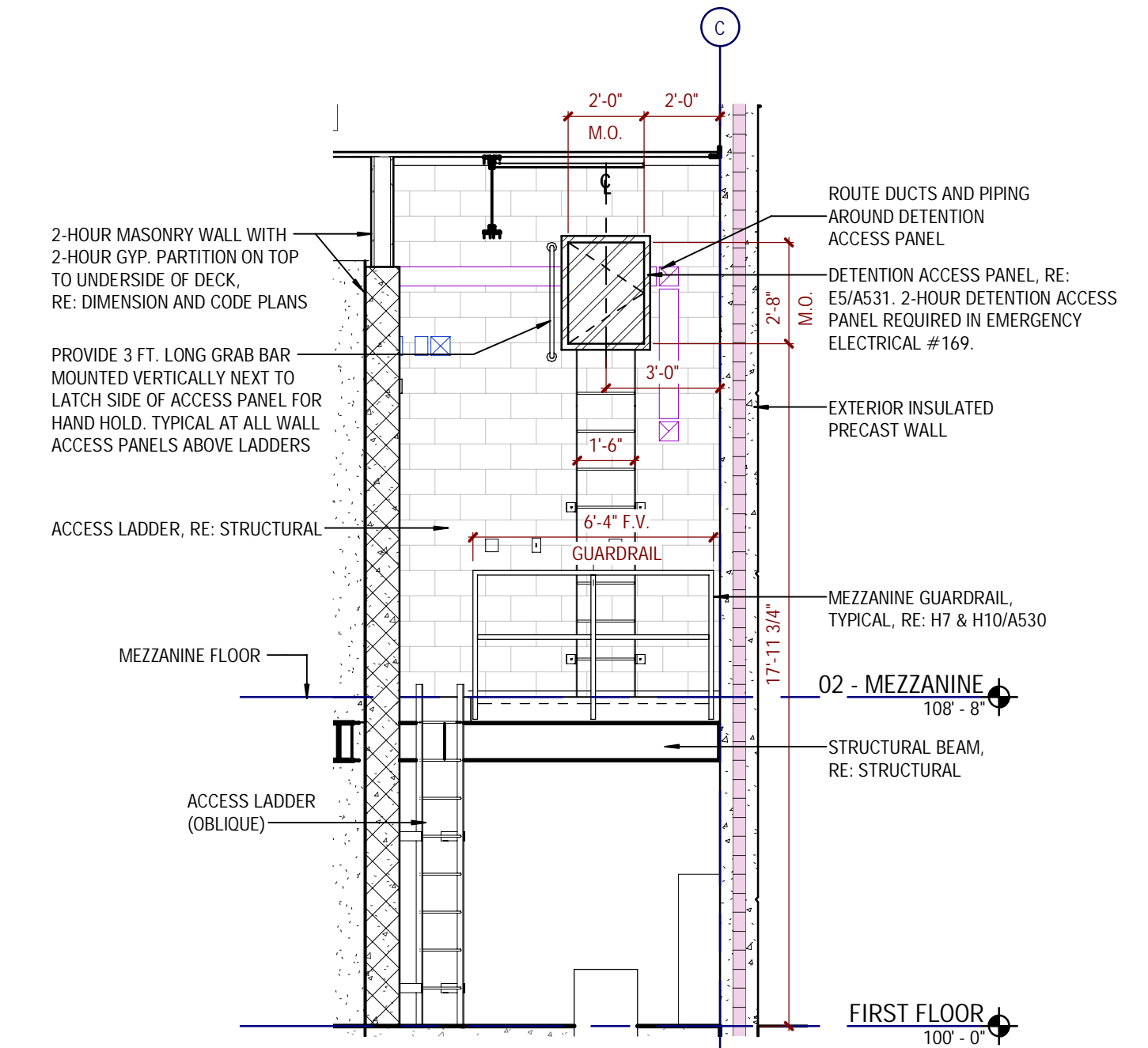
Issue	Date



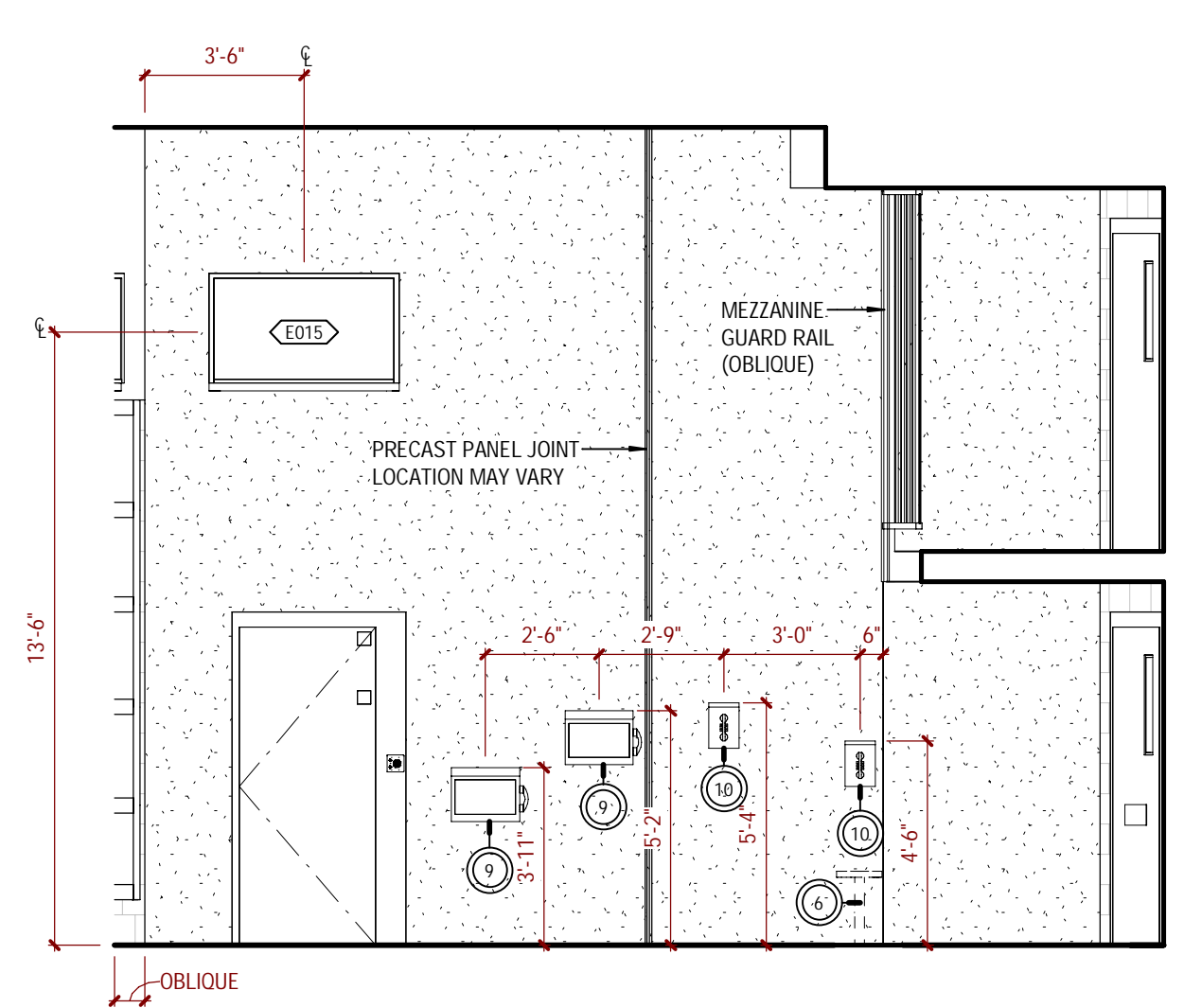
GENERAL INTERIOR ELEV. NOTES				INTERIOR ELEVATION LEGEND				DETENTION EQUIPMENT				EQUIPMENT SCHEDULE				MATERIALS LEGEND - MILLWORK																																																																																																																																																																																																																																																			
<p>1. CONTRACTOR SHALL REINFORCE WALLS AT LOCATIONS OF WALL HUNG ITEMS WITH METAL OR FIRE TREATED WOOD IN-WALL BLOCKING AS REQUIRED. COORDINATE WITH THE OWNER ON OWNER FURNISHED OWNER INSTALLED (OFBI) ITEMS.</p> <p>2. DIMENSIONS FOR MEP ITEMS ARE TO THE CENTERLINE OF COVERPLATE OR CENTER OF OPERABLE MECHANISM, TYPICAL.</p> <p>3. ALL ELECTRICAL, MECHANICAL, AND PLUMBING ITEMS SHOWN ARE FOR REFERENCE AND LOCATION ONLY. REFER TO MEP DRAWINGS FOR SIZES, TYPES, AND QUANTITIES. WHERE ARCHITECT PROVIDES DIMENSIONS TO MEP ITEMS, INSTALL SUCH ITEMS PER ARCHITECT INSTRUCTION. F ARCHITECT DIMENSION CONFLICTS WITH MEP INSTRUCTION OR REQUIREMENT, SUBMIT RFI TO ARCHITECT FOR CLARIFICATION.</p> <p>4. ALL COUNTERTOPS SHALL BE 24" DEEP, FRONT TO BACK EDGES, UNLESS NOTED OTHERWISE.</p> <p>5. ALL UPPER CABINETS SHALL BE 14" DEEP, FROM FACE OF WALL TO FACE OF CABINET BOX, UNLESS NOTED OTHERWISE.</p>				<p><b>CASEWORK SECTION CUT</b></p> <p><b>GYP/SUM BOARD SOFFIT</b></p> <p><b>TACK SURFACE</b></p> <p><b>WALL PROTECTION</b></p> <p><b>WOOD</b></p> <p><b>FINISH TAG</b></p> <p><b>RE: A700 SERIES FOR COUNTERTOP DETAILS</b></p> <p><b>SECTION: DETAIL SECTION</b></p> <p><b>CASEWORK DOOR/DRAWER LOCKED</b></p> <p><b>FINISHED END PANEL (FEP)</b></p>				<table border="1"> <thead> <tr> <th>CODE</th> <th>ITEM DESCRIPTION</th> <th>SPEC SECTION</th> <th>PROVIDED BY</th> </tr> </thead> <tbody> <tr> <td>C</td> <td>CABLE</td> <td></td> <td></td> </tr> <tr> <td>D</td> <td>DATA</td> <td></td> <td></td> </tr> <tr> <td>DP</td> <td>DATAPHONE</td> <td></td> <td></td> </tr> <tr> <td>E</td> <td>EMERGENCY POWER</td> <td></td> <td></td> </tr> <tr> <td>E4</td> <td>EMERGENCY POWER, QUAD</td> <td></td> <td></td> </tr> <tr> <td>N</td> <td>NORMAL POWER</td> <td></td> <td></td> </tr> <tr> <td>N4</td> <td>NORMAL POWER, QUAD</td> <td></td> <td></td> </tr> <tr> <td>P</td> <td>PHONE</td> <td></td> <td></td> </tr> <tr> <td>S</td> <td>SWITCH</td> <td></td> <td></td> </tr> </tbody> </table>				CODE	ITEM DESCRIPTION	SPEC SECTION	PROVIDED BY	C	CABLE			D	DATA			DP	DATAPHONE			E	EMERGENCY POWER			E4	EMERGENCY POWER, QUAD			N	NORMAL POWER			N4	NORMAL POWER, QUAD			P	PHONE			S	SWITCH			<table border="1"> <thead> <tr> <th>CODE</th> <th>ITEM DESCRIPTION</th> <th>RESPONSIBILITY</th> <th>COMMENTS</th> </tr> </thead> <tbody> <tr> <td>E001</td> <td>Computer Monitor (Desktop)</td> <td>OFBI</td> <td></td> </tr> <tr> <td>E002</td> <td>Exam Table</td> <td>X</td> <td></td> </tr> <tr> <td>E003</td> <td>Printer/Copy/Fax (Floor)</td> <td>X</td> <td></td> </tr> <tr> <td>E004</td> <td>Coffee Maker</td> <td>X</td> <td></td> </tr> <tr> <td>E005</td> <td>Microwave</td> <td>X</td> <td></td> </tr> <tr> <td>E006</td> <td>Refrigerator</td> <td>X</td> <td></td> </tr> <tr> <td>E007</td> <td>PHONE (DESKTOP)</td> <td>X</td> <td></td> </tr> <tr> <td>E008</td> <td>Bread/Toaster (Countertop)</td> <td>X</td> <td></td> </tr> <tr> <td>E009</td> <td>Track Board</td> <td>X</td> <td></td> </tr> <tr> <td>E010</td> <td>TRASH BIN (24 GAL)</td> <td>X</td> <td></td> </tr> <tr> <td>E011</td> <td>Commercial Washer</td> <td>X</td> <td>SEE SECTION 11 1100</td> </tr> <tr> <td>E016</td> <td>52" ENCASED MESSAGE MONITOR</td> <td>X</td> <td>SEE SECTION 11 1100</td> </tr> <tr> <td>E017</td> <td>Refrigerator (full size w/ice maker)</td> <td>X</td> <td></td> </tr> <tr> <td>E018</td> <td>Marker Board</td> <td>X</td> <td></td> </tr> <tr> <td>E019</td> <td>Folding Table</td> <td>X</td> <td></td> </tr> <tr> <td>E020</td> <td>Package Pass</td> <td>X</td> <td></td> </tr> <tr> <td>E021</td> <td>Drop-In Deal Tray</td> <td>X</td> <td>SEE SECTION 10 2813.63 - 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<p><b>TOILET ACCESSORIES SCHEDULE</b></p> <table border="1"> <thead> <tr> <th>CODE</th> <th>ITEM DESCRIPTION</th> <th>OFBI</th> <th>OFCI</th> <th>CFCI</th> <th>COMMENTS</th> </tr> </thead> <tbody> <tr> <td>FSS</td> <td>Folding Shower Seat</td> <td></td> <td></td> <td>X</td> <td></td> </tr> <tr> <td>GB18</td> <td>Vertical Grab Bar (18")</td> <td></td> <td></td> <td>X</td> <td></td> </tr> <tr> <td>GB24</td> <td>Grab Bar (24")</td> <td></td> <td></td> <td>X</td> <td></td> </tr> <tr> <td>GB36</td> <td>Grab Bar (36")</td> <td></td> <td></td> <td>X</td> <td></td> </tr> <tr> <td>GB42</td> <td>Grab Bar (42")</td> <td></td> <td></td> <td>X</td> <td></td> </tr> <tr> <td>HK</td> <td>Hook</td> <td></td> <td></td> <td>X</td> <td></td> </tr> <tr> <td>M2436</td> <td>Mirror (24" x 36")</td> <td></td> <td></td> <td>X</td> <td></td> </tr> <tr> <td>M8H</td> <td>Mop &amp; Broom Holder</td> <td></td> <td></td> <td>X</td> <td></td> </tr> <tr> <td>PTD</td> <td>Paper Towel Dispenser</td> <td></td> <td></td> <td>X</td> <td></td> </tr> <tr> <td>SC</td> <td>Shower Curtain</td> <td></td> <td></td> <td>X</td> <td></td> </tr> <tr> <td>SC2</td> <td>Detention Shower Curtain (Tear Away)</td> <td></td> <td></td> <td>X</td> <td></td> </tr> <tr> <td>SC3</td> <td>Shower Curtain Track</td> <td></td> <td></td> <td>X</td> <td></td> </tr> <tr> <td>SD</td> <td>Soap Dispenser</td> <td></td> <td></td> <td>X</td> <td></td> </tr> <tr> <td>SND</td> <td>Sanitary Napkin Disposal</td> <td></td> <td></td> <td>X</td> <td></td> </tr> <tr> <td>TB24</td> <td>Towel Bar</td> <td></td> <td></td> <td>X</td> <td></td> </tr> <tr> <td>TPD</td> <td>Toilet Paper Dispenser</td> <td></td> <td></td> <td>X</td> <td></td> </tr> </tbody> </table>				CODE	ITEM DESCRIPTION	OFBI	OFCI	CFCI	COMMENTS	FSS	Folding Shower Seat			X		GB18	Vertical Grab Bar (18")			X		GB24	Grab Bar (24")			X		GB36	Grab Bar (36")			X		GB42	Grab Bar (42")			X		HK	Hook			X		M2436	Mirror (24" x 36")			X		M8H	Mop & Broom Holder			X		PTD	Paper Towel Dispenser			X		SC	Shower Curtain			X		SC2	Detention Shower Curtain (Tear Away)			X		SC3	Shower Curtain Track			X		SD	Soap Dispenser			X		SND	Sanitary Napkin Disposal			X		TB24	Towel Bar			X		TPD	Toilet Paper Dispenser			X																																																																																																																																																											
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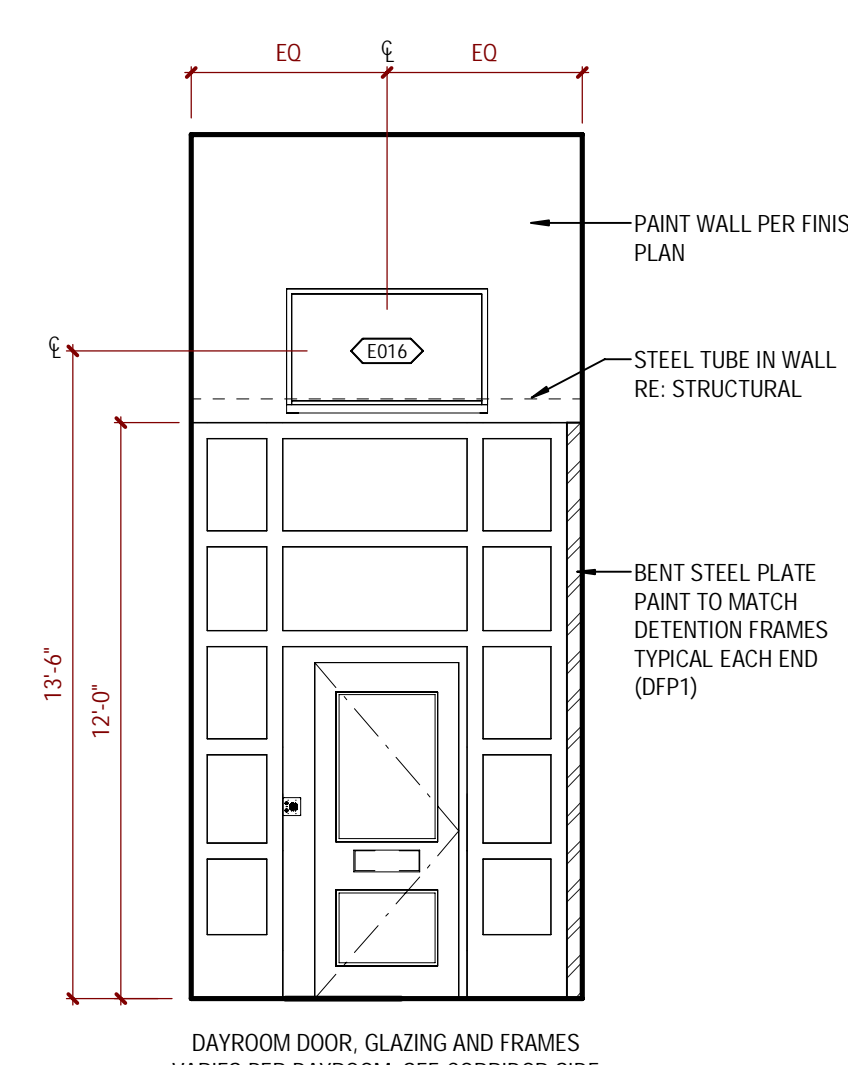




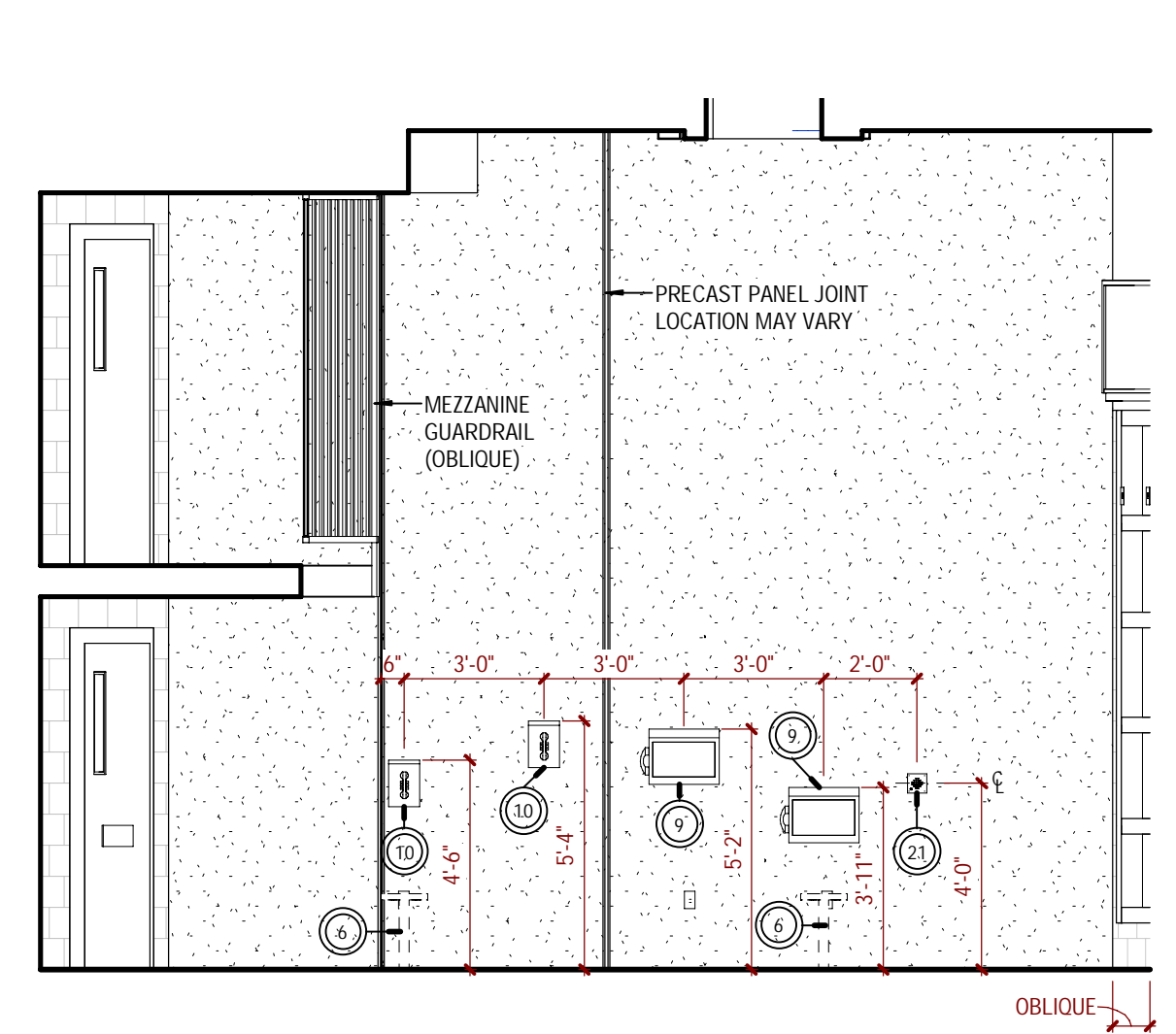
E2 MECHANICAL ROOM  
1/4" = 1'-0" (A109A120)



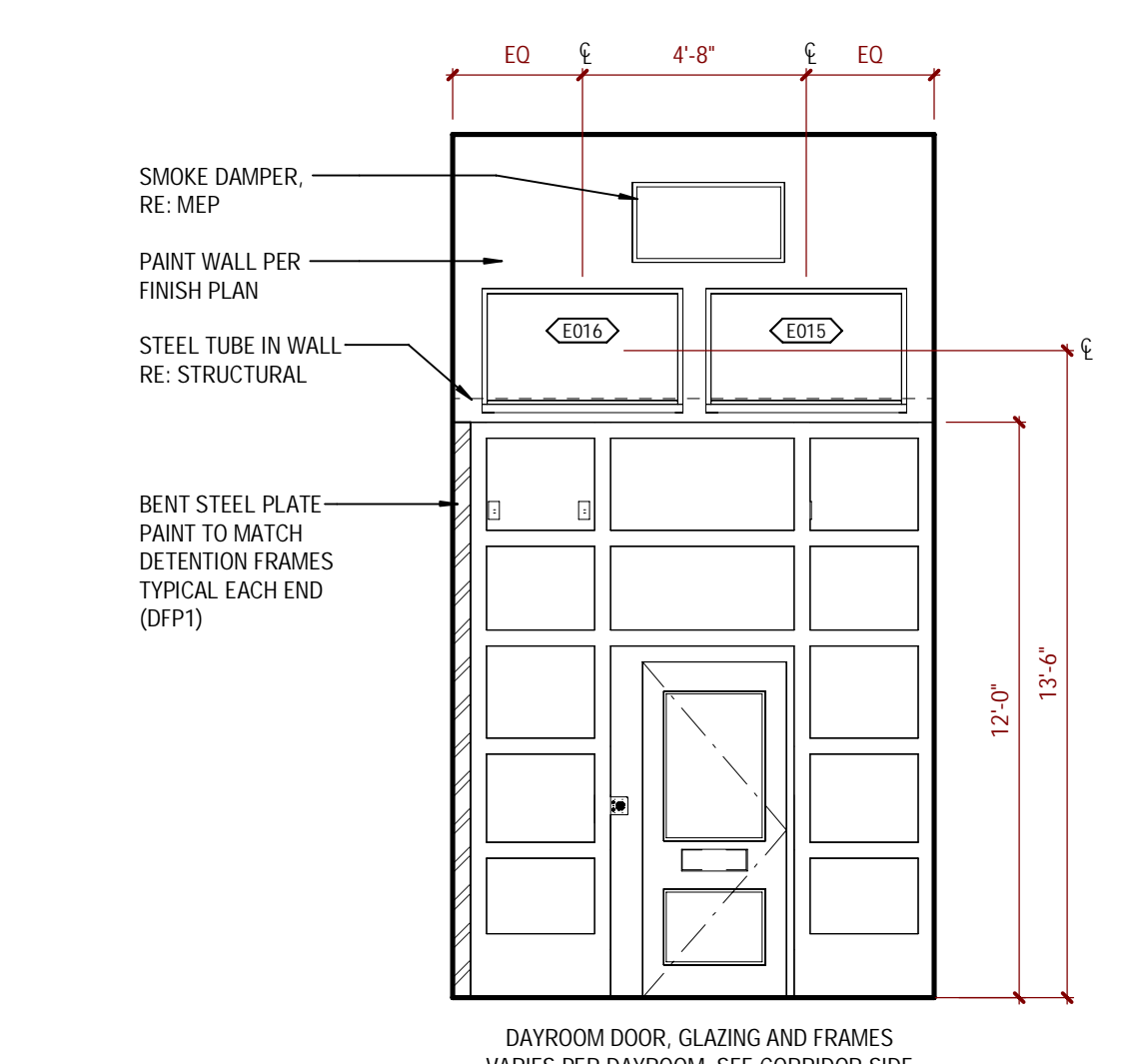
C10 DAYROOM C & D SIDEWALL  
1/4" = 1'-0" (A51A102) DAYROOM C - OPPOSITE



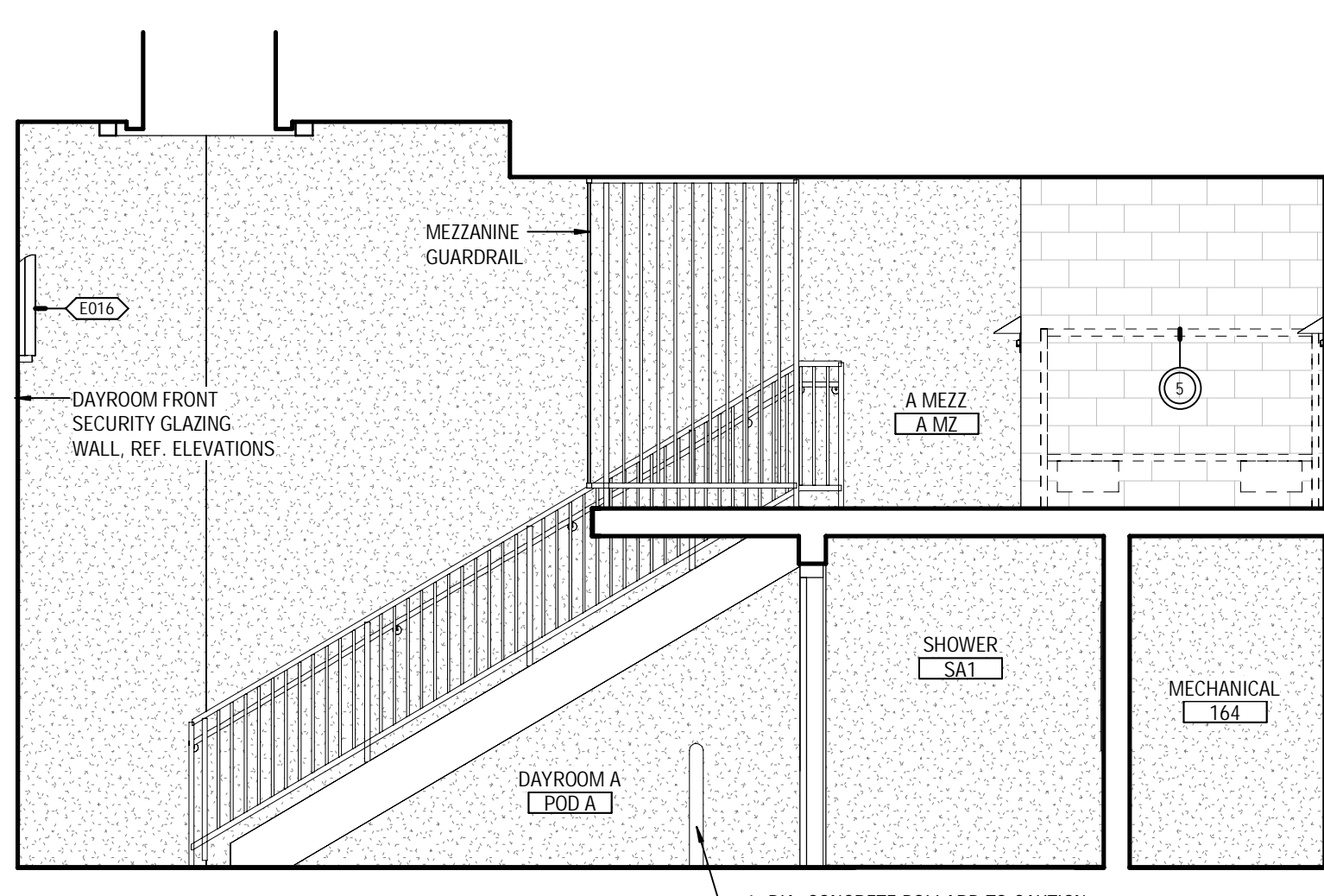
C7 DAYROOM C & D FRONT WALL  
1/4" = 1'-0" (A51A102)



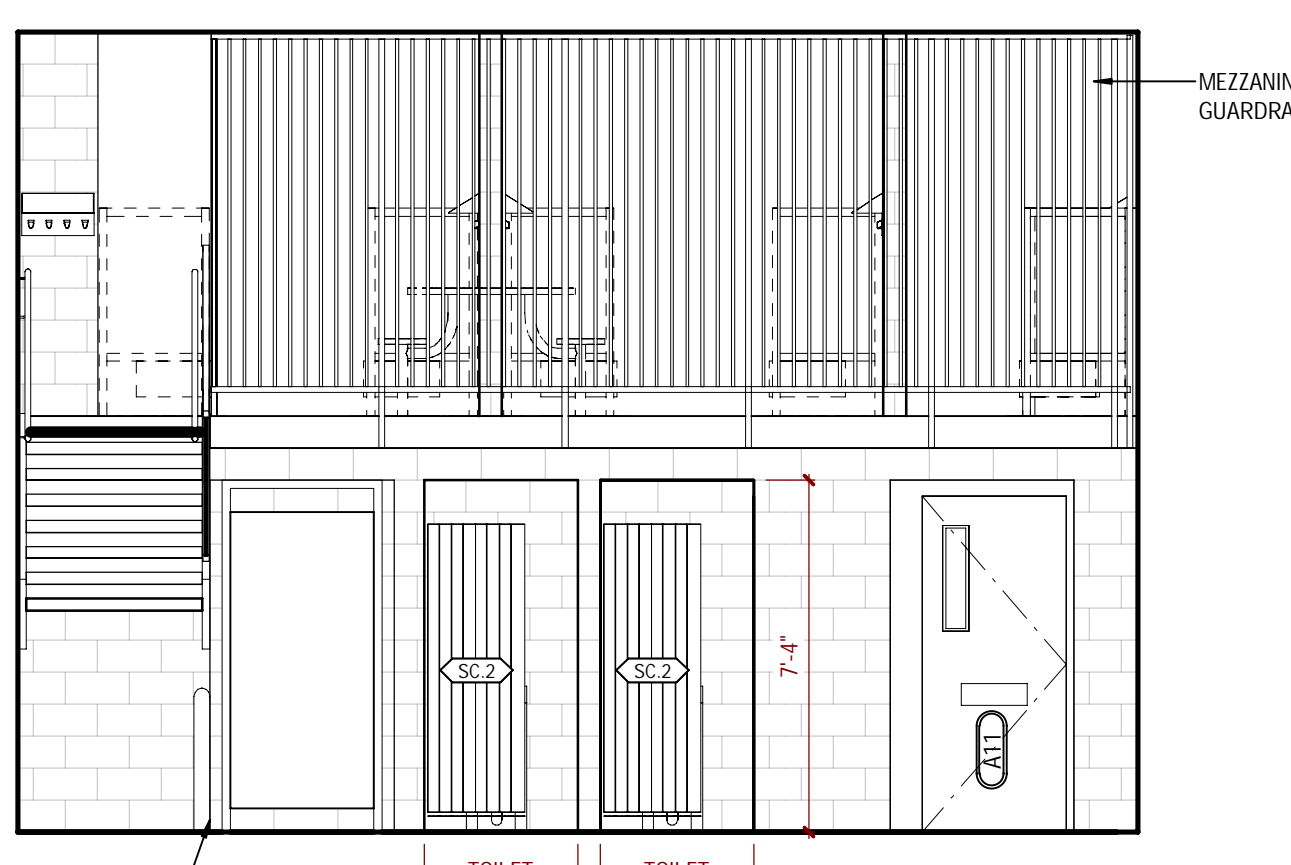
C5 DAYROOM B & E SIDEWALL  
1/4" = 1'-0" (A51A102) DAYROOM B - OPPOSITE



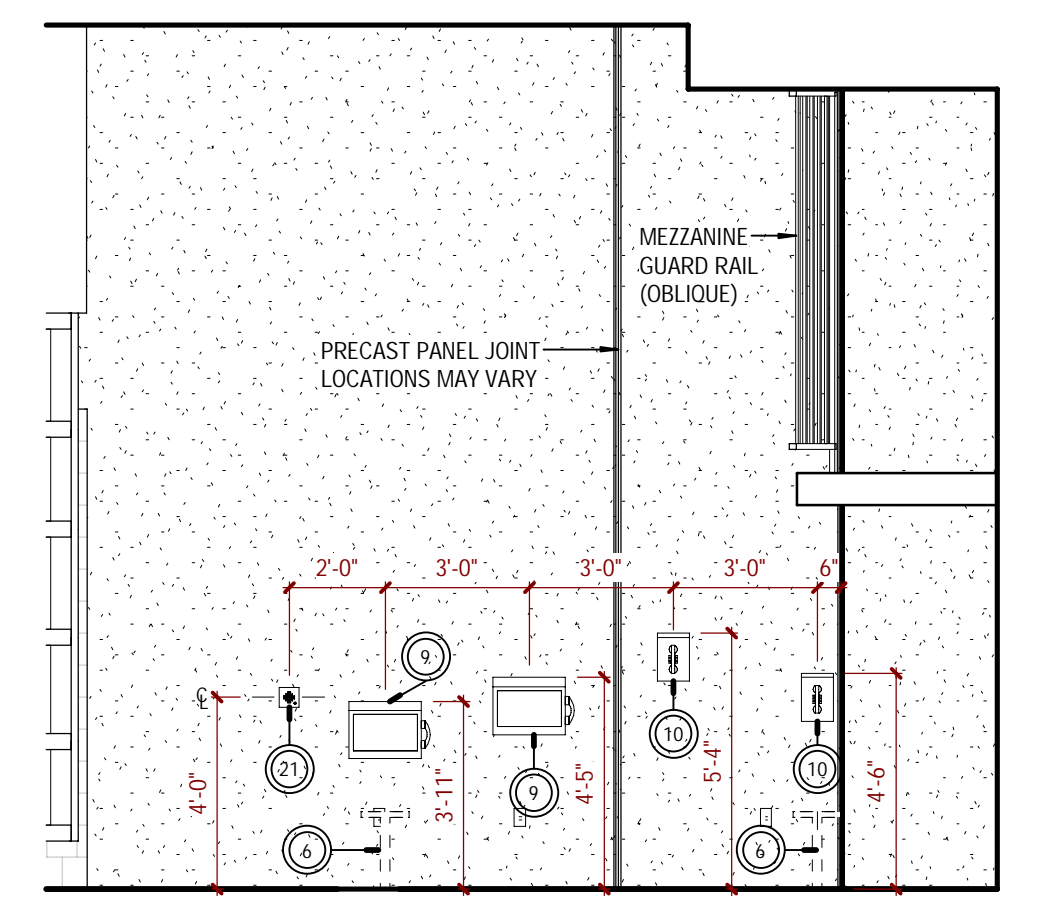
C2 DAYROOM B & E FRONT WALL  
1/4" = 1'-0" (A51A102)



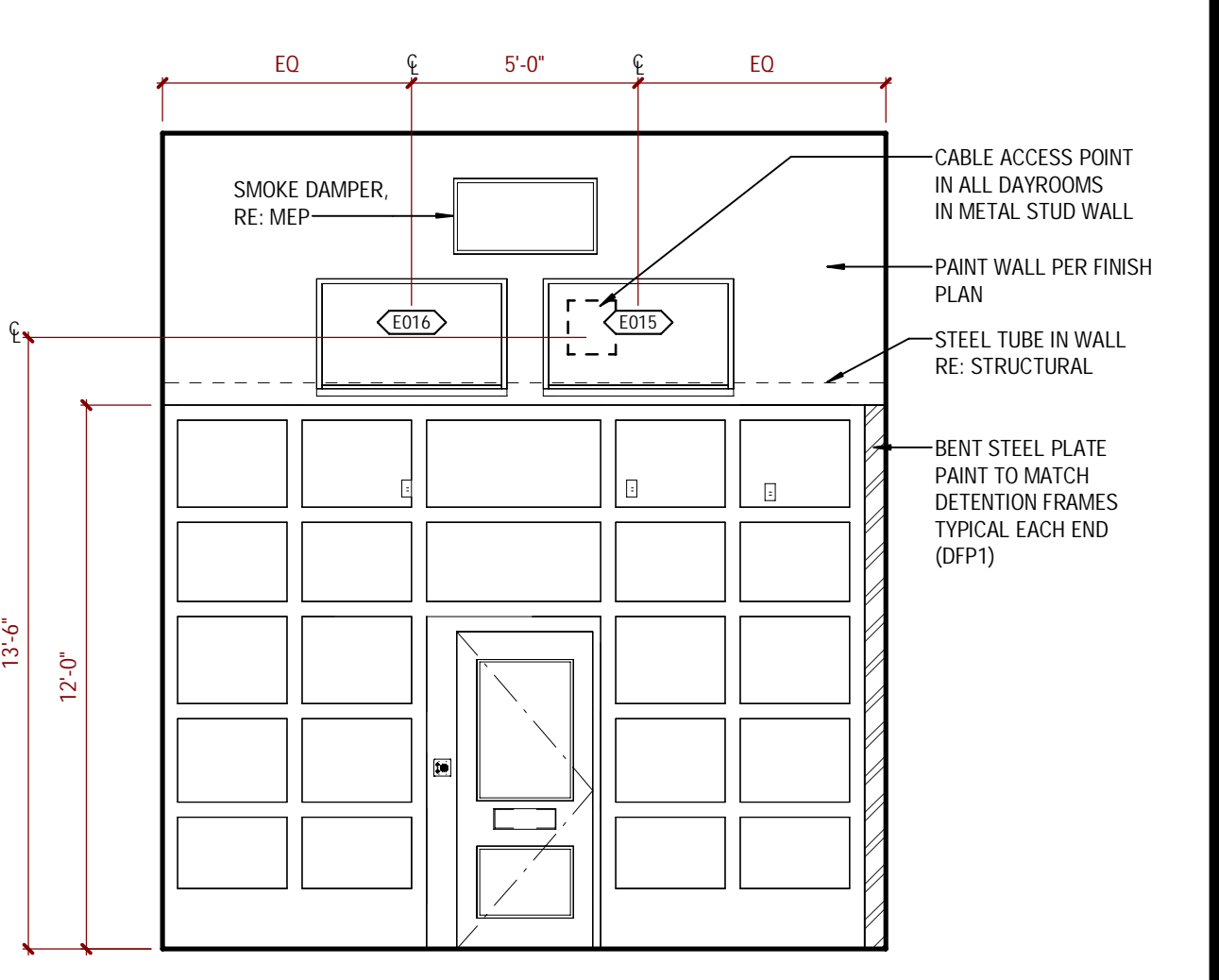
A11 DAYROOM A & F STAIRS  
1/4" = 1'-0" (A7/A131)



A7 DAYROOM A & F BACK WALL  
1/4" = 1'-0" (E8/A130)



A5 DAYROOM A & F SIDE WALL  
1/4" = 1'-0" (A5/A102) DAYROOM A - OPPOSITE



A2 DAYROOM A & F FRONT WALL  
1/4" = 1'-0" (A5/A102)

J	H	G	F	E	D
G	F	E	D	C	B
F	E	D	C	B	A
E	D	C	B	A	
D	C	B	A		
J	H	G	F	E	D
I	H	G	F	E	D
H	G	F	E	D	C
G	F	E	D	C	B
F	E	D	C	B	A
E	D	C	B	A	
D	C	B	A		
C	B	A			
B	A				
A					
J	H	G	F	E	D
I	H	G	F	E	D
H	G	F	E	D	C
G	F	E	D	C	B
F	E	D	C	B	A
E	D	C	B	A	
D	C	B	A		
C	B	A			
B	A				
A					
J	H	G	F	E	D
I	H	G	F	E	D
H	G	F	E	D	C
G	F	E	D	C	B
F	E	D	C	B	A
E	D	C	B	A	
D	C	B	A		
C	B	A			
B	A				
A					

**EDGAR COUNTY PUBLIC SAFETY CENTER**  
 EDGAR COUNTY, ILLINOIS  
 12636 950TH ROAD  
 PARIS, ILLINOIS 61944  
 HMN 21003.003  
 BID & PERMIT

KLINGNER ARCHITECT PROJECT #		22-4046
Date		03/01/2024
B	Issue	
	Date	
Drawn by: Author		

CASEWORK SECTIONS  
A700

3/1/2024 11:29:34 AM

OPENING SCHEDULE (STANDARD)

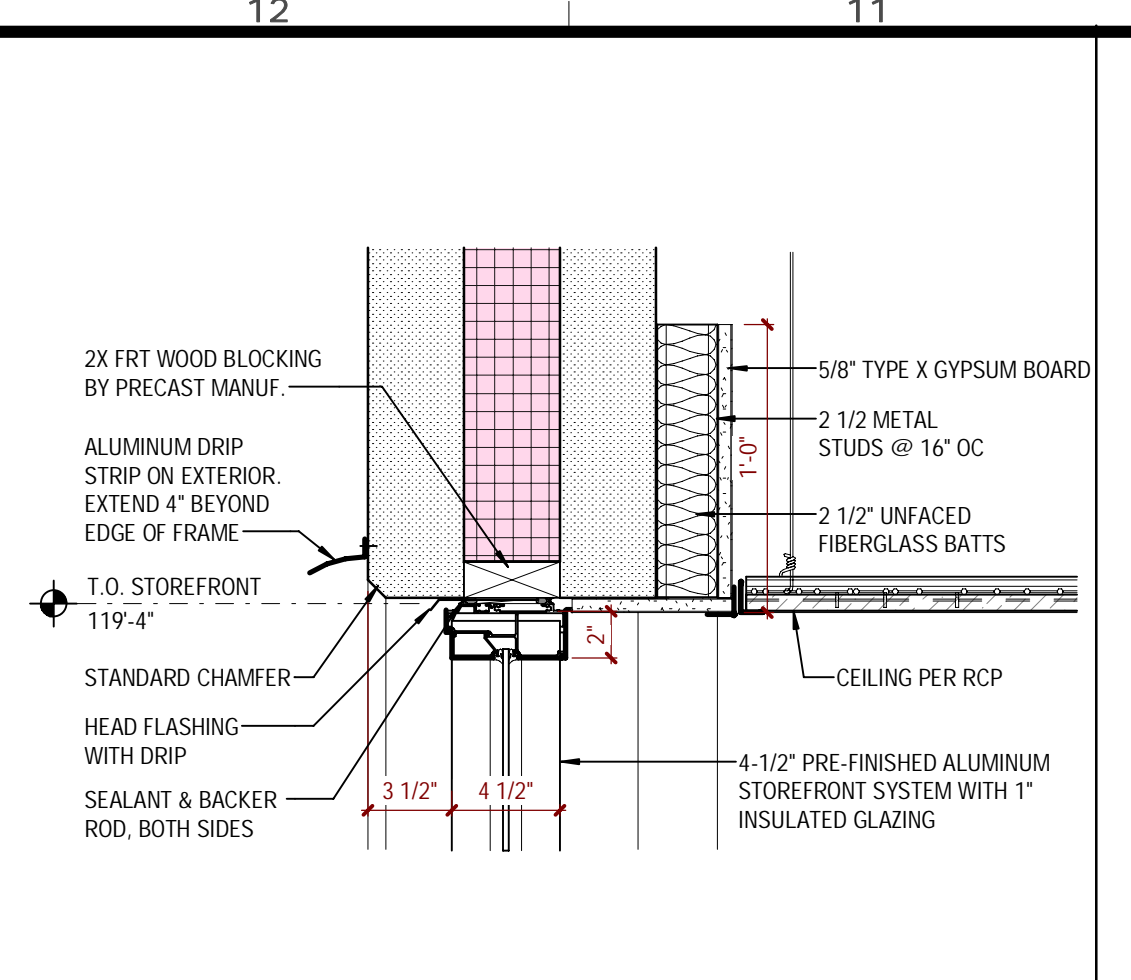
MARK	TYPE	OPENING (REFER TO OPENING NOTES BELOW)				FRAME (REFER TO OPENING NOTES BELOW)				FIRE RATING	SMOKE CONTROL	HARDWARE SET	COMMENTS	Revision #			
		WIDTH	UNEQUAL LEAF WIDTH	HEIGHT	THICKNESS	DOOR TYPE	DOOR MATERIAL	GLAZING TYPE	FRAME MATERIAL						HEAD	JAMB	SILL
FIRST FLOOR																	
101	PR	3'-0"		7'-0"	1-3/4"	FG-CW	ALUM	TT	HM	E6	C6	C10	---	1.0	Intercom at exterior (S/E)		
103	F-M	3'-0"		7'-0"	1-3/4"	FEMA	WD	TT	HM	E6	C6	---	No	24.0	Monitored		
104	S	3'-0"		7'-0"	1-3/4"	F	WD	---	HM	E6	C6	---	No	20.0	---		
105A	S-M	3'-0"		7'-0"	1-3/4"	F	WD	---	HM	E6	C6	---	No	36.0	FEMA ICC-500 Storm door, frame and hardware, Storage lockset		
105B	S	2'-6"		7'-0"	1-3/4"	F	WD	---	HM	E6	C6	---	No	7.0	---		
106	S-M	3'-0"		7'-0"	1-3/4"	F	WD	---	HM	E6	C6	---	No	32.0	Card Reader and Intercom to enter (S/E)		
107	S	3'-0"		7'-0"	1-3/4"	F	WD	---	HM	E6	C6	---	No	6.0	---		
108	S	3'-0"		7'-0"	1-3/4"	F	WD	---	HM	E6	C6	---	No	6.0	---		
109	S	3'-0"		7'-0"	1-3/4"	F	WD	---	HM	E6	C6	---	No	6.0	---		
110	S	3'-0"		7'-0"	1-3/4"	F	WD	---	HM	E6	C6	---	No	6.0	---		
111	S	3'-0"		7'-0"	1-3/4"	F	WD	---	HM	E6	C6	---	No	13.0	Mech room, storage lockset		
114A	S	3'-0"		7'-0"	1-3/4"	FG-CW	ALUM	2T	ALUM	---	---	---	No	2.0	Card Reader to enter, Intercom on both sides (S/E)		
114B	S-M	3'-0"		7'-0"	1-3/4"	F	WD	---	HM	E6	C6	---	No	35.0	Card reader and Intercom both sides (S/E)		
115	S-M	3'-0"		7'-0"	1-3/4"	F	WD	---	HM	E6	C6	---	No	22.0	Request to exit		
116	S-M	3'-0"		7'-0"	1-3/4"	F	WD	---	HM	E6	C6	---	No	19.0	---		
117A	S-M	3'-0"		7'-0"	1-3/4"	N	WD	---	HM	E6	C6	---	No	19.0	---		
119	S-M	3'-0"		7'-0"	1-3/4"	N	WD	---	HM	E6	C6	---	No	33.0	Card Reader and Intercom to enter, Request to exit (S/E)		
120	S-M	3'-0"		7'-0"	1-3/4"	F	WD	---	HM	E6	C6	---	No	8.0	Monitored		
123A	S-M	3'-0"		7'-0"	1-3/4"	N	WD	---	HM	E6	C6	---	No	24.0	Card Reader to enter (S/E)		
124	S-M	3'-0"		7'-0"	1-3/4"	F	WD	---	HM	E6	C6	---	No	45 min	Yes	8.0	DPS
125	S-M	3'-0"		7'-0"	1-3/4"	F	WD	---	HM	E6	C6	---	No	30.0	Card Reader to enter (S/E)		
127	S-M	2'-6"		7'-0"	1-3/4"	F	WD	---	HM	E6	C6	---	No	10.0	Janitor's closet		
128	S-M	3'-0"		7'-0"	1-3/4"	F	WD	---	HM	E6	C6	---	No	37.0	Card Reader to enter (S/E)		
129	S	3'-0"		7'-0"	1-3/4"	F	WD	---	HM	E6	C6	---	Yes	18.0	Card Reader, Armory with 14 gb. door and frame		
131	S	3'-0"		7'-0"	1-3/4"	F	WD	---	HM	E6	C6	---	Yes	18.0	---		
133	S-M	3'-0"		7'-0"	1-3/4"	F	WD	---	HM	E6	C6	---	No	34.0	Storage lockset for Record Storage		
134	S	3'-0"		7'-0"	1-3/4"	F	WD	---	HM	E6	C6	---	No	31.0	Card Reader to enter (S/E)		
135A	F-M	3'-0"		7'-0"	1-3/4"	FEMA	HM	---	HM	E6	C6	---	No	37.0	FEMA ICC-500 Storm door, frame and hardware		
135B	F-M	3'-0"		7'-0"	1-3/4"	FEMA	HM	---	HM	E6	C6	---	No	36.0	FEMA ICC-500 Storm door, frame and hardware, Card reader to exit		
135C	S	2'-6"		7'-0"	1-3/4"	F	WD	---	HM	E6	C6	---	No	9.0	Storage lockset		
136A	S-M	3'-0"		7'-0"	1-3/4"	F	WD	---	HM	E6	C6	---	No	31.0	Card Reader to enter, Request to exit (S/E)		
136B	S-M	3'-0"		7'-0"	1-3/4"	F	WD	---	HM	E6	C6	---	No	27.0	Card Reader to enter, Request to exit (S/E)		
137	S	3'-0"		7'-0"	1-3/4"	F	WD	---	HM	E6	C6	---	No	6.0	---		
139	S	3'-0"		7'-0"	1-3/4"	F	WD	---	HM	E6	C6	---	No	20.0	---		
140	S	3'-0"		7'-0"	1-3/4"	N	WD	---	HM	E6	C6	---	No	23.0	---		
141	S	3'-0"		7'-0"	1-3/4"	F	WD	---	HM	E6	C6	---	No	13.0	Storage lockset		
142	S-M	3'-0"		7'-0"	1-3/4"	F	WD	---	HM	E6	C6	---	No	13.0	Storage lockset		
144	S-M	3'-0"		7'-0"	1-3/4"	G	WD	TT	HM	E6	C6	---	Yes	28.0	Integral blinds, Request to exit, Card reader to enter (S/E)		
148A	S-M	3'-0"		7'-0"	1-3/4"	F	WD	---	HM	E6	C6	---	No	16.0	Storage lockset		
148B	S-M	2'-6"		7'-0"	1-3/4"	F	WD	---	HM	E6	C6	---	No	17.0	Storage lockset		
149	S-M	3'-0"		7'-0"	1-3/4"	F	WD	---	HM	E6	C6	---	No	25.0	Card Reader to enter (S/E)		
150	S-M	3'-0"		7'-0"	1-3/4"	F	WD	---	HM	E6	C6	---	No	29.0	Card Reader to enter (S/E)		
151	S-M	2'-6"		7'-0"	1-3/4"	F	WD	---	HM	E6	C6	---	No	11.0	Janitor's closet		
152	S-M	3'-0"		7'-0"	1-3/4"	N	WD	TT	HM	E6	C6	---	Yes	15.0	Storage room lockset, Hold-open		
153	S-M	4'-0"		7'-0"	1-3/4"	F	HM	---	HM	---	---	---	No	5.0	Stainless steel frame, Monitored		
155B	n/a	6'-0"		7'-0"	0"	CD	STL	---	STL	D12/AB01	C12/AB01	---	3.0	Interlocked with #155A, Intercom on exterior (S/E)			
156	S-M	3'-0"		7'-0"	1-3/4"	F	HM	---	HM	E6	C6	---	Yes	14.0	Mech room, storage lockset		
157	S-M	3'-0"		7'-0"	1-3/4"	F	HM	---	HM	---	---	---	No	5.0	Monitored, exterior Mech., SS frame		
158	S-M	3'-0"		7'-0"	1-3/4"	F	HM	---	HM	---	---	---	No	5.0	Monitored, exterior Mech., SS frame		
160	S	3'-0"		7'-0"	1-3/4"	F	WD	---	HM	E6	C6	---	No	21.0	Privacy lockset for toilet		
161	S-M	2'-6"		7'-0"	1-3/4"	F	WD	---	HM	E6	C6	---	No	11.0	Janitor's closet		
163	S-M	3'-0"		7'-0"	1-3/4"	F	HM	---	HM	---	---	---	No	5.0	Monitored, exterior Mech., SS frame		
164	S-M	3'-0"		7'-0"	1-3/4"	F	HM	---	HM	---	---	---	No	5.0	Monitored, exterior Mech., SS frame		
167	S-M	2'-6"		7'-0"	1-3/4"	F	WD	---	HM	E6	C6	---	No	12.0	---		
169	S-M	3'-0"		7'-0"	1-3/4"	F	HM	---	HM	---	---	---	No	5.0	Monitored, exterior Mech., SS frame		
CB1	S-M	2'-8"		7'-0"	1-3/4"	F	HM	---	HM	E6	C6	---	No	11.0	14 Ga. HM Frame		
CD1	S-M	2'-8"		7'-0"	1-3/4"	F	HM	---	HM	E6	C6	---	No	11.0	14 Ga. HM Frame		
CD1	S-M	2'-8"		7'-0"	1-3/4"	F	HM	---	HM	E6	C6	---	No	11.0	14 Ga. HM Frame		
CE1	S-M	2'-8"		7'-0"	1-3/4"	F	HM	---	HM	E6	C6	---	No	11.0	14 Ga. HM Frame		
CH1	S	2'-0"		7'-0"	1-3/4"	F	HM	---	HM	E6	C6	---	No	11.0	14 Ga. HM Frame		
DV1	S	3'-0"		7'-0"	1-3/4"	FG-CW	ALUM	TT	ALUM	H12	G12	C10	---	2.0	Card Reader and Intercom to enter (S/E)		
EX5	n/a	8'-0"		7'-0"	0"	CD	STL	---	STL	D12/AB01	C12/AB01	---	3.0	Overhead door at Mezz. level			
SP4C	n/a	12'-0"		12'-0"	2"	SO	STL	---	STL	F10	E10	C10	---	4.0	All 7 saltport doors interlocked w/only one door open at a time		
SP4D	n/a	12'-0"		12'-0"	2"	SO	STL	---	STL	F10	E10	C10	---	4.0	All 7 saltport doors interlocked w/only one door open at a time		
SP4E	n/a	12'-0"		12'-0"	2"	SO	STL	---	STL	F10	E10	C10	---	4.0	All 7 saltport doors interlocked w/only one door open at a time		
SP4F	n/a	12'-0"		12'-0"	2"	SO	STL	---	STL	F10	E10	C10	---	4.0	All 7 saltport doors interlocked w/only one door open at a time		
1169																	
02 - MEZZANINE																	
CB2	S-M	2'-8"		7'-0"	1-3/4"	F	HM	---	HM	E6	C6	---	No	11.0	14 Ga. HM Frame		
CD2	S-M	2'-8"		7'-0"	1-3/4"	F	HM	---	HM	E6	C6	---	No	11.0	14 Ga. HM Frame		
CE2	S-M	2'-8"		7'-0"	1-3/4"	F	HM	---	HM	E6	C6	---	No	11.0	14 Ga. HM Frame		
CE2	S-M	2'-8"		7'-0"	1-3/4"	F	HM	---	HM	E6	C6	---	No	11.0	14 Ga. HM Frame		

DOOR ABBREVIATIONS LEGEND

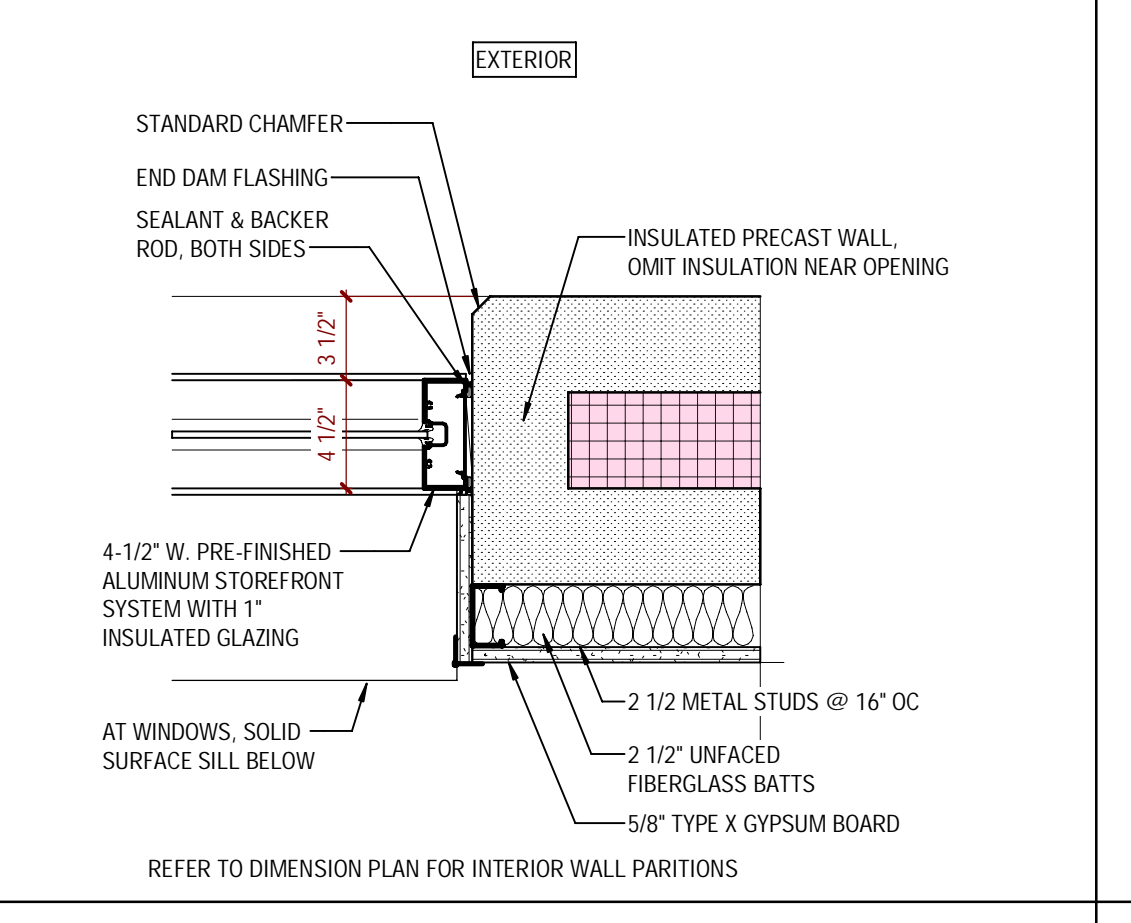
ALUM	ALUMINUM	F	FLUSH	N	NARROW LITE	RVS	RIGID VINYL SHEET
ACWV	ACROBYN	FG	FULL GLASS	P	POCKET	SK	SMOKE & BRIGHT KIT
BL-#	BORROWED LITE	FRP	FIBERGLASS REINFORCED PANELS	PL	POSITIVE LATCHING	SL-#	SLIDE LITE
CG	COILING GRILL OVERHEAD DOOR	G	HALF GLASS	PLK	PAINT LITE KIT	SO	SECTIONAL OVERHEAD DOOR
CL	COILING OVERHEAD DOOR	HM	HOLLOW METAL	PP	POSITIVE PRESSURE	T	TEMPERED
CD	CASED OPENING	IB	INTEGRAL BLINDS	PR	PAIR	V	VISION GLASS
CO	DUTCH DOOR	LEDM	LEAD-LINED DOOR & FRAME	S	SINGLE	WD	WOOD
DE	DOUBLE EGRESS	M	MIRRORED				

DOOR ABBREVIATIONS LEGEND (continued)

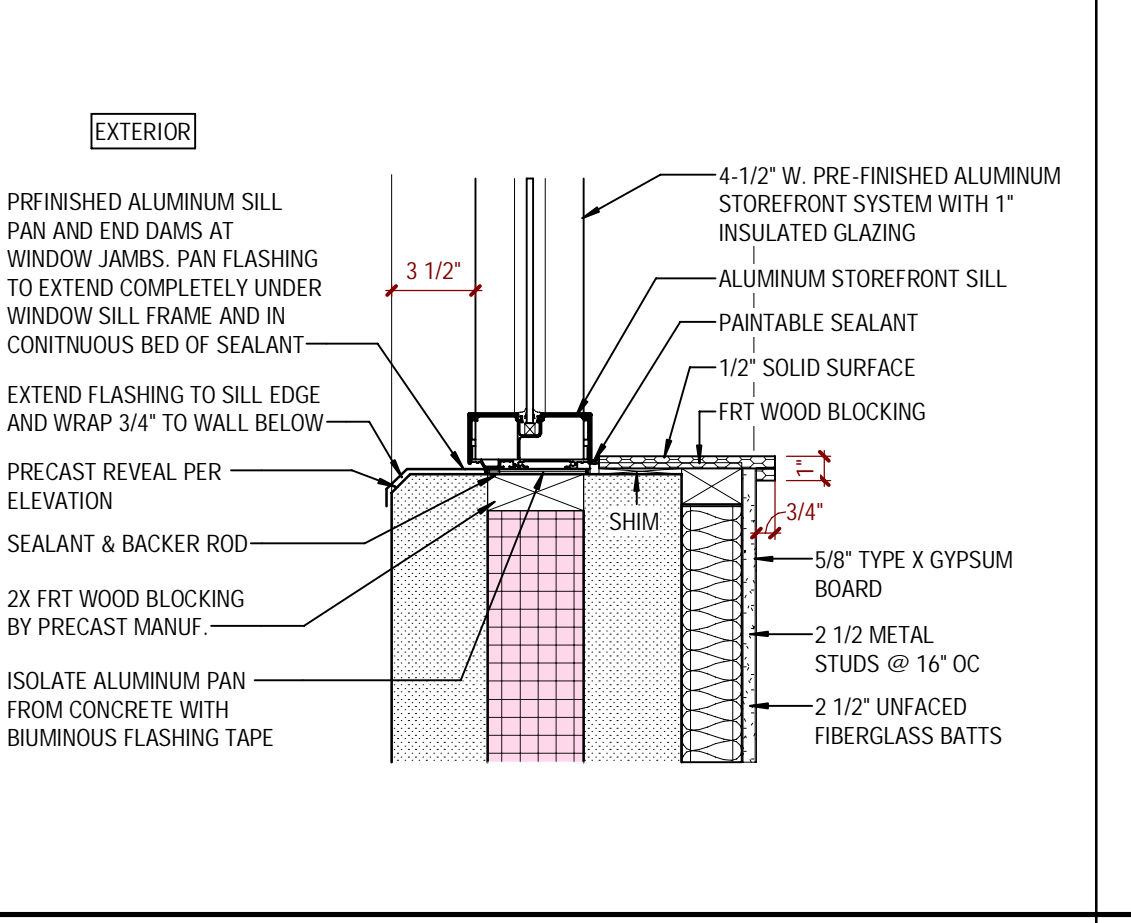
ALUM	ALUMINUM	F	FLUSH	N	NARROW LITE	RVS	RIGID VINYL SHEET
ACWV	ACROBYN	FG	FULL GLASS	P	POCKET	SK	SMOKE & BRIGHT KIT
BL-#	BORROWED LITE	FRP	FIBERGLASS REINFORCED PANELS	PL	POSITIVE LATCHING	SL-#	SLIDE LITE
CG	COILING GRILL OVERHEAD DOOR	G	HALF GLASS	PLK	PAINT LITE KIT	SO	SECTIONAL OVERHEAD DOOR
CL	COILING OVERHEAD DOOR	HM	HOLLOW METAL	PP	POSITIVE PRESSURE	T	TEMPERED
CD	CASED OPENING	IB	INTEGRAL BLINDS	PR	PAIR	V	VISION GLASS
CO	DUTCH DOOR	LEDM	LEAD-LINED DOOR & FRAME	S	SINGLE	WD	WOOD
DE	DOUBLE EGRESS	M	MIRRORED				



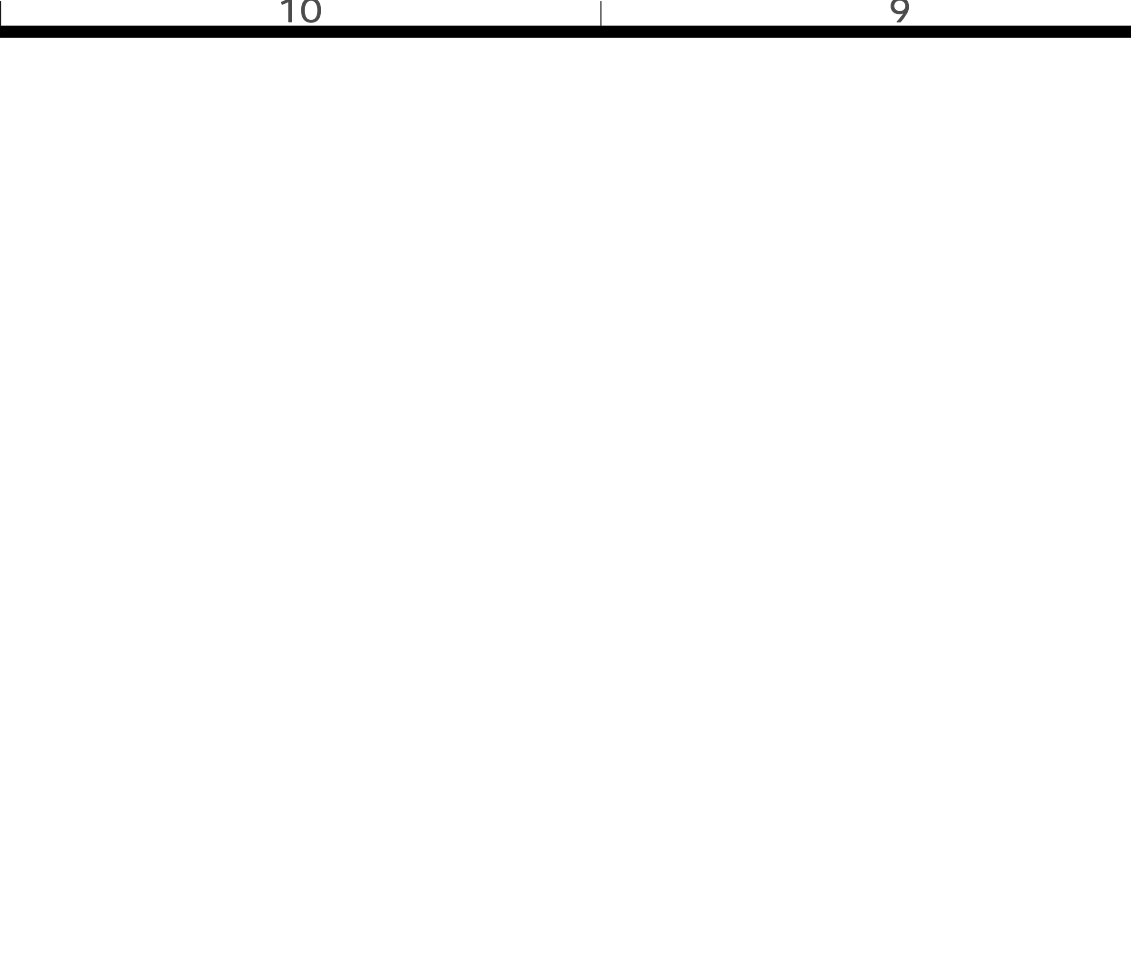
H12 STOREFRONT HEAD @ PRECAST  
1 1/2" x 1'-0" (A10/A430)



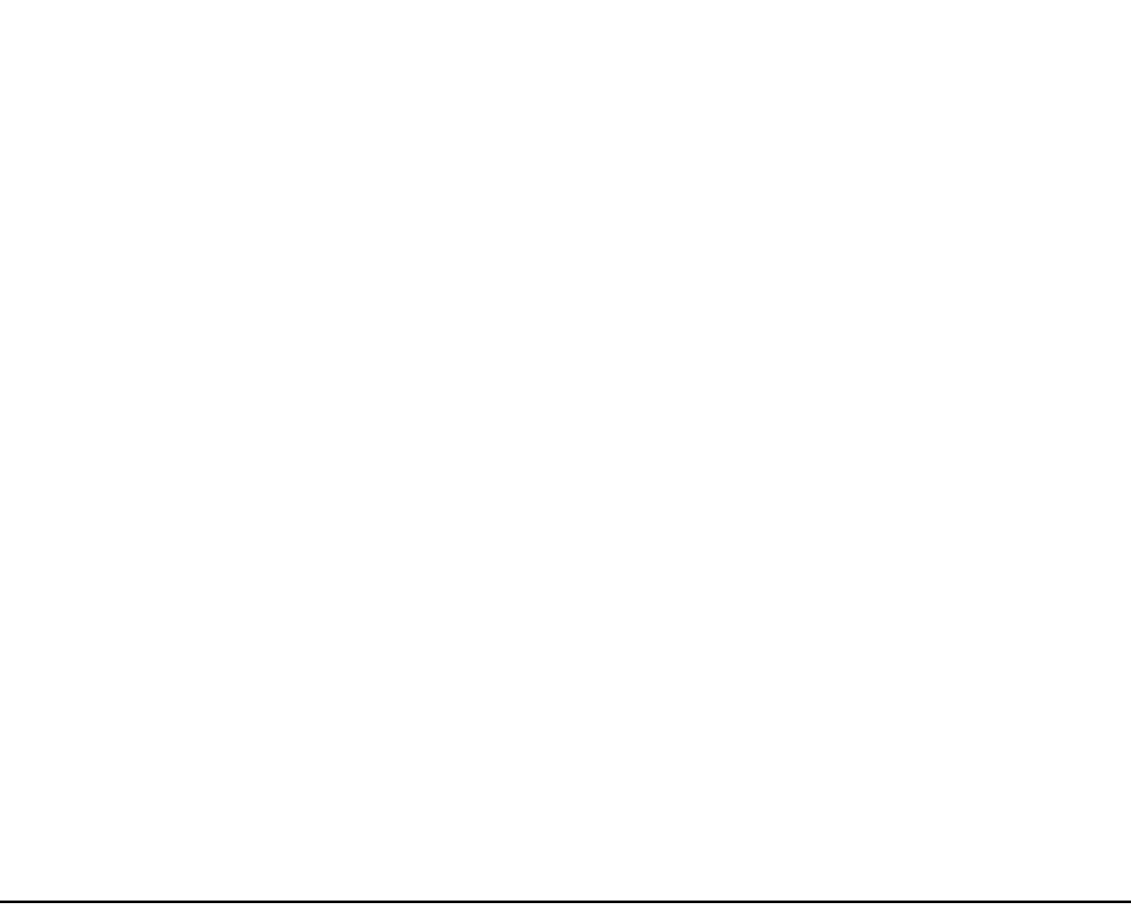
G12 STOREFRONT JAMB @ PRECAST  
1 1/2" x 1'-0" (A10/A430)



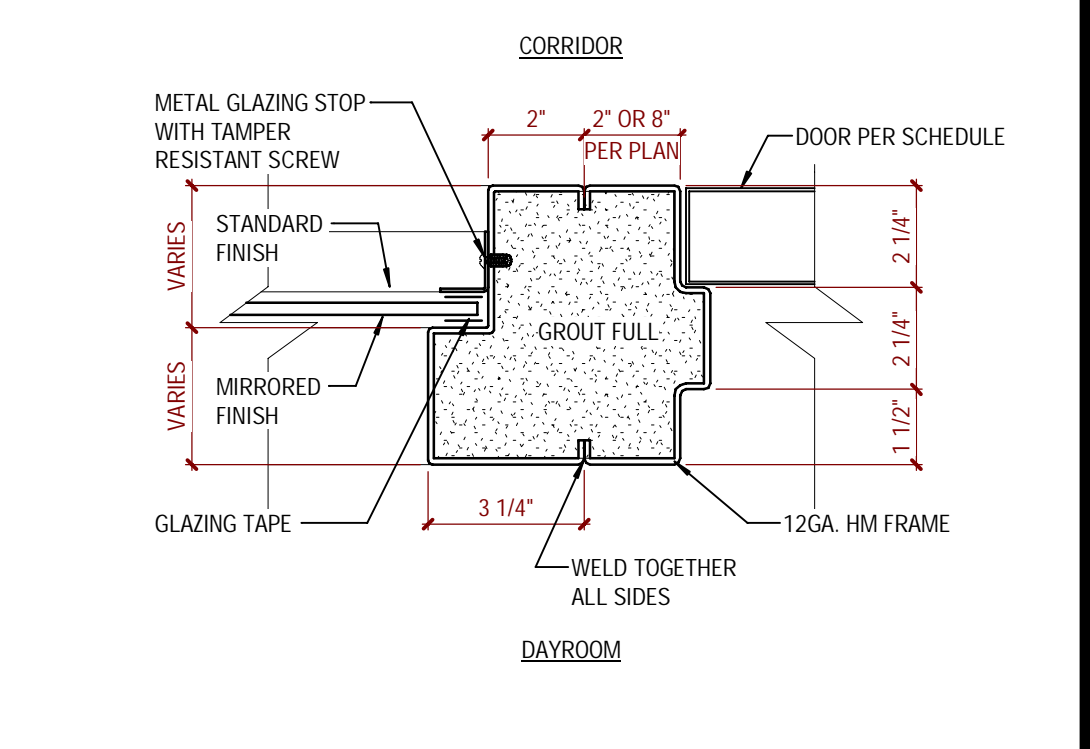
E12 STOREFRONT SILL @ PRECAST  
1 1/2" x 1'-0" (A10/A430)



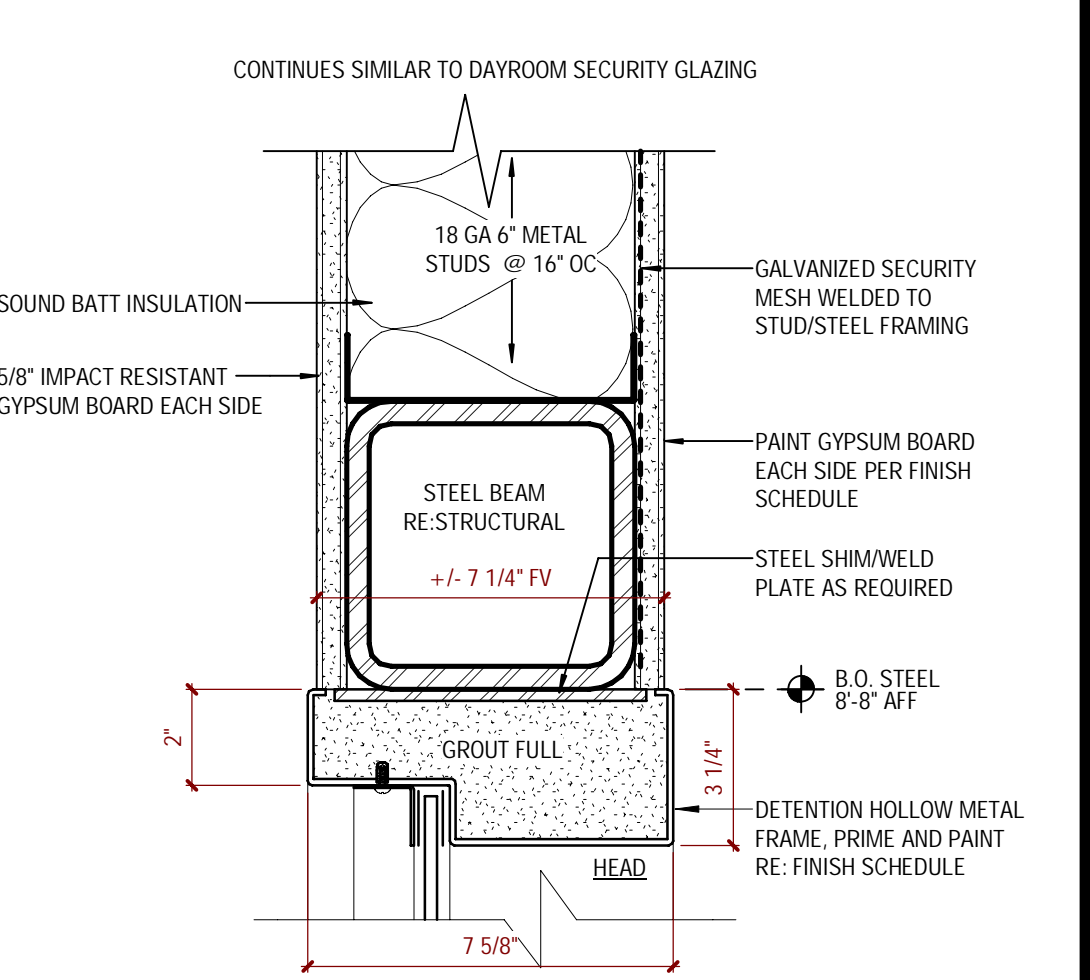
H8 HM DOOR FRAME HEAD @ PRECAST  
3" x 1'-0"



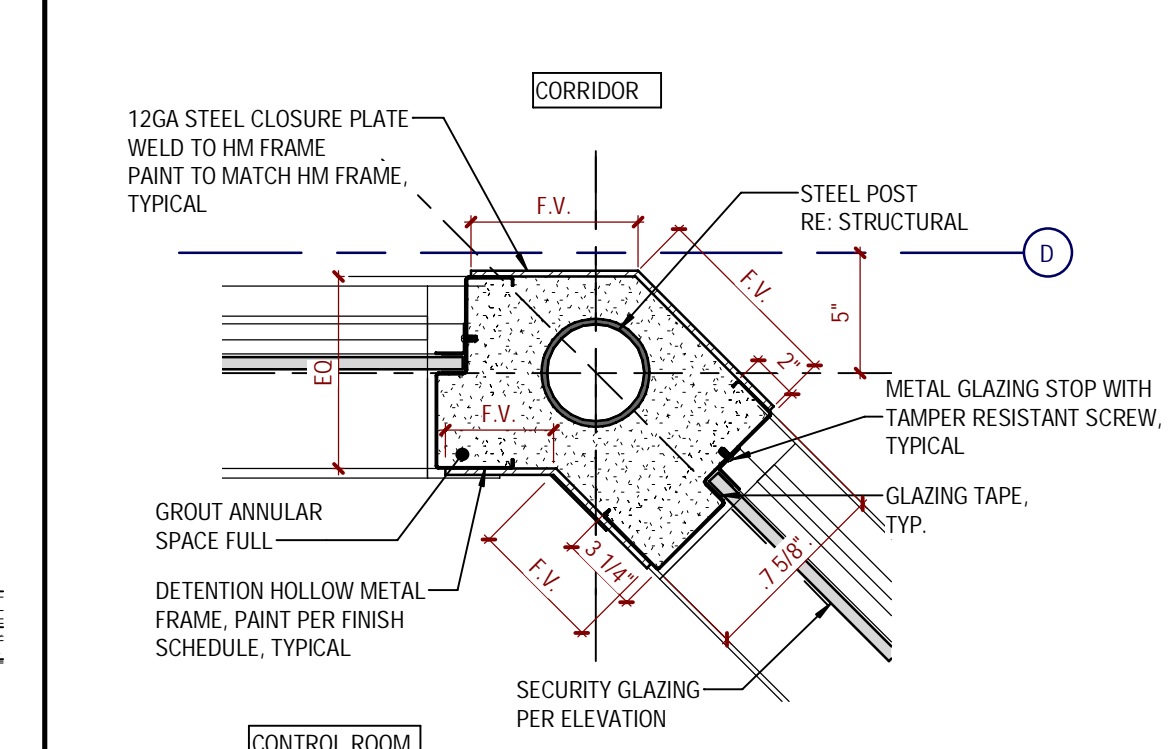




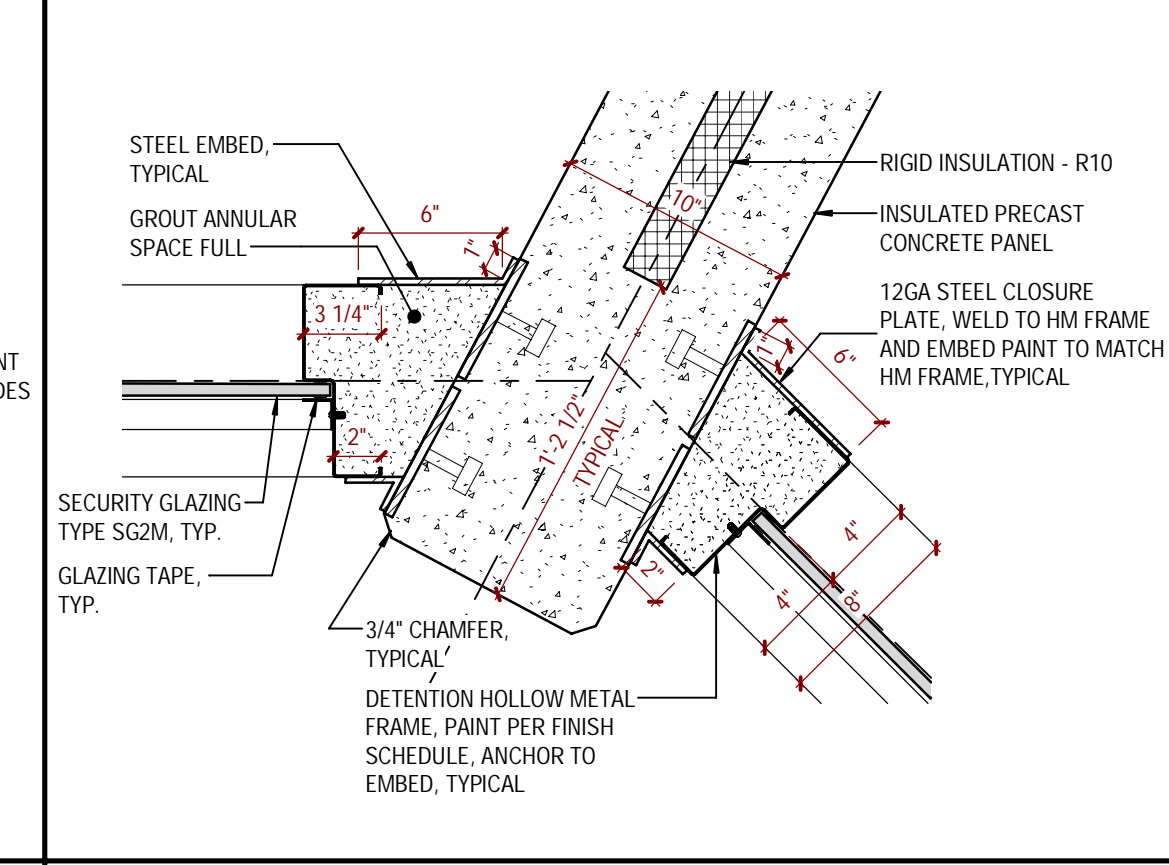
**H2 DAYROOM DOOR FRAME**  
3'-0" (A1A850)



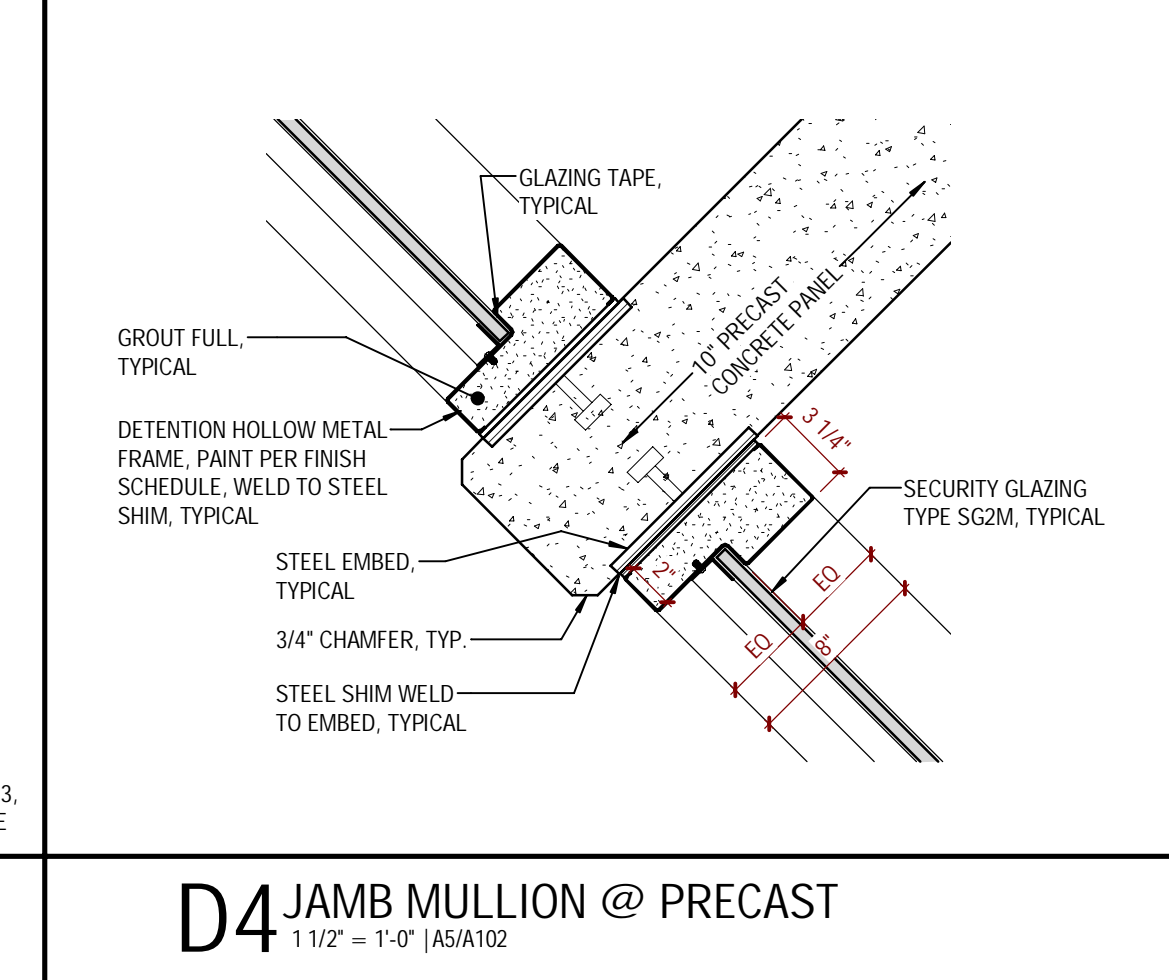
**D2 CONTROL ROOM SECURITY GLAZING**  
3'-0" (C2A850)



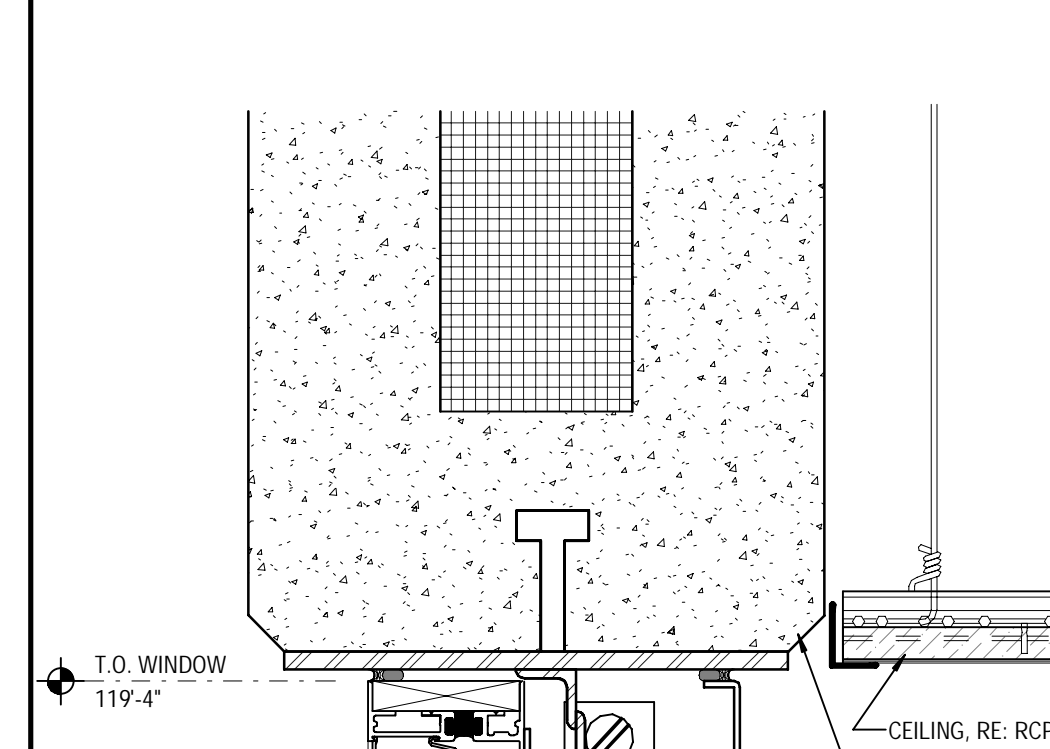
**H4 CORNER MULLION - CONTROL ROOM**  
1'-0" (A10A130)



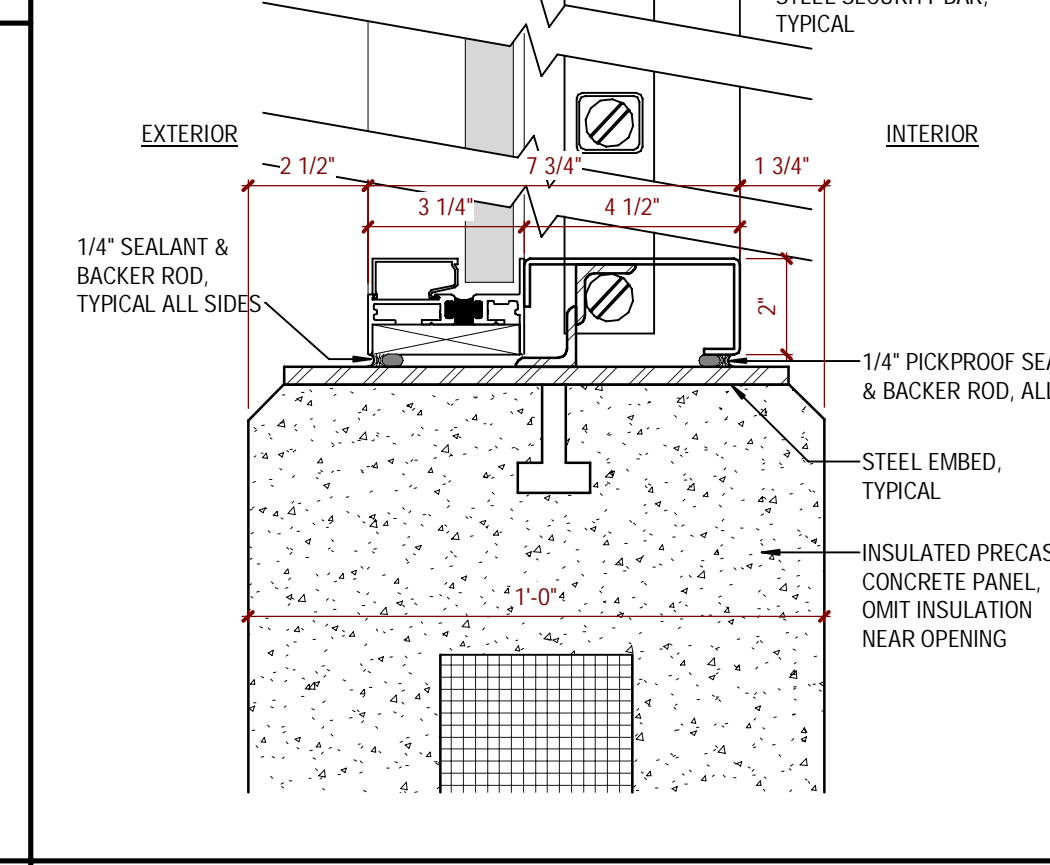
**F4 JAMB MULLION - ANGLED @ PRECAST**  
1'-0" (A5A102)



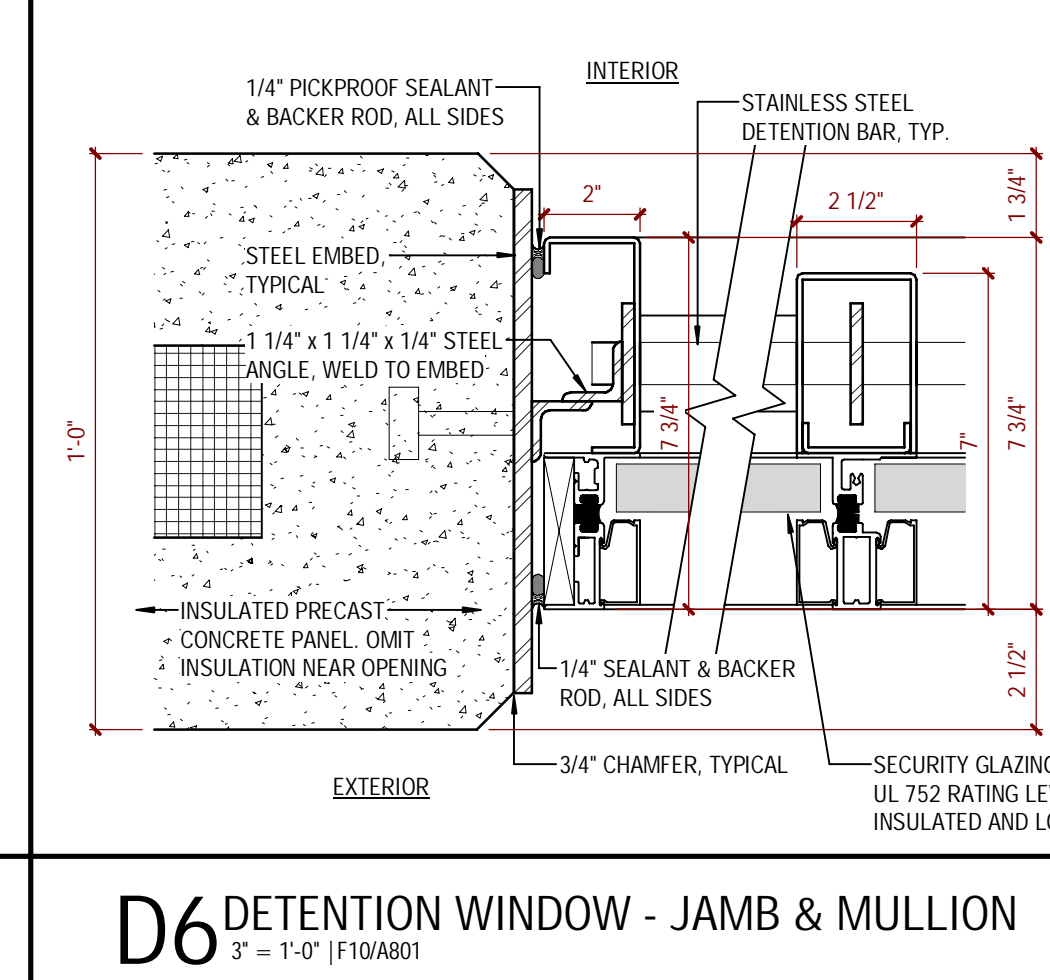
**D4 JAMB MULLION @ PRECAST**  
1'-0" (A5A102)



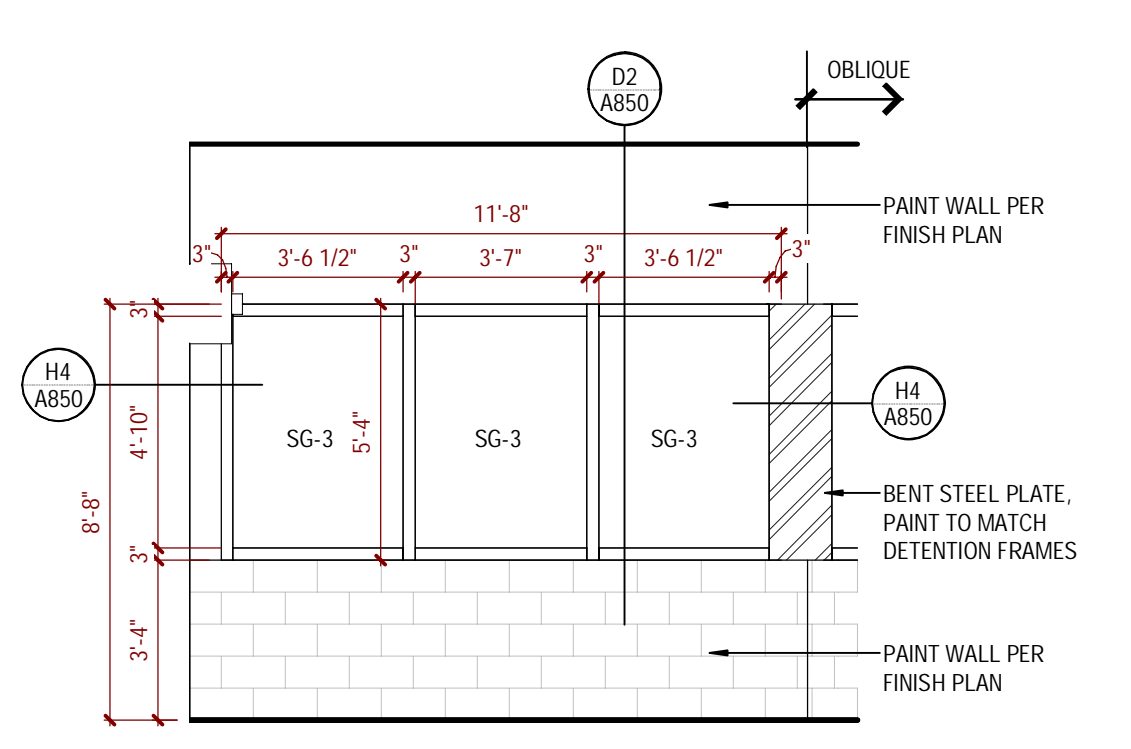
**F6 DETENTION WINDOW - HEAD & SILL**  
3'-0" (F10A801)



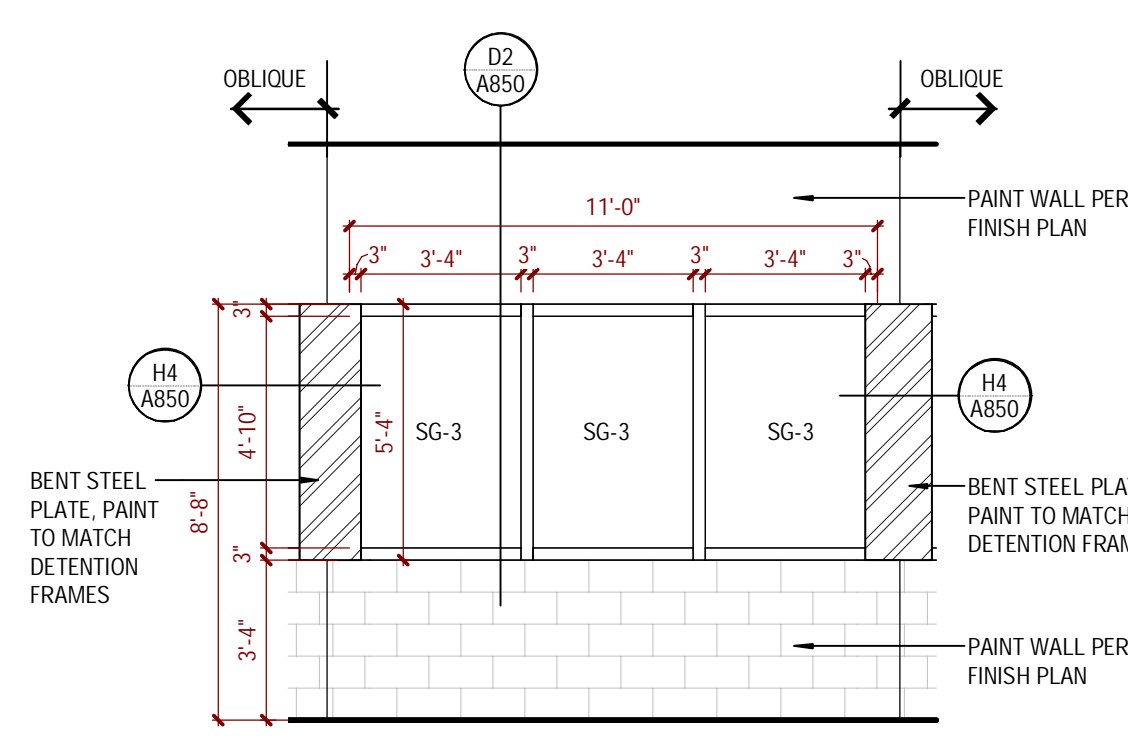
**D6 DETENTION WINDOW - JAMB & MULLION**  
3'-0" (F10A801)



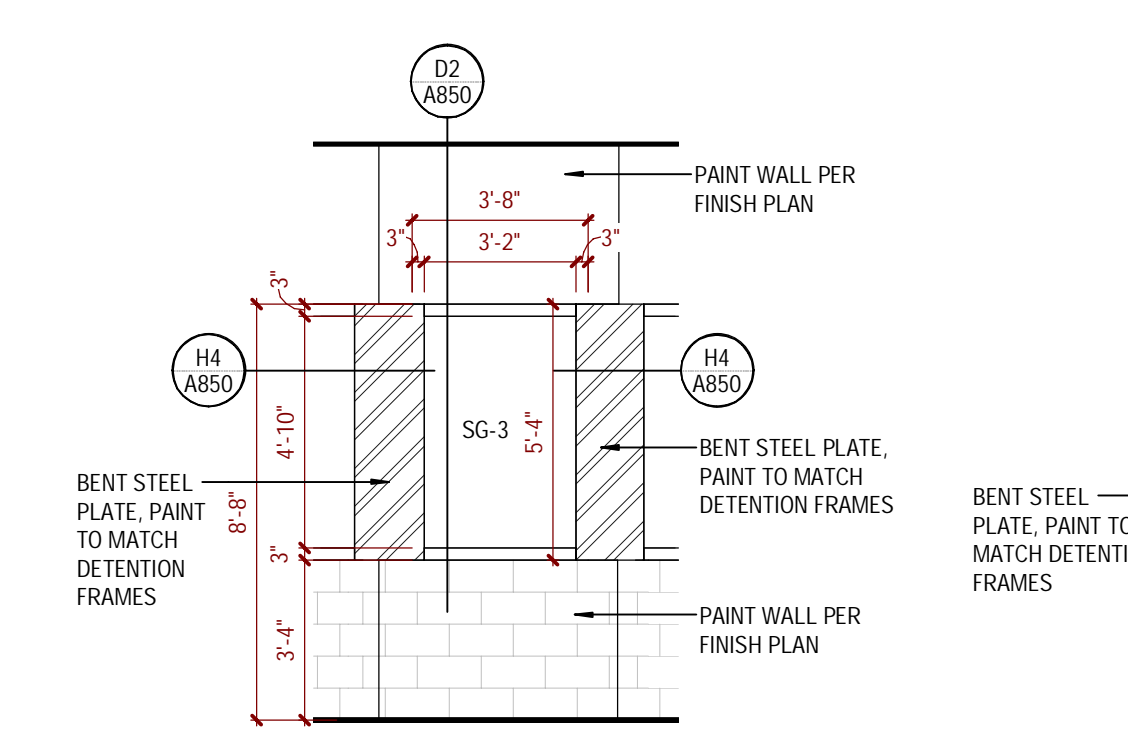
**D6 DETENTION WINDOW - JAMB & MULLION**  
3'-0" (F10A801)



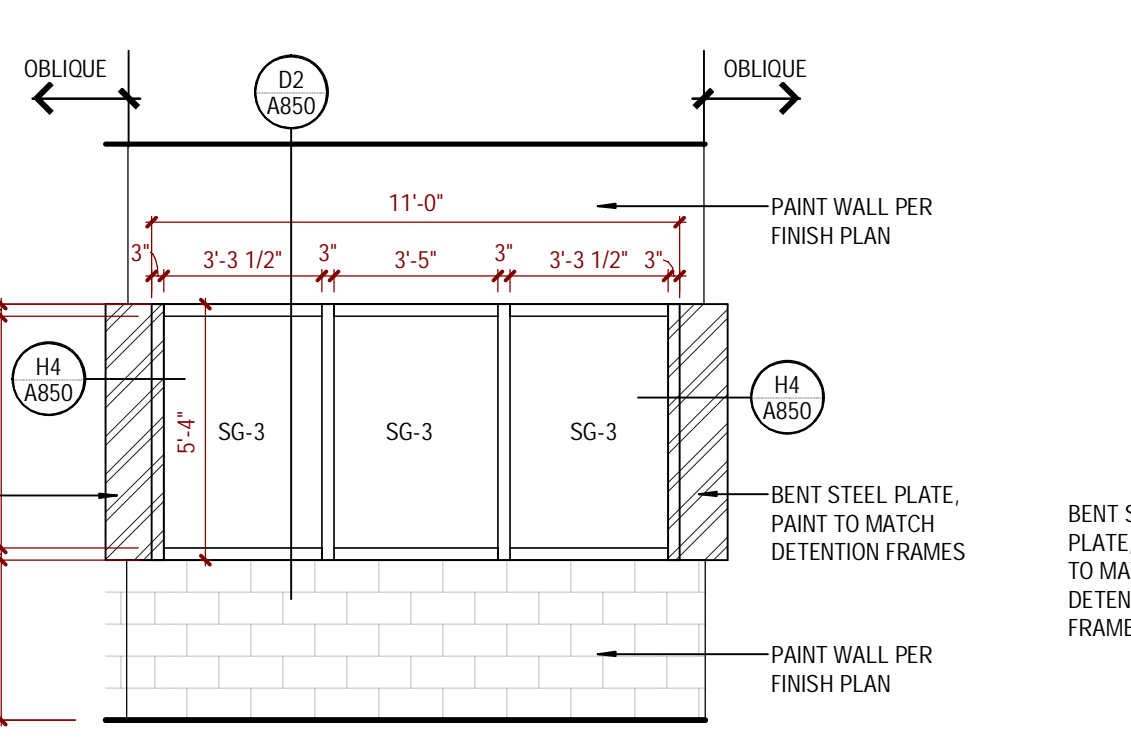
**C10 SECURITY GLAZING - CONTROL - SW**  
1'-0" (A10A130)



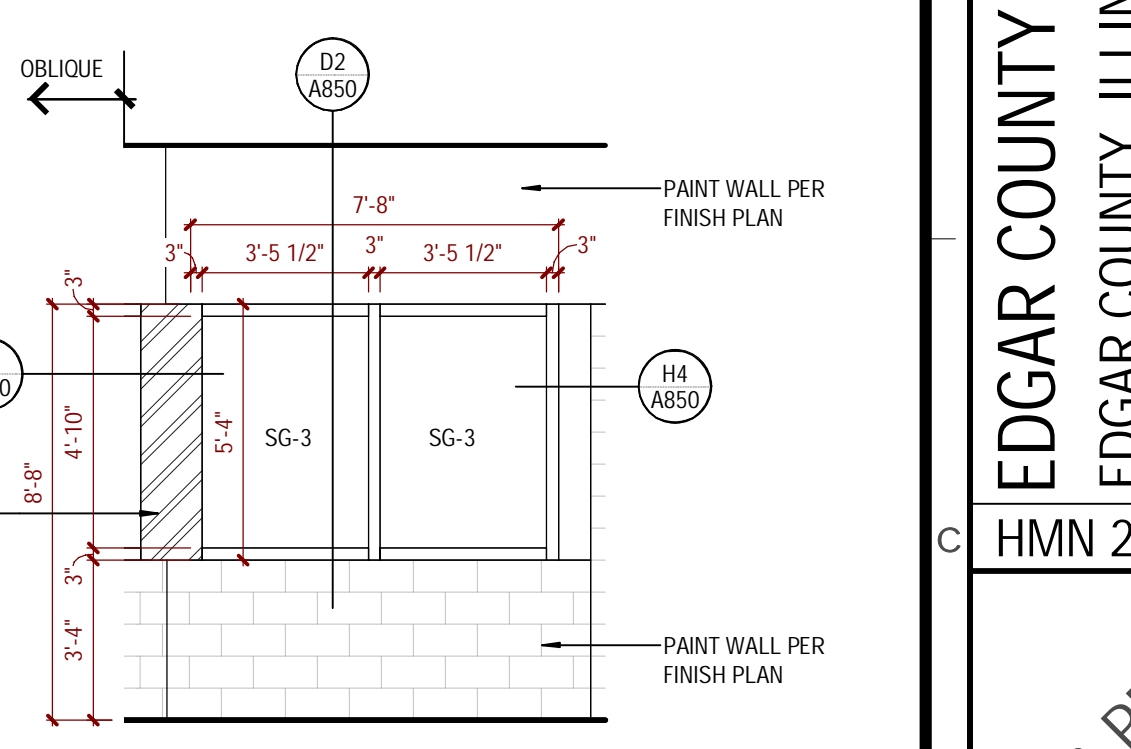
**C8 SECURITY GLAZING - CONTROL - SE**  
1'-0" (A10A130)



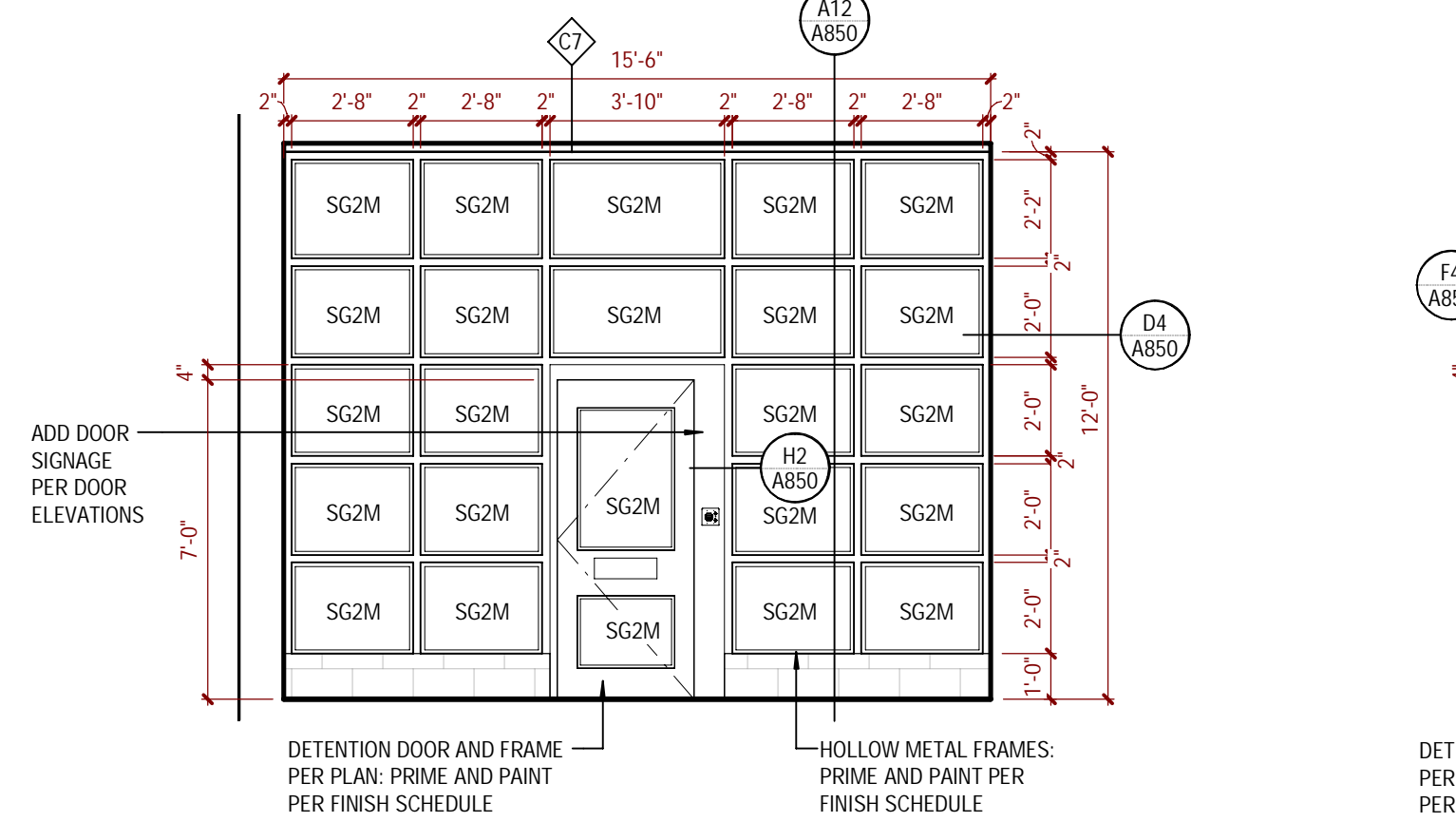
**C6 SECURITY GLAZING - CONTROL - E**  
1'-0" (A10A130)



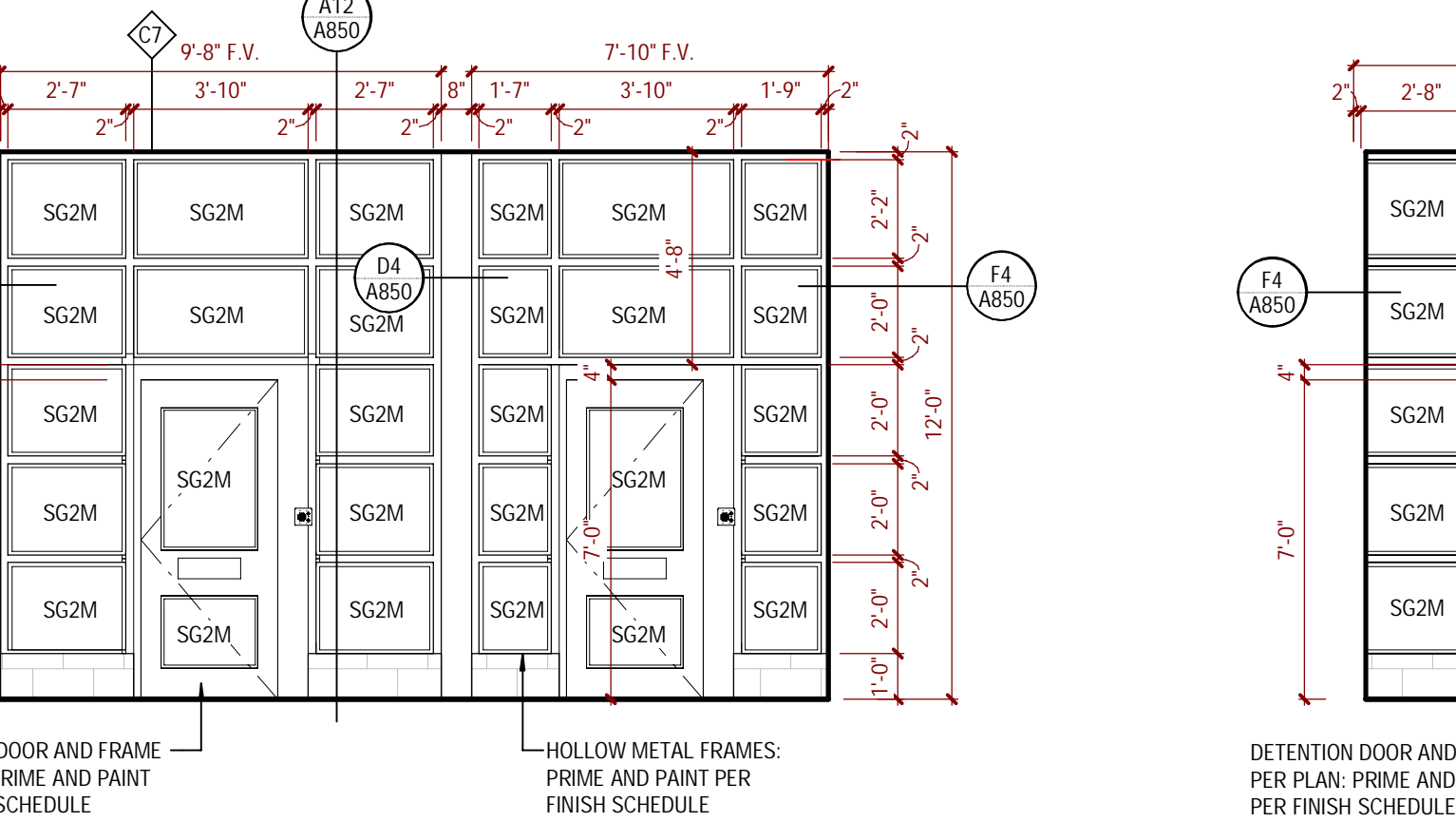
**C4 SECURITY GLAZING - CONTROL - NE**  
1'-0" (A10A130)



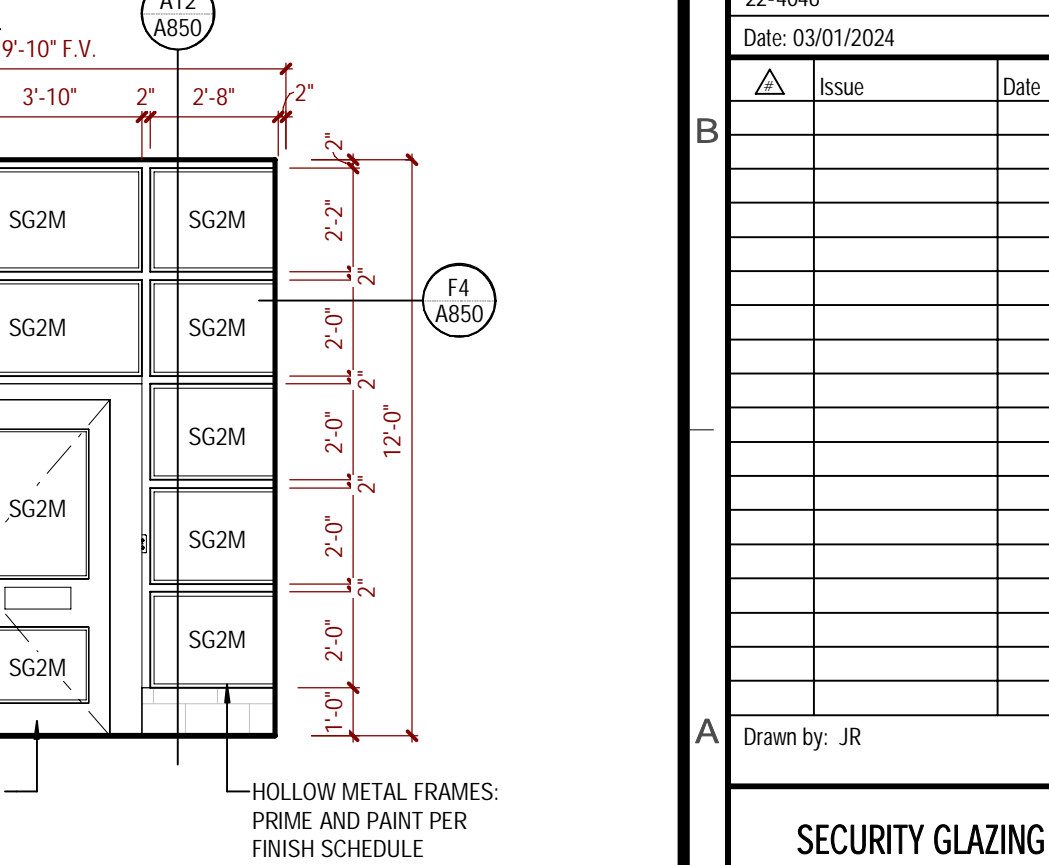
**C2 SECURITY GLAZING - CONTROL - N**  
1'-0" (A10A130)



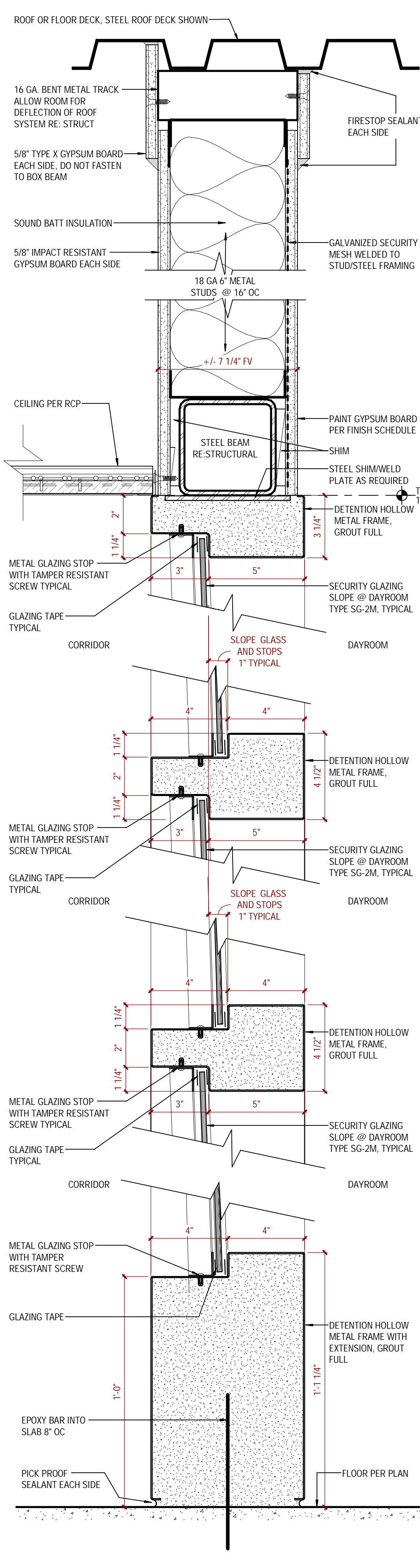
**A7 SECURITY GLAZING - DAYROOM A & F**  
1'-0" (A7A131)



**A4 SECURITY GLAZING - DAYROOM B, C, D & E**  
1'-0" (A7A131)



**A2 SECURITY GLAZING - RECREATION**  
1'-0" (A5A102)



**A12 DAYROOM SECURITY GLAZING**  
3'-0" (A2A850)

REFER TO SHEET A100 FOR SECURITY CONSTRUCTION NOTES







DESIGN CRITERIA
1. BUILDING CODES:
A. IBC 2021
B. ASCE 7-16
2. DESIGN LOADS:
A. Risk Category IV
B. Dead Loads
a. Roof = 10 psf
b. Colateral = 10 psf
c. Concrete Slab on Metal Deck = 95 psf
d. Floor Slab on 15 psf
e. See roof framing plan for additional loads & locations. (RTU, Point Loads, etc.)
C. Live Loads
a. Roof = 20 psf
b. Floor = 40 psf
c. Catwalks = 40 psf
d. Stairs = 40 psf
e. Stairs = 100 psf
D. Roof Snow Load
a. Ground Snow Load, P<sub>s</sub> = 20 psf
b. Flat Roof Snow Load, P<sub>f</sub> = 16.8 psf
c. Minimum Snow Load, P<sub>s</sub> = 24 psf
d. Snow Load Importance Factor, I<sub>s</sub> = 1.0
e. Snow Exposure Factor, C<sub>e</sub> = 1.0
f. Roof Thermal Factor, C<sub>t</sub> = 1.0
g. Drifting: (See Special Loading Diagram for Drift Loads)
E. Wind Loading
a. Basic Wind Speed, V<sub>b</sub> = 120 mph
b. Exposure Category C
c. Internal Pressure Coefficient, GC<sub>p</sub> = ±0.18
d. Components and Cladding Design ASCE 7-16
e. Net Uplift for Joist Design (0.00+0.00) = 25 psf
F. Seismic Loading
a. Importance Factor, I<sub>e</sub> = 1.5
b. Response Coefficient, R<sub>c</sub> = 4
c. System Overstrength Factor, O<sub>s</sub> = 2.5
d. Seismic Design Coefficient, C<sub>d</sub> = 0.078 precast
e. Seismic Design Coefficient, C<sub>d</sub> = 0.09 masonry
f. Seismic Design Category C
g. Design Coefficients and Factors for Seismic Force-Resisting Systems
1. Restraint System - Bearing Wall System, Intermediate Precast Shear Walls
2. Response Coefficient, R<sub>c</sub> = 4
3. Deflection Amplification Factor, C<sub>d</sub> = 2.5
4. System Overstrength Factor, O<sub>s</sub> = 2.5
5. Bearing Wall System Intermediate Reinforced Masonry Shear Walls
6. Response Coefficient, R<sub>c</sub> = 3.5
7. Deflection Amplification Factor, C<sub>d</sub> = 2.5
8. System Overstrength Factor, O<sub>s</sub> = 2.5
h. Component Design per ASCE 7-16
i. Seismic Base Shear = W x C<sub>s</sub>

DESIGN LOADS - STORM SHELTER ONLY:
1. Standard: ICC 500-2020
2. Type of shelter: Community Tornado Shelter
3. Location of shelter shall not be constructed within an area susceptible to flooding in accordance with chapter 4 of ICC 500-2020.
4. Risk Category IV
a. Shelter Slab Live Load = 100 psf
b. Wind Loading - Directional Procedure (ICC 500 Tornado AWL)
A. Wind design conforms to the provisions of ICC 500-2020
B. Basic Wind Speed, V<sub>b</sub> = 250 mph
C. Exposure Category C
D. Importance Factor, I<sub>w</sub> = 1.00
E. Directional Factor, K<sub>d</sub> = 1.0
F. Topographic Factor, Z<sub>t</sub> = 1.0
G. Gust-effect Factor, G<sub>f</sub> = 0.85
H. Internal Pressure Coefficient, GC<sub>p1</sub> = ±0.55
I. Main Wind Force Resisting System (Ultimate Wind Load)
a. Walls
1. 150 psf WW + LW
2. 170 psf WW only
b. Roof Uplift = 180 psf
c. Roof Downward = 40 psf
J. Components and Cladding (Ultimate Wind Load)
a. a = 3'0"
b. Walls (200 SQ FT) = 200 psf
c. Doors (20 SQ FT) = 240 psf
d. Windows (10 SQ FT) = 250 psf
e. Roof Uplift:
1. Beam (100 SQ FT) = 320 psf
2. Connections (10 SQ FT) = 390 psf
f. Messle Size and Spacing:
1. Vertical Surfaces - 15 lb 2x4 at 100 mph
Horizontal Surface - 15 lb 2x4 at 67 mph
5. Testing Requirements - Low wind pressures, low wind impact protective systems, and attachments to the structure shall be tested by the manufacturer or contractor according to ICC-500-20 chapter 8 for wind pressures and missile impact. All gaps exceeding 3/8" shall be protected by a tested assembly. All penetrations and openings exceeding 2 1/2" diameter or an area of 3.5 square inches shall be protected by a tested impact protective system assembly. The contractor is responsible for costs associated with the testing.
6. Penetrations and openings in Storm Shelter
A. All materials placed in storm shelter penetrations and openings shall be tested according to the note above.
B. The contractor shall submit a shop drawing showing all penetrations and openings in the walls and roof of the storm shelter. Penetrations and openings that are not shown on the shop drawing are not permitted in the storm shelter.
C. Submit shop drawings documenting showing that all doors, windows, louvers, impact protective systems, and their attachments to the structure will meet the pressure and missile impact test requirements identified in ICC 500-2020 chapters 3 and 8.
D. The contractor shall avoid penetrations and openings in the walls and roof of the storm shelter as much as possible.
E. All gaps of 3/8" or less shall be sealed with joint material in accordance with TUMS 602 or ASTM D320.
F. Penetrations in the storm shelter envelope shall not be located closer than 8 inches on center. Penetrations shall allow all reinforcing steel to pass between the penetrations without damage and with the specified clear cover.

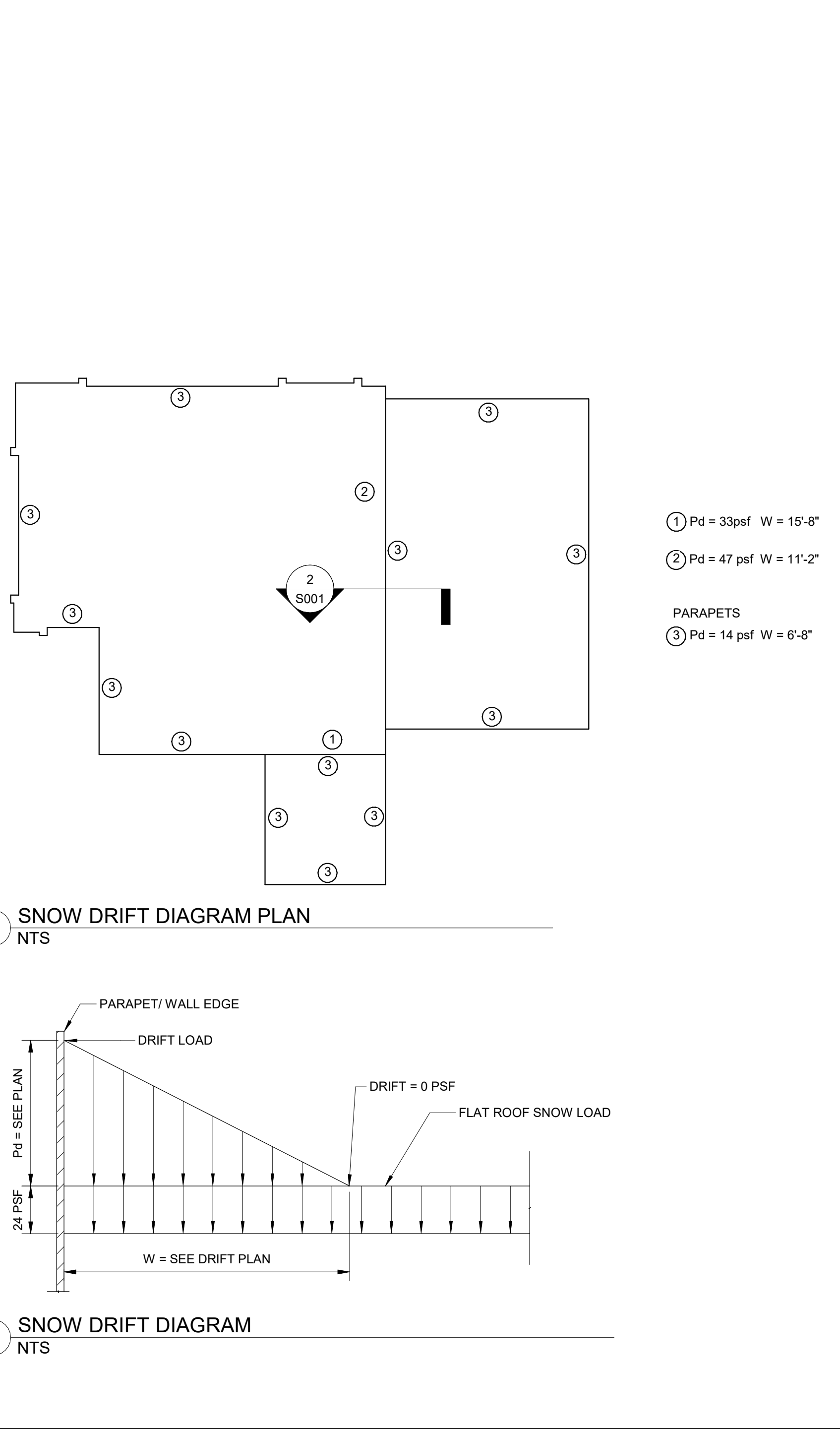
GENERAL
1. The structure is designed to be self-supporting and stable after the building is fully completed. It is solely the contractor's responsibility to determine erection procedure and sequence and ensure the safety of the construction personnel, public, building and its components parts, and adjacent buildings and properties. This includes the addition of whatever temporary or permanent shoring and bracing is required for the structure during construction, including the addition of bracing for new construction, adjacent buildings, so that the structure is braced for wind, seismic, gravity, construction loads, etc. and that no horizontal or vertical settlement or any damage occurs to the adjacent existing structure. Temporary supports shall be maintained in place until permanent supports and/or shoring and bracing are installed.
2. Fall protection support from perimeter columns or walls shall be provided in accordance with OSHA requirements as required. Such material shall remain the contractor's property after the completion of the project.
3. It is the contractor's responsibility to enforce all applicable safety codes and regulations during all phases of construction.
4. The contractor shall perform all construction for the project in a manner and sequence that are based on accepted industry standards that recognize the interrelation of the components that comprise the structure, without causing distress, unanticipated movements or irregular load paths as a result of the construction means and methods employed.
5. Construction loads shall not exceed design live loads. The contractor shall be responsible for all design required to support construction equipment used in constructing this project. Shoring and restoring is the responsibility of the contractor.
6. Principal openings through the framing are shown on these drawings. The general contractor shall examine the structural and mechanical drawings for the required openings and shall verify size and location of all openings with the mechanical contractor. Providing all openings required by the mechanical, electrical, plumbing, or other trades shall be part of the general contract, whether or not shown in the structural drawings. Any deviation from the openings shown on the structural drawings shall be brought to the engineer's attention for review.
7. All contractors are required to examine the drawings and specifications carefully, visit the site and fully inform themselves as to all existing conditions and limitations, prior to agreeing to perform the work. Failure to visit the site and familiarize themselves with the existing conditions and limitations will in no way relieve the contractor from furnishing any materials or performing any work in accordance with drawings and specifications without additional cost to the owner.
8. Contractor shall verify all dimensions and conditions at the job site before commencing work and shall report any discrepancies to the engineer.
9. Omissions or conflicts between various elements of the drawings, notes, details and specifications shall be brought to the attention of the engineer and resolved before proceeding with the work.
10. Details labeled "Typical Details" on drawings apply to situations occurring on the project that are the same or similar to those specifically detailed. Such details apply whether or not details are referenced at each location. Notify engineer of clarification regarding applicability of "Typical Details".
11. Work these drawings with architectural, civil, mechanical, and electrical drawings.
12. Do not scale drawings.
13. Should any of the general notes conflict with any details or instructions on plans, the strictest provision shall govern.
14. Coordinate masonry and concrete work with the plumbing. The plumbing shall not be placed inside of exterior reinforced masonry walls where it could freeze. Route plumbing around masonry cells and knock-out bond beams with reinforcing steel. Do not place plumbing in masonry cells with reinforcing steel. Maintain the continuity of the masonry horizontal joint reinforcing. Do not route plumbing vertically through footings. The plumbing engineer needs to be informed when there may be conditions like those described above.
15. Shop drawings and submittals:
A. These drawings shall be checked and coordinated with other materials and contracts by the general contractor and shop drawings and submittals shall bear the contractor's review stamp with the checker's initials before being submitted to the architect for approval.
B. Where the fabricator has been authorized to use the architect and engineer's drawings as erection drawings, the fabricator must remove all title blocks, professional seals and any other reference to the architect and engineer from that erection drawing. The fabricator's name and title shall be placed on the erection drawing.
SPECIALLY STRUCTURAL INSPECTIONS AND TESTING
1. Owner will engage a qualified testing and inspecting agency to perform field special structural inspections and testing in accordance with the applicable International Building Code and to submit reports.
2. The Contractor shall provide a minimum of 48 hrs. notification to the Special Inspector prior to needing an inspection. The Contractor shall provide access to the work so the Special Inspections can be completed. The Contractor shall verify all Special Inspections have been completed and discrepancies corrected prior to covering the work.
3. See specifications and list of elements below for a summary of the elements of construction that shall require verification or special inspection. The tables shall be considered a guide, and the contractor and inspector shall refer to the IBC for complete requirements, qualifications, exceptions and details.
4. Special inspections noted as "Continuous" requires the presence of a qualified inspector in the vicinity of the work being performed for 100% of that work. Special inspections noted as "Periodic" requires part-time observation of the work being performed and observation of the final condition of the work before the Special Inspector notes that "N/A" are Not Applicable for this project.
5. Special inspection and testing reports shall be furnished to owner, structural engineer, and contractor. Special Inspector shall inform engineer of record immediately of any items found in non-compliance with construction documents or approved submittals.
6. The special inspector shall submit a final report stating that the structural work was, to the best of the special inspector's knowledge, performed in accordance with the construction documents.
7. Special inspections shall conform to Chapter 17 of the International Building Code, IBC, 2021. Special inspections include:
A. Steel Construction - 1705.2
B. Concrete Construction - 1705.3
C. Masonry Construction - 1705.4 (Level 3)
D. Soils - 1705.6
E. Wind Resistance Construction (Storm Shelter) - 1705.12
F. Post installed anchors - Products ICC-ES reports
G. Stone column ground improvement - 1705.6 and Product's ICC-ES reports
H. Seismic Resistance - 1705.13
I. Seismic Testing - 1705.14.2
CAST-IN-PLACE CONCRETE
1. All concrete construction shall conform to ACI 301, "Specification for Structural Concrete" and ACI 302, "Guide for Concrete Floor and Slab Construction", ACI 305 "Specification for Hot Weather Concrete" and ACI 306, "Standard Specification for Cold Weather Concrete", unless noted otherwise for the year referenced in the building code notes.
2. All detailing, fabrication and placing of reinforcing bars, unless otherwise noted, shall conform to ACI 318, "Building Code Requirements for Structural Concrete", ACI 117, "Specification for Tolerances for Concrete Construction and Materials", and the latest ACI detailing manual.
3. Concrete Types:
A. Interior Concrete:
a. Min. Cementitious Content = 564 lb/cu yd
b. Max Water-Cement Ratio = 0.45
c. Specified 28-day Compressive Strength, F<sub>c</sub> = 4000 psi
d. Specified Slump Range for Placement 4" max. w/o W.R. (8" max with W.R.)
e. Specified Air Content % by Volume = 0 - 3 (Entrapped)
f. Max Size Aggregate = 1"
B. Concrete Permanently Exposed to Weather, Exterior Walls, Exterior Footings:
a. Min. Cementitious Content = 658 lb/cu yd
b. Max Water-Cement Ratio = 0.42
c. Specified 28-day Compressive Strength, F<sub>c</sub> = 4500 psi
d. Specified Slump Range for Placement 4" max. w/o W.R. (8" max with W.R.)
e. Specified Air Content % by Volume = 6.0 to 8.5
f. Max Size Aggregate = 1"
C. Concrete Permanently Exposed to Weather & Deicing Chemicals; Exterior Sloops:
a. Min. Cementitious Content = 658 lb/cu yd
b. Max Water-Cement Ratio = 0.40
c. Specified 28-day Compressive Strength, F<sub>c</sub> = 5000 psi
d. Specified Slump Range for Placement 4" max. w/o W.R. (8" max with W.R.)
e. Specified Air Content % by Volume = 6.0 to 8.5
f. Max Size Aggregate = 1"
D. Concrete for Cast-in-place concrete slabs shall contain the specified water-reducing or air-entraining admixture. All concrete slabs, placed at air temperature below 50°F shall contain the specified non-corrosive, non-chloride accelerator. All concrete placed at air temperature above 80°F shall contain specified water-reducing/retarding admixture. All concrete required to be air-entrained shall contain an approved air-entraining admixture. All pumped concrete shall contain the specified high-range water-reducing admixture. Concrete with a water-cement ratio between 0.4 and 0.6 shall contain the specified water-reducer.
L. Calcium chloride shall not be permitted nor shall any admixture containing calcium chloride be permitted.
4. All pipe sleeve openings through concrete slabs shall be formed with standard steel pipe.
5. No electrical conduit shall be placed above the welded wire fabric or top reinforcing of slab.
6. All aluminum in contact with concrete or dissimilar metals shall be coated with two coats of coal tar epoxy, approved by the engineer, unless otherwise noted.
7. Concrete shall be discharged at the water table has been added to the cement and aggregates. Addition of water to the mix at the project site will not be permitted. All water must be added at the batch plant. Slump may be adjusted only through the use of additional water reducing admixtures or high range water reducing admixtures.
8. All concrete shall be placed without horizontal construction joints, except where specifically noted.
9. All exposed edges of concrete members shall be chamfered 3/4" unless shown otherwise.
10. See architectural drawings for concrete finishes, masonry anchors, and for miscellaneous embedded plates, bolts, anchors, angles, etc.
11. The placement of sleeves, outlet boxes, box-outs, anchors, etc. for the mechanical, electrical and plumbing trades is the responsibility of the trade involved; however, any box-outs not covered by typical details in structural drawings shall be submitted for approval.
12. Reinforcing bars shall conform to ASTM A615, Grade 60. No lack welding of reinforcing in the field will be permitted.
13. Reinforcing bars for welded applications shall conform to ASTM A708, Grade 60.
14. Welded wire fabric reinforcing shall conform to ASTM A185 and be furnished in flat sheets and installed on chairs.
15. Wire bar supports shall be furnished for all reinforcing within slabs, inclusive of welded wire fabric. Bottom bars in slabs-on-grade may be supported by other suitable supports. Reinforcing shall be placed to concrete placement and may not be re-positioned once concrete operations have begun. Wire bar and other types of supports shall be in accordance with the concrete reinforcing steel institute manual of standard practice.
16. Reinforcement shall be continuous through all construction joints unless otherwise noted on drawings.
17. All hooks shown on drawings shall be ACI standards unless otherwise noted.
18. Where continuous bars are called for, they shall run continuously around corners and be lapped at necessary splices. Lap lengths shall be as given in the splice and development table.
19. Provide additional reinforcing at the side and corners of all openings in concrete in accordance with typical details.
A. Minimum additional requirements are as follows:
a. (2) #5 top and bottom in CIP Concrete Slabs
b. (2) #5 each face in walls
c. (2) #5 x 4'-0" long diagonally each corner of opening
B. Extend bars a minimum of 2'-0" beyond openings, hook where extension is not possible.
20. Reinforced concrete walls, grade beams and trench footings provide corner splices of same size and spacing as horizontal reinforcing. Dowels shall lap with horizontal reinforcing in each direction.
21. The following minimum concrete cover shall be provided for reinforcement, unless otherwise noted:
A. Earth formed and cast directly against earth
B. Cast against forms but exposed to earth and weather
a. #6 and Larger - 2"
b. #5 and Smaller - 1 1/2"
C. Slabs and walls not exposed to earth or weather - 3/4"
D. Others - 2"
22. Reinforcing bars shall have a minimum clear spacing of 4"
23. SPICE LENGTHS:
Bar Size Min. Lap
#3 1'-0"
#4 1'-6"
#5 2'-0"
#6 2'-6"
#7 3'-0"
#8 4'-0"
#9 5'-0"
#10 6'-0"
A. When lapping two different size bars, use the lap dimension of the smaller bar or the anchorage dimension of the larger bar, use whichever dimension is larger.
24. Concrete slabs-on-metal deck shall be placed with the minimum thickness as described on the plans. Recess and slope slabs as described on the drawings. Place concrete to the top of slab elevations indicated on the drawings. The metal deck and steel beams will deflect, the contractor needs to provide the additional concrete required, due to deck and beam deflections.
25. Openings in concrete slab-on-metal deck. Some but not all openings are shown on the structural drawings. The contractor shall coordinate size and location of all openings with work shown on other trades drawings. Locate openings a minimum of 8" from headed studs located on the composite beams. Openings in concrete slab-on-metal deck shall be reinforced. Openings less than 6" in maximum dimension the reinforcing shall be relocated to each side of the opening and the openings reinforced with #4 @ 6" placed in the bottom flute of the deck on each side of the opening provided other openings are not located within 12". Openings greater than 6" or multiple smaller openings shall be reinforced per detail G/SS13. Openings that cannot be made in accordance with these requirements shall be brought to the attention of the architect/engineer. All openings and reinforcing steel shall be shown on the reinforcing steel shop drawings.
26. Embedded electrical boxes in concrete slab-on-metal deck shall be reinforced as openings. Embedded conduit and piping shall not be larger than 1" diameter, shall not be spaced closer than 6" on centers, shall not interfere with the reinforcing steel placement, shall be placed 1" clear the top of the slab, shall be placed 1" clear the bottom of the concrete, shall be placed 1" clear of reinforcing bars and welded wire fabric. Locate conduit and lockouts 8" minimum from headed studs located on the composite steel beams.
27. Do not support piping in excess of 500 pounds from the concrete slab-on-metal deck. Where the piping load exceeds 500 pounds provide supplemental framing spanning to the beams. All plumbing attachments and framing shall be designed by the plumbing contractor's engineer.
PRECAST STRUCTURAL CONCRETE
1. Precast structural concrete construction shall consist of the design, manufacture, transportation and erection of structural precast wall panels.
2. Design of precast members shall be in accordance with the following standards:
ACI 301 - Specifications for Structural Concrete for Buildings
ACI 318 - Building Code Requirements for Reinforced Concrete
PC MN - 127
PC MN - 117 - Manual for Quality Control for Plants and Production of Architectural Precast Concrete Products
AWS D1.4 - Structural Welding Code - Reinforcing Steel
AWS D1.1 - Structural Welding Code - Steel
ACI 315 - Details and Detailing of Concrete Reinforcement.
3. Precast members may be conventionally reinforced and/or prestressed, consistent with the design loads, spans, handling stresses, etc. Design loads are shown on drawings.
4. Precast contractor shall furnish complete design calculations signed by a licensed structural engineer licensed in Illinois, including details of connections, bearings, fittings.
5. Precast contractor shall furnish and place any temporary shoring, bracing, etc., required for erection of precast work.
6. Precast contractor shall furnish all plates, inserts, angles, rods, etc., required to connect precast to precast concrete members, structural steel members or masonry members. Details placing plans shall be furnished for all items to be embedded in masonry on concrete footings.
7. Precast manufacturer shall cast in structural inserts, bolts, plates, angles, dowels, skeways as detailed in the contract drawings.
8. In order to accommodate varying manufacturing and erection procedures, shop practices, etc., deviations from the details and members sizes shown in the contract will be considered. Such deviations will be permitted only after the engineer's approval. Cloud all deviations on the shop drawings.
9. Refer to architectural and mechanical drawings for openings required through the wall and locations of spandrel panels. PRECASTER shall layout his walls, considering such openings, and shall furnish any headers or supports as required. Locations of field cut openings shall be coordinated with wall layout. The contractor shall coordinate with mechanical, electrical and plumbing work for wall penetrations and embedded items. All openings shall be shown on the shop drawings. All openings shall be accounted for in the precast design weather cast in, field cut, or core drilled. Rules for making these openings shall be clearly shown and described on the precast shop drawings.
10. Wall thickness are shown on the drawings. See the Architectural drawings for the wall layout and joint locations. Provide joint locations where shown on the drawings.
11. The precast concrete shall be coordinated with all other work.
12. Connections:
A. Connect all panels to the foundation with a minimum of two connections per panel.
B. Connect all panels to the roof framing members as shown on the drawings.
C. Design all connections for a minimum horizontal load equal to 5% of the vertical reactions.
D. Galvanize embedded steel items that will be exposed to the weather in the completed construction or in direct contact with soils.
E. Design connections to be ductile for horizontal in-plane volume change.
F. Connections shall have sufficient strength to maintain in-plane panel to panel alignment.
G. Connect panels together with a minimum of two connections per panel. Additional connections may be required by design. Locate connections on the interior side of exterior wall members. Where required by the architectural drawings or specifications the connections shall be concealed. Connections are subject to the aesthetic approval of the architect/engineer. Wall panels shall be connected together to resist in-plane lateral loads as a whole and not individual panels.
H. Connections shall accommodate the expansion of the steel during welding without damage to the precast concrete.
I. Embed plates welded to continuous steel members such as chords, studs, and edge angles shall not be recessed below the surface of the panel, shall accommodate welding tolerances, and shall be in alignment.
J. It is acceptable to provide 1 1/2" minimum thickness insulation between the connections at insulated precast panels. The PRECASTER needs to take into account all thermal effects as specified.
K. The PRECASTER may provide embedded plates and welded connections in place of steel angles cast directly into the panel. The PRECASTER will still provide a design for all items delegated to the PRECASTER on the drawings. The general contractor needs to account for any shifting of work from one trade to another.

FOUNDATION
1. The contractor shall familiarize themselves with the survey and the geotechnical investigation report before starting construction. All foundation work shall be in accordance with the recommendation of the geotechnical report by Geotechnics dated February 11, 2023, except where noted otherwise on drawings or specifications.
2. A soils testing laboratory shall be retained by the owner for project construction review to insure conformance with the construction documents during the excavation, back fill, and foundation phases of the project.
3. Foundation design is based on:
A. 4000 psf net allowable soil bearing pressure for footings on stone column ground improvement.
B. 1000 psf net allowable soil bearing pressure for footings without ground improvement.
4. All fill material shall be free of organic contaminants and other deleterious matter.
5. All soil surrounding and under footings shall be protected from frost action and freezing during the course of construction.
6. Notify structural engineer of any unusual soil conditions that are in variance with the geotechnical report.
7. Footing excavations shall be made to the required lines and grades as rapidly as possible. Footing excavations are left open for a minimum of time to prevent disturbance to the foundation soils. Foot traffic should be prevented on the base of the footing excavations if disturbance is noted. Hand cleaning, if required and setting of reinforcing steel should then be accomplished from the sides of the excavation.
8. The top of the stone column piers shall be compacted prior to footing placement. If the top of the piers are disturbed they shall be repaired in accordance with the stone column ground improvement design.
STONE COLUMN GROUND IMPROVEMENT (STONE COLUMNS)
1. Stone column ground improvement design shall be capable of supporting the foundation as shown on structural drawings. Stone column ground improvement design shall be approved by a Geotechnical Engineer prior to stone column construction.
2. Stone column ground improvement shall improve the substrate to net allowable soil bearing pressure of 4,000 psf minimum with maximum differential deflection of 1/2".
3. Stone column layout is the responsibility of the contractor. Stone Column elements shall be installed within tolerance as prescribed by the stone column ground improvement design.
4. Contractor shall submit stone column ground improvement design drawings and calculations to the engineer prior to stone column ground improvement installation. Drawings and calculations shall be stamped and signed by an engineer registered in the state the project is located.
5. A qualified, full-time quality control (QC) representative provided by stone column ground improvement installer shall be responsible for installation of stone column elements in accordance with the design, and shall report all stone column foundation construction activities to the designer. If authorized by the owner, the QC representative shall coordinate QC activities with the testing agency hired by the owner. Under no circumstances shall testing agency direct stone column ground improvement installation procedures.
6. Stone column element design shall be confirmed by a full-scale modulus test performed at the site under supervision of the QC representative.
7. New excavations shall be performed outside the limits of the installed stone column ground improvement except where details are provided by the stone column contractor on the engineered shop drawings. The contractor shall coordinate excavations with stone column ground improvement design drawings and representative prior to shop drawing preparation and prior to performing excavation work.
ABBREVIATIONS
& ANCHOR BOLT LG LONG
AR ARCHITECT LL LIFE LOAD
ARCH ARCHITECT LLH LONG LEG HORIZONTAL
@ AT LONG LEG VERTICAL
BLDG BUILDING LVC LONGITUDINAL
BM BEAM LWC LIGHT WEIGHT CONCRETE
BOC BOTTOM OF MAX MAXIMUM
BRT BOTTOM MECHANICAL
BRG BEARING MFR MANUFACTURER
BRG BRG MINIMUM
BTW BETWEEN NTS (N) NUMBER
BYD BEYOND NTS NOT TO SCALE
CIP CAST IN PLACE OC ON CENTER
CONJ CONSTRUCTION JOINT OH OPPOSITE HAND
CL (C) CENTERLINE OPNG OPENING
CLR CLEAR OPP OPPOSITE
CONC CONCRETE MASONRY UNIT OTB OPEN TO BELOW
COL COLUMN PAR PARALLEL
CONC CONCRETE PEMB PRE-ENGINEERED METAL BUILDING
CTR CENTER PERP PERPENDICULAR
DBA DEFORMED BAR ANCHOR PSF POUNDS PER SQUARE FOOT
DIA (Ø) DIAMETER PT PRESSURE TREATED
DIAPH DIAPHRAGM RENF REINFORCING
DLD DEAD LOAD W/O WITHOUT
DWLS DOWELS RTS ROOF TOP UNIT
EA EACH SCH SCHEDULE
EF EACH FACE SIM SIMILAR
ELEV (EL) ELEVATION SL (S) STEEL LINE
EMB EMBEDMENT SS STAINLESS STEEL
EW EACH WAY STAG STAGGERED
EX EXISTING NTS STANDARD
STIFF STIFFENER
FB FIELD BEND TRB TO BE REMOVED
FDN FOUNDATION THK THICK
FINISH FLOOR THRU THROUGH
FLR FLOOR TO TOP OF
FRT FOOTING TO TOP OF FOOTING
FV FIELD VERIFY TOS TOP OF STEEL
GA GAUGE TOW TOP OF WALL
GALV GALVANIZED TRANS TRANSVERSE
TYP TYPICAL
HDS HOT DIP GALVANIZED UNO UNLESS OTHERWISE NOTED
HDR HANGER VERT VERTICAL
HGR HORIZONTAL
HSS HOLLOW STRUCTURAL SECTION W/ WITH
HT HEIGHT W/ WIDE FLANGE
HT WITHOUT W/O WITHOUT
ID INSIDE DIAMETER WP WORKING POINT
JST JOIST WWF WELDED WIRE FABRIC
W/R WATER REDUCER

MASONRY
1. All masonry shall conform to "Building Code Requirements for Masonry Structures" (ACI 530/ASCE 5/TUMS 402) and "Specifications for Masonry Structures" (ACI 530.1/ASCE 6/TUMS 602) for the year referenced in the building code noted.
2. All brick and concrete masonry construction shall comply with the recommendations of Brick Industry of Association (IA) and the National Concrete Masonry Association (NCA) and minimum requirements for new masonry built-up codes.
3. Shop Drawings: submit reinforcing steel elevations of each wall showing all of the reinforcing steel coordinated with each opening. Show lap splices and reinforcing lengths which are coordinated with the masonry lifts. Reinforcing steel shall be coordinated with plumbing, electrical and other trades.
4. Grout to fill masonry unit cores shall be ASTM C476, coarse grout (3/8" maximum aggregate) with a minimum compressive strength of 3000 psi in 28 days.
5. All concrete masonry units shall be units conforming to ASTM C90, Grade N, Type I, Normal Weight (density of unit = 135 pcf), min. block compressive strength = 2000 psi, specified design strength of masonry, fm=2000 psi.
6. Mortar:
A. ASTM C270 Type "S" mortar with a minimum compressive strength of 1800 psi shall be used for concrete masonry units.
B. ASTM C270 Type "N" mortar with a minimum compressive strength of 800 psi shall be used for brick masonry units.
7. Reinforcing bars shall conform to ASTM A615, Grade 60.
8. All concrete masonry units shall have galvanized horizontal joint reinforcement as follows:
A. 9 gage side and cross rods (ladder type) spaced 16" o.c. vertically with a 6" minimum lap.
9. All concrete masonry cores with reinforcing bars shall be filled solid with grout. Place reinforcing bars before grouting. Place grout in lifts not exceeding 5 feet. Consolidate each lift by mechanical vibration. The next lift of the pour may be after initial water loss and reconsolidation of the prior lift, while it is still plastic.
10. Vertical Concrete Masonry Reinforcement:
A. 6" CMU partitions up to 15' tall shall be reinforced with #3's vertical at 48" centers in center of grouted cells.
B. 8" CMU partitions up to 15' tall shall be reinforced with #3's vertical at 48" centers in center of grouted cells.
C. 8" CMU partitions up to 24' tall shall be reinforced with #4's vertical at 48" centers in center of grouted cells.
D. Securely partition 6" or 8" CMU up to 32' tall reinforced with #4's vertical at 8" centers in center of grouted cells, grout solid.
E. Place continuous vertical reinforcing at jamps, intersections, corners, and ends of walls unless noted otherwise.
11. Properly secure reinforcing bars to maintain the position indicated on the drawings. Bars are to be located in center of cells unless otherwise noted.
12. Reinforcing bars shall run continuously around corners and be lapped at necessary splices. Lap lengths shall be as given in the splice and development table.
13. Wall intersections shall be connected as follows:
A. 50% of the Masonry units at the interface shall interlock or
B. Walls shall be regularly toothed with #8 maximum offsets and anchored by 1/4"x1 1/2"x28" including 2" long 90 degree bend at each end to form a zee shape grouted at 4'-0" centers maximum or by intersecting bond beams with (2) #4 continuous at 4'-0" centers maximum.
14. Mortar protrusions extending into cells or cavities to be reinforced and filled, shall be removed.
15. Provide dowels in footings and floor slabs at each vertical reinforcing bar. The dowels shall be the same size as the reinforcing bars. The dowels shall extend to 3" clear the bottom of the footings or floor slabs with and ACI standard 90 degree hooks, unless noted otherwise.
16. Lay concrete masonry units with full mortar coverage on horizontal and vertical face shell. Bed webs in mortar in starting course of footing and in all courses of columns and pilasters, and where adjacent to cells or cavities to be reinforced with full grout.
17. All CMU shall be temporarily braced during construction per the governing building code for lateral loads until permanent restraints have been installed. Temporary bracing is the sole responsibility of the contractor. The contractor is responsible for all cost associated with repairs resulting from improper or insufficient bracing.
18. All openings shall have a lintel at the head. All CMU bond beams shall have two continuous #4 bars bottom and be grouted solid to a height of 16", unless noted otherwise. Provide 16" minimum between adjacent openings. Bear lintels 8" minimum at each jamb. Each sill shall have a knockout bond beam w/ (2) cont. #4's. Extend lintel ends at least 2'-0" past jambs of openings. Where corners occur, extend bars around the corner.
19. Core drill holes are not permitted in reinforced cells or lintels. Holes less than 6" in diameter may be core drilled, formed or sleeved at all other locations without a lintel provided it is located at least 16" from other openings.
20. Masonry shall be coordinated with MEP. Embedded plumbing shall be placed in cells without reinforcing steel. Embedded electrical conduit shall be 1 1/2" maximum outside diameter, placed 2 1/2" clear from the reinforcing steel, and centered in the CMU wall. Where masonry webs or face shells are cut for embedding plumbing or electrical conduit, horizontal joint reinforcing shall be provided at 8" on centers within 4'-0" of the location.
21. Where vertical reinforcing steel is interrupted by a steel beam or joist, locate vertical reinforcing steel to an adjacent cell and lap reinforcing steel one splice length plus the distance of the offset.
22. Top of interior non-bearing CMU partition walls shall be laterally braced at 10'-0" on centers maximum. Where the wall form intersections or corners with another attached masonry wall, the wall is considered braced at that location. Provide bracing between corners and intersections per detail G/SS11.
23. Provide (2) - cont. #4's horizontal in knockout bond beam at top of walls, unless noted otherwise.
24. SPICE LENGTHS:
Bar Size Min. Lap
#3 4'-0"
#4 4'-6"
#5 5'-0"
#6 5'-6"
#7 6'-0"
#8 6'-6"
#9 7'-0"
A. When lapping two different size bars, use the lap dimension of the smaller bar or the anchorage dimension of the larger bar, use whichever dimension is larger.
25. Provide 2 1/4" minimum clear cover.
26. Submit reinforcing steel shop drawings for reinforced masonry. Vertical reinforcing lengths shall be coordinated with lifts and heights of the CMU walls being placed.

POST INSTALLED ANCHORS
1. Concrete adhesive anchors Hilli HY200 or approved equal. Concrete Mechanical Anchors Hilli Kwik Bolt T22 or approved equal.
2. Masonry adhesive anchors Hilli HY270 or approved equal. Masonry Mechanical Anchors Hilli Kwik Bolt III in grouted CMU or approved equal.
3. Submit ICC-ES reports for all post installed anchors.
4. Install all post installed anchors per the product's ICC-ES report and the manufacturer's written instructions.
5. Post installed anchors shall be inspected per the product's ICC-ES report.
6. Install adhesive anchors in dry hammer drilled holes.

SNOW DRIFT DIAGRAM PLAN
1. NTS
2. NTS



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REVISION HISTORY	DATE	APPR
DESCRIPTION		

ISSUED FOR: **03/01/2024**

**BID & PERMIT**

**EDGAR COUNTY PUBLIC SAFETY CENTER**  
**EDGAR COUNTY, ILLINOIS**  
**12636 950TH ROAD**  
**PARIS, ILLINOIS 61944**

Non-Reduced Sheet Size 30" x 42"  
Full sheet plots have been prepared using standard scales.  
Reduced sheet plots may not conform to standard scales.

DATE	DESCRIPTION	BY	CHKD
JTS	DRAWING		
NML	FIELD BOOK		

CHECKED: **ADL** CHECK DATE: **03/01/2024**

SHEET TITLE: **STRUCTURAL NOTES**

PROJECT NO: **22-0448**  
DRAWING ISSUED DATE: **03/01/2024**  
SHEET: **S001**

COLD-FORMED METAL FRAMING

- 1. All cold-formed steel framing shall comply with the building code and AISI - American Iron and Steel Institute - "Cold-formed Steel Design Manual."
2. Structural cold-formed metal framing is all joists, load bearing walls, exterior walls, soffits, parapets, bulkheads, headers, jamb studs, suspended walls, interior stud walls over 20'-0" tall, cold-formed channel framing (Unistrut), partial height cantilevered walls (pony walls), and custom built suspended ceiling systems.
3. Structural cold-formed metal framing shall be constructed to achieve the geometry shown on the architectural and structural drawings. Construct the cold-formed metal framing in accordance with the shop drawings and the delegated design submittal.
4. See the specifications for additional requirements.
5. All framing shall be galvanized. All welds shall be touched up with zinc-rich paint.
6. Studs and Joists shall be spaced at 16" o.c. unless noted otherwise on the drawings.
7. Temporary bracing shall be provided and remain in place until work is completely stabilized.
8. Field cutting of steel framing members shall be by saw or shear. Torch cutting is not permitted.
9. Connect multiple studs and track together with #10 TEKs screws at 12" on centers unless noted otherwise on the shop drawings.
10. Material shall conform to the following:
A. Galvanized Material:
a. All galvanized studs and joists shall be formed from steel that corresponds to the minimum requirements of ASTM A1003, Type H with a minimum yield strength specified on the shop drawings.
b. All galvanized studs and joists, all galvanized track, bridging, end closures and accessories shall be formed from steel that corresponds to the minimum requirements of ASTM A1003, Type H, with minimum yield strength specified on the shop drawings.
c. All galvanized studs, joists, tracks, bridging, and accessories shall be formed from steel having a galvanized coating meeting the requirements of A1003, Type H.
B. Properties:
a. The physical and structural properties listed by the steel stud manufacturer association and AISI design manual shall be considered the minimum required for all framing members. The properties include: Lx (in4), Sx (in3), Area (in2), Rx (in3/ft), Iy (in4) and Resisting Moment (in-ft).
C. Substitutions: Any Substitutions must be approved in writing prior to delivery, by the architect and/or engineer of record.
11. Installation of studs shall be as per ASTM C1007 "Standard Specification for Installation of Load Bearing (Transverse and Axial) Steel Studs and Related Accessories"; ASTM C 955 "Standard Specification for Load-Bearing (Transverse and Axial) Steel Studs, Runners (Tracks), and Bridging or Bridging for Screw Application of Gypsum Panel Products and Metal Plaster Bases"; ASTM C754 "Standard Specification for Installation of Steel Framing Members to Receive Screw-Attached Gypsum Panel Products."
12. All framing components shall be cut squarely for attachment to perpendicular members, or as required for an angular fit against abutting members.
13. All track butt joints, abutting pieces of track shall be securely anchored to a common structural element, or that shall be bolt-welded or spliced together.
14. All stud bridging shall be attached in a manner to prevent stud rotation. Bridging rows shall be spaced according to industry recommendations.
15. Temporary bracing shall be provided until erection is completed.
16. Design Criteria: The cold-formed metal framing supplier is responsible for the design of the members and connections to resist all loads which act upon the framing. See the specifications for the deflection criteria. The framing design loads are included but not limited to the following:
A. Dead loads of walls, floors, and ceilings may be assumed to be 15 psf unless specifically calculated. Allow for loads of screens, speakers, mechanical units and other concentrated loads from items shown on the drawings.
B. Live loads shown in the design criteria shall be accounted for in the design of floors. Live loads of areas above cold-formed ceilings where more than 42 inches high by 12 inches wide rectangle is able to be contained shall be designed for a 250 lbs concentrated load non-concurrent with a 20 pound per square foot live load.
C. Cold-formed steel framing shall not be hung from metal roof deck. The maximum reaction of 100 lbs may be suspended from a steel joist. Support from as many joist panel points as needed to limit the reactions.
D. Wind loads shown in the design criteria shall be accounted for in the design of exterior walls, parapets, and soffits.
E. Interior walls shall be designed for a horizontal out-of-plane load of 5 pounds per square foot live load.
F. Where cold-formed metal framing is a guardrail the framing and connections shall be designed for the greater of a 200 pound concentrated load or 50 pounds per linear foot per the International Building Code.
17. Where sheathing does not occur on both sides of walls or ceilings provide lateral bracing. Provide stud bridging with 1 1/2" cold rolled U-channel. The U-channel must be attached to each stud by welding or attaching with clip angles and screws. Horizontal strapping and solid bridging with track members can also be used for bridging. Bridging shall be spaced at 4'-0" o.c. maximum. Rigid insulation may not be used for bracing.
18. The following minimum cold-form steel attachments shall be provided unless otherwise noted on the shop drawings:
A. Track to Stud: (1) - #10 TEKs screw each flange, each stud.
B. Track to Structural Steel: (1) - 0.157" dia. power driven fastener at 2'-0" o.c.
C. Track to Concrete: (1) - 0.157" dia. low velocity power driven fastener at 16" o.c. with 1 1/2" penetration.
D. Track to Metal Deck: (1) - #10 TEKs screw at 16" o.c.
E. Track to Masonry: (1) - 0.157" dia. power driven fastener at 12" o.c.
F. Stud to Structural Steel: (1) - L2x2x1/4 GA clip angle connection with (3) #10 TEKs screws into metal stud and (3) 0.157" dia. power driven fasteners into structural steel or (3) 1/2" TEKs screws into structural steel.
G. Exterior Sheathing to Stud: #8x1" long Flat Head Self-Drilling Screw at 12" o.c. (6" o.c. at panel edges).
19. All framing components shall be cut squarely or at an angle to fit squarely against abutting members. Splicing of axially loaded members shall not be permitted. Members shall be held firmly in place until properly fastened. Attachments of similar components shall be by welding, screw attachment, or bolting. Wire tying of components is not permitted.
20. Members shall not be spliced other than at the locations indicated on the shop drawings. All splices shall conform to the details in the shop drawings.
21. Contractor shall verify sizes and locations of structural components where members attach.
22. No notching or coping of studs is allowed, unless stated on the shop drawings.
23. The maximum allowable gap (measured between the web of the stud and of the track) for a stud seated in a track is 1/8" for non-axial load bearing conditions and 1/16" for axial load bearing conditions (U.N.O.) Pressure should be applied to nest the studs into the tracks until the tolerances listed above are achieved. Failure to do so could result in serviceability problems in the future.
24. Provide 12 gage minimum stud and track thickness adjacent to doors, windows, and louvers. Contractor coordinate with fastening requirements at fastening openings.
25. See typical framing details on sheet S516.

PRODUCT IDENTIFICATION

- 1. The designations of the Steel Stud Manufacturer Association are used in this package. Any Manufacturer whose product geometry meets or exceeds SSMA standards is acceptable. See below for SSMA nomenclature.

MEMBER WEB DEPTH

(Example: 6" = 600 x 1/160 inch)
All member depths are given in 1/160 inch.
For all "T" sections, member depth is the inside to inside dimension.

FLANGE WIDTH

(Example: 1 1/2" = 1.625" = 162 x 1/160 inch)
All flange widths are given in 1/160 inch.

STYLE

(Example: Stud or Joist section = S)
Nomenclature uses the following four characters to designate the profile:
S = Stud or Joist Sections
T = Track Sections
U = Channel Sections
F = Furring Channel Sections

MIL THICKNESS

(Example: 0.054" = 54 mils; 1 mil = 1/160 inch)
Mil thickness is the minimum base steel thickness measured in 1/160 inch. Minimum base steel thickness represents 95 percent of the design thickness.

Table with 2 columns: Web Size (Nominal Member Depth) and Flange Designations. Rows include 3-5/8" Member = 362, 6" Member = 600, 8" Member = 800, 10" Member = 1000, 12" Member = 1200.

3. The Last Two Numbers Indicate the Steel Thickness:

Table with 4 columns: Gage, Design, Minimum, SSMA, Color Coding. Rows include 20, 18, 16, 14, 12 gages with corresponding design, minimum, SSMA, and color coding.

STRUCTURAL STEEL

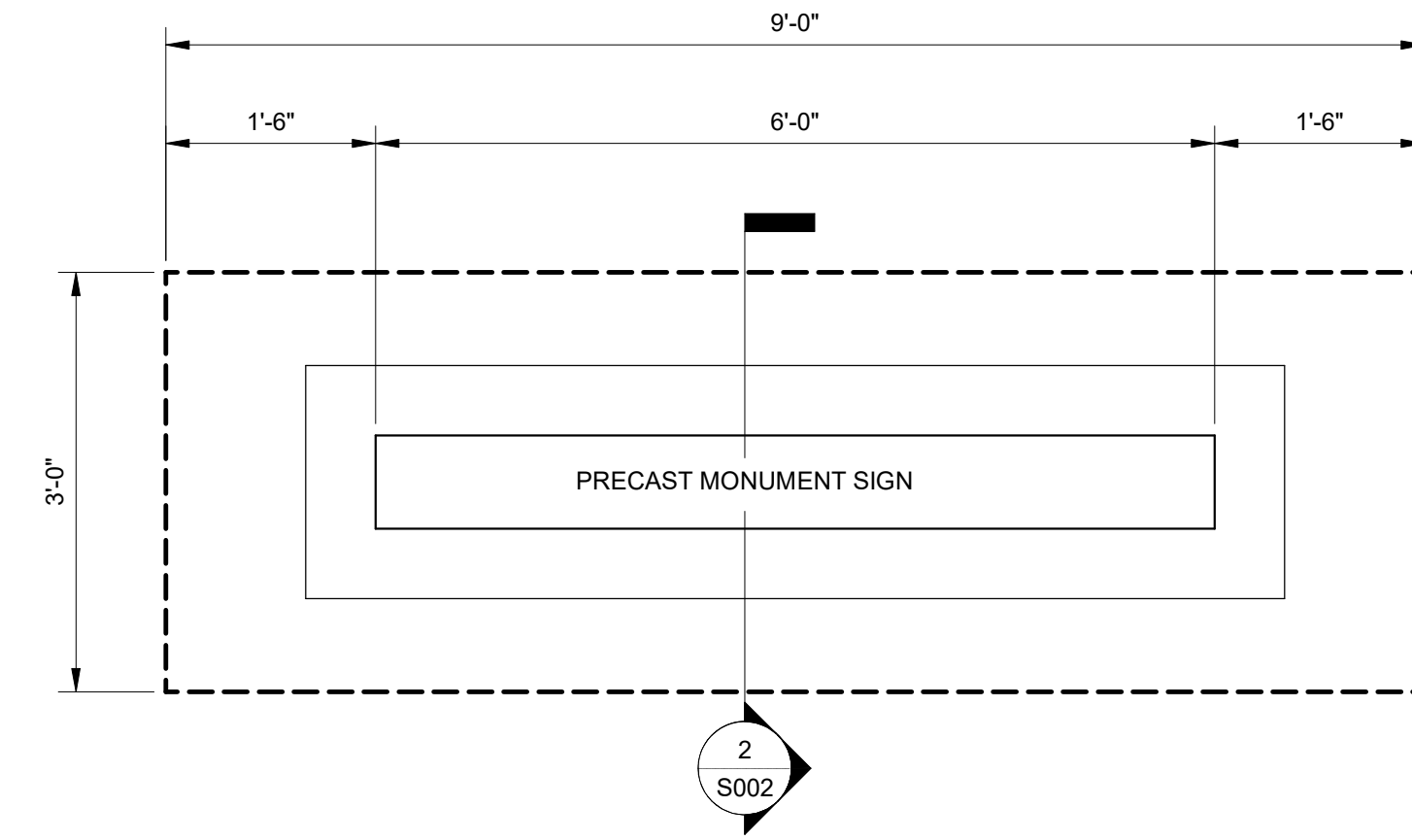
- 1. Detailing, fabrication and erection shall conform to the AISC Specifications and Standard Code of Practice for the year referenced in the building code noted, except as modified by these notes and the project specifications.
2. Steel shall conform to the following grades unless otherwise noted:
A. W Shapes - ASTM A992 Grade 50 (Fy=50 ksi)
B. Plate, Angles, M, S and C Shapes - ASTM A572 (Fy=50 ksi)
C. HSS Rectangle Shapes - ASTM A500 Grade C (Fy=50 ksi)
D. HSS Round Shapes - ASTM A500 Grade C (Fy=48 ksi)
E. Pipes - ASTM A53 (Fy=35 ksi)
F. Bolts - ASTM F125, Grade A325-N, 3/4" diameter minimum.
G. Washers - ASTM F844, plain
H. Deformed Bar Anchor (DBA) - ASTM A1064 (Gr. 70) and AWS D1.1
I. Anchor Rods (Bolts) - ASTM F1554 Grade 36 (Fy=36 ksi) (If exposed to weather or in contact with treated timber hot dip galvanized per ASTM A123)
J. Welding Electrodes - E70xx
3. Unless being Galvanized, all structural steel shall be primed. Asphaltic paints are not acceptable. Exposed Steel shall be painted per Painting Specification, Field Touch up Primer and Paint.
4. All column base plates shall have a minimum of four (4) anchor rods.
5. The minimum plate thickness shall be 3/8", unless otherwise noted.
6. The minimum length of connection angle shall be equal to 1/2 the depth of the member to be supported.
7. Bolts not designated as slip critical bolts shall be considered bearing bolts. Tighten bearing bolts to a snug condition per AISC Specifications.
8. All welding shall be in accordance with the "Structural Welding Code", AWS D1.1, Latest Edition.
9. Fabricate all beams with the mill camber up.
10. Work these drawings with architectural drawings for nailer holes and architectural clearances.
11. General contractor shall verify all structural beam locations, mechanical units weights and opening sizes and locations with mechanical contractor and vendor's drawings for actual mechanical unit purchased.
12. Splicing of structural members where not detailed on the drawings is prohibited without prior approval of the structural engineer.
13. Cuts, holes, coping, etc. required for work of other trades shall be shown on the shop drawings and made in the shop. Cuts or burning of holes in the structural steel members in the field will not be permitted, unless specifically approved in each case by the engineer. Radius reentrant corners 1/2" minimum for a smooth transition.
14. All structural steel, including base plates and top of anchor bolts that are exposed to soil are to be coated with an approved cold tar epoxy, 16 mils minimum thickness.
15. Anchor Rods shall be located using templates with exposed threads (only) of rods greased after concrete has set.
16. GROUT for Base plates: Prepacked, non-metallic, non-gaseous and non-shrink per CRD C621 and ASTM C1107 at fluid consistency (flow cone) of 20-30 seconds. Minimum 28 Day Compressive Strength = 7000 PSI.
17. Hot dip galvanize per ASTM A123 after fabrication the following structural steel members:
A. Items identified on the architectural and structural drawings, including RTU support, roof top screens and their supports.
B. Items exposed to weather which are not primed with zinc rich primer and painted.
C. Items in direct contact with soil.
D. Items in direct contact with treated timber.
18. Repair galvanized surfaces according to ASTM A 780.
19. Handrails, guardrails, and posts shall be stop welded. Handrails shall be ground smooth at joints.

STEEL JOISTS

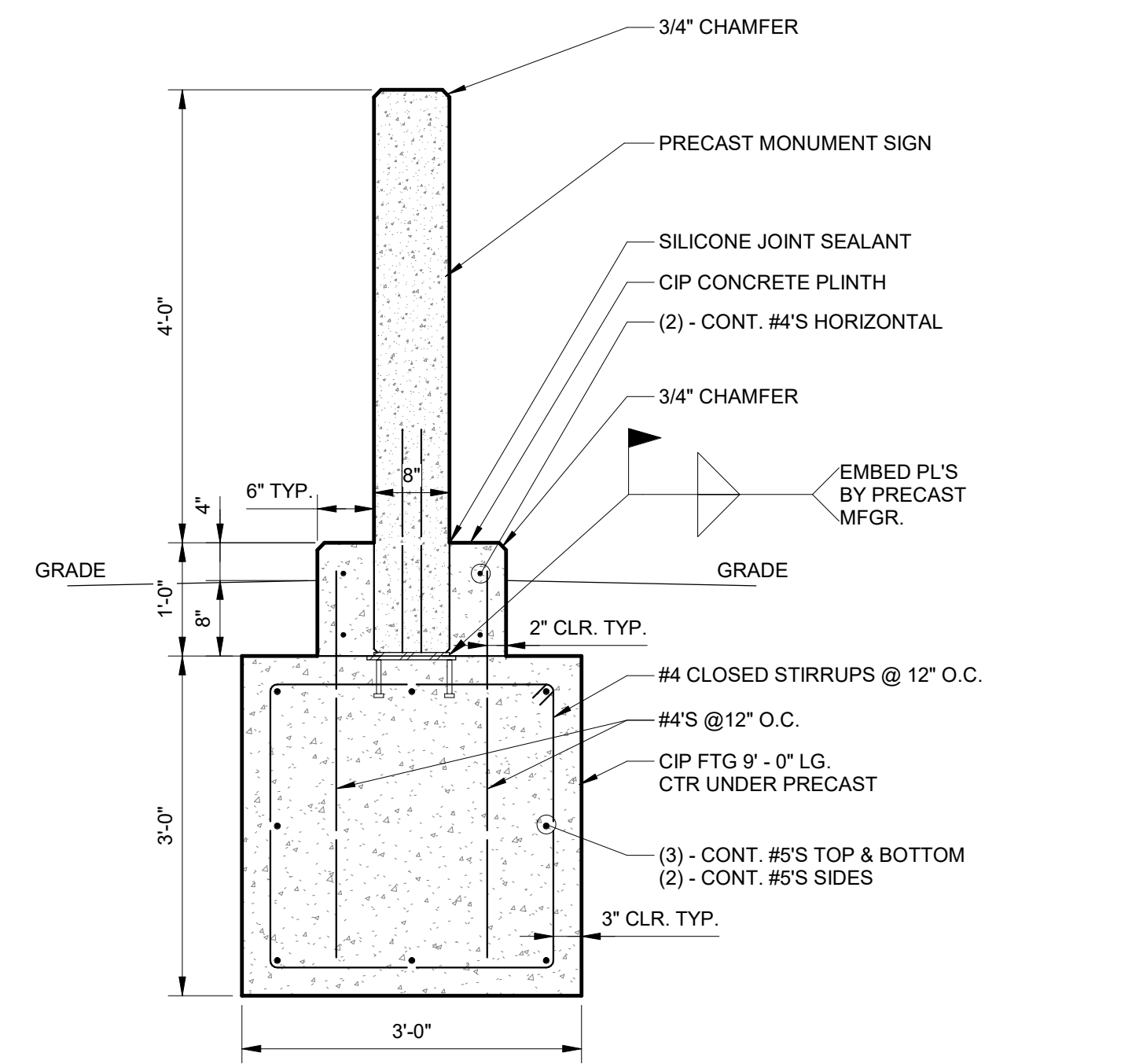
- 1. Steel joists shall be designed, detailed, fabricated, and erected in accordance with Steel Joist Institute (SJI) Specification (latest edition). Joist Manufacturer to design Joist to support Loads as shown.
2. Bridging shall be spaced in accordance with SJI Specification and the erection drawings.
3. Steel joist bridging shall be placed and joist ends fixed prior to the application of any loads.
4. Minimum bearing requirement, unless noted otherwise:
A. K-Series: 2 1/2" on Structural Steel.
5. Joist seat depths, unless noted otherwise:
A. K-Series: 2 1/2"
6. Joists, at column centerlines, shall be bolted to structural steel beams with (2) bolts.
7. Bridging that terminates at, or is interrupted by, structural steel beams, shall be attached thereto by field welding or bolting. See drawings for detail of attachment of bridging to concrete or masonry.
8. Joist shall be stock piled at the job site in a vertical position, resting on their top or bottom chords, and shall be adequately supported with wood blocking. Keep joists free of mud, dirt and covered.
9. It shall be the contractor's responsibility to see that joists which are damaged, kinked bent, or with broken welds, are not placed in the structure.
10. Joist ends, at roof diaphragm boundaries, shall be capable of transmitting the boundary shear to the supporting structure. Joist manufacture to design joists for a rollover force of 2.5 kips unless a higher force is noted on the drawings.
11. Loads between panel points shall not exceed 100 lbs unless joist reinforcing is provided.
12. The joist design and bridging placement shall be checked by the joist manufacturer using the net uplift specified on the drawings. Changes in joist size and/or bridging placement will show up on the shop drawings.
13. Loads between panel points shall not exceed 100 lbs unless joist reinforcing is provided.
14. Locate pipe and equipment hangers and other concentrated loads only where loads are shown on the joist shop drawings. Attachment method as approved by joist manufacturer.
15. Joist welds to supporting steel work to conform to the following unless otherwise noted on the drawings. Joist shop drawings shall show weld sizes and lengths.
A. K-Series: (2) 1/8" fillet welds x 2 1/2" long.
16. Joists shall be Primed by Manufacturer. Exposed Joists shall be Painted per Painting Specification.
17. Joist Manufacturer shall be responsible for the design and detail of the joist connection to shear wall against the applicable lateral loads of the building.
18. See detail S1/S13 where bridging is interrupted by ductwork.

METAL DECKING

- 1. Fabricated roof decks, without top-flange stiffening grooves, shall comply with "Steel Deck Institute (SDI) Standard for Steel Roof Decks" in ANSISDI RD-2010.
2. Fabricated noncomposite steel floor deck shall comply with "Steel Deck Institute (SDI) Standard for Non-Composite Steel Floor Deck", in ANSISDI NC-2010.
3. Roof deck shall be prime painted by the manufacturer. Exposed deck shall be painted per painting specification.
4. Floor deck shall be galvanized. Exposed deck shall be primed and painted per the paint specification.
5. Lap deck 4" minimum at splices center on support.
6. Deck manufacturer shall coordinate size and location of roof openings with architectural and mechanical drawings and suppliers.
A. Furnish HSS blocks x 0'-0" long beneath deck flutes at bearing at steel supports beneath mechanical units to prevent crushing square. Fully weld two edges of the HSS blocks to the structure.
B. Mechanical units shall be attached to the structural steel framing unless noted otherwise. Support Mechanical equipment per between joists. Support mechanical equipment with framing laid between joists welded to top of bottom chord of joists to distribute load to joist panel points. Where curb is parallel to joist, maximum spacing between framing laid between joists shall be 6'-0", or as required by equipment manufacturer, whichever is less.
C. Unless noted otherwise, all openings in metal roof deck shall be framed per 3/ S513.
D. Roof deck openings less than 6" in either direction and spaced farther than 3'-0" on center may be framed with the steel angles described in the roof deck specification.
7. No light gage framing, mechanical, electrical or other equipment shall be suspended from or attached to any metal roof deck.
8. See drawings for deck attachment patterns.
9. Concrete slab on metal deck openings shall be formed as described in the cast-in-place concrete notes.



1 MONUMENT SIGN PLAN
3/4" = 1'-0"



2 MONUMENT SIGN.
3/4" = 1'-0"



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Table with 4 columns: NO., DESCRIPTION, DATE, APPR.

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EDGAR COUNTY, ILLINOIS
12636 950TH ROAD
PARIS, ILLINOIS 61944

Non-Reduced Sheet Size 30" x 42"
Full sized plans have been prepared using standard scales.
Reduced sized plans may not conform to standard scales.

Table with 2 columns: DESIGNED, DRAWN, CHECKED, DATE. Rows include JTS, NML, ADL, 03/01/2024.

STRUCTURAL NOTES

PROJECT NO. 22-4046
DRAWING ISSUED DATE: 03/01/2024
SHEET

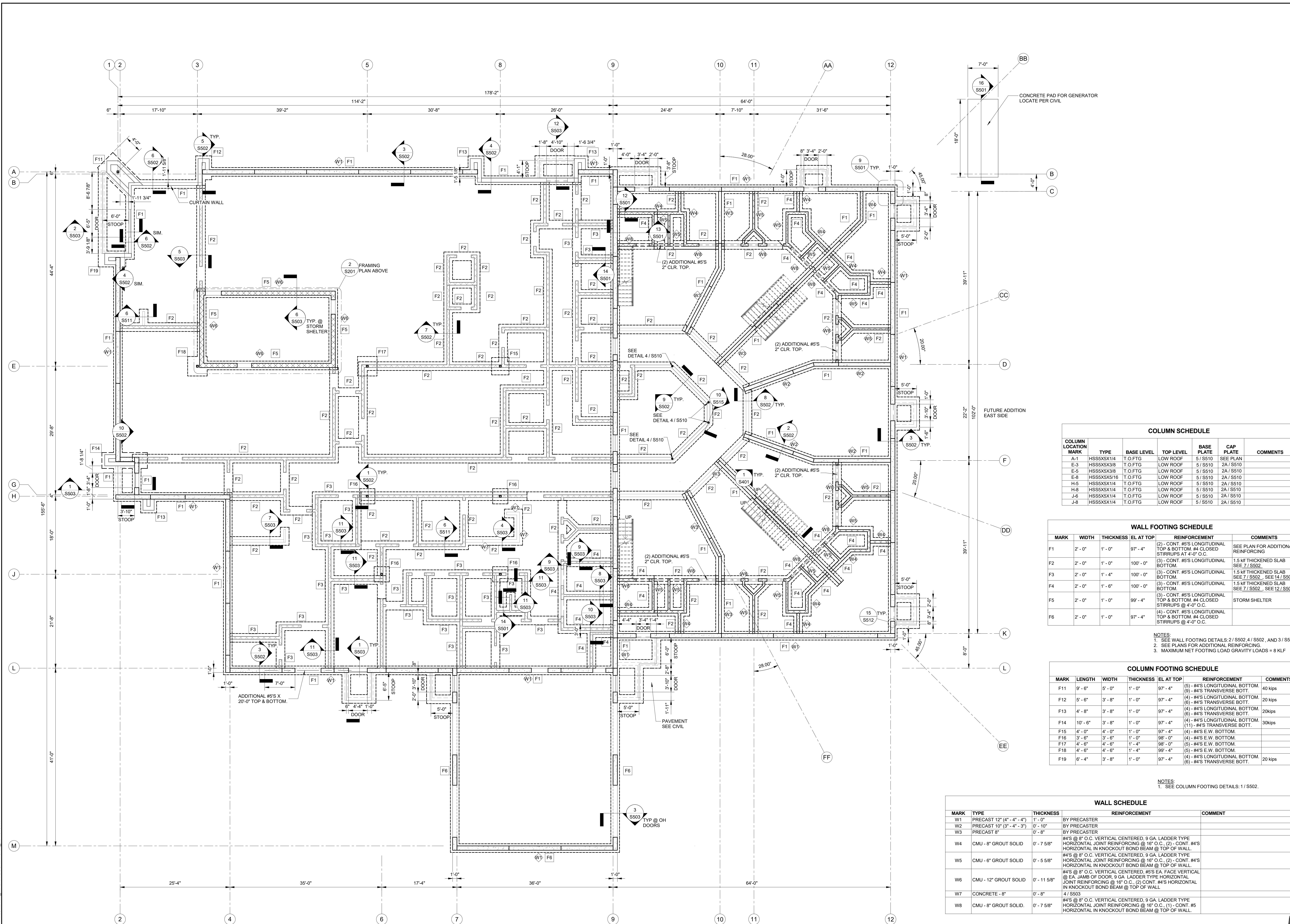
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REVISION HISTORY		
NO.	DESCRIPTION	DATE
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REVISION HISTORY		
NO.	DESCRIPTION	DATE
1	ISSUED FOR	03/01/2024

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COLUMN SCHEDULE						
COLUMN LOCATION MARK	TYPE	BASE LEVEL	TOP LEVEL	BASE PLATE	CAP PLATE	COMMENTS
A-1	HSS5X5X1/4	T.O.FTG	LOW ROOF	5/ S510	SEE PLAN	
E-3	HSS5X5X3/8	T.O.FTG	LOW ROOF	5/ S510	2A/ S510	
E-5	HSS5X5X3/8	T.O.FTG	LOW ROOF	5/ S510	2A/ S510	
E-8	HSS5X5X5/16	T.O.FTG	LOW ROOF	5/ S510	2A/ S510	
H-5	HSS5X5X1/4	T.O.FTG	LOW ROOF	5/ S510	2A/ S510	
H-8	HSS5X5X1/4	T.O.FTG	LOW ROOF	5/ S510	2A/ S510	
J-6	HSS5X5X1/4	T.O.FTG	LOW ROOF	5/ S510	2A/ S510	
J-8	HSS5X5X1/4	T.O.FTG	LOW ROOF	5/ S510	2A/ S510	

WALL FOOTING SCHEDULE						
MARK	WIDTH	THICKNESS	EL AT TOP	REINFORCEMENT	COMMENTS	
F1	2'-0"	1'-0"	97'-4"	(2) - CONT. #5'S LONGITUDINAL TOP & BOTTOM. #4 CLOSED STIRRUPS AT 4'-0" O.C.	SEE PLAN FOR ADDITIONAL REINFORCING	
F2	2'-0"	1'-0"	100'-0"	(3) - CONT. #5'S LONGITUDINAL BOTTOM.	1.5 HI THICKENED SLAB SEE 7/ S502	
F3	2'-0"	1'-4"	100'-0"	(3) - CONT. #5'S LONGITUDINAL BOTTOM.	1.5 HI THICKENED SLAB SEE 7/ S502, SEE 14/ S501	
F4	2'-0"	1'-6"	100'-0"	(3) - CONT. #5'S LONGITUDINAL BOTTOM.	1.5 HI THICKENED SLAB SEE 7/ S502, SEE 12/ S501	
F5	2'-0"	1'-0"	99'-4"	(3) - CONT. #5'S LONGITUDINAL TOP & BOTTOM. #4 CLOSED STIRRUPS AT 4'-0" O.C.	STORM SHELTER	
F6	2'-0"	1'-0"	97'-4"	(4) - CONT. #5'S LONGITUDINAL TOP & BOTTOM. #4 CLOSED STIRRUPS AT 4'-0" O.C.		

NOTES:  
 1. SEE WALL FOOTING DETAILS: 2/ S502, 4/ S502, AND 3/ S502.  
 2. SEE PLANS FOR ADDITIONAL REINFORCING.  
 3. MAXIMUM NET FOOTING LOAD GRAVITY LOADS = 8 KLF

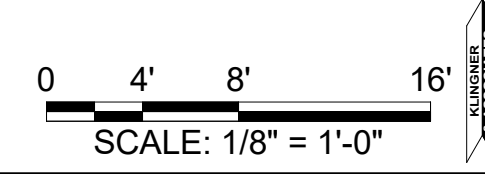
COLUMN FOOTING SCHEDULE						
MARK	LENGTH	WIDTH	THICKNESS	EL AT TOP	REINFORCEMENT	COMMENTS
F11	9'-6"	5'-0"	1'-0"	97'-4"	(5) - #4'S LONGITUDINAL BOTTOM (9) - #4'S TRANSVERSE BOTT.	40 kips
F12	5'-6"	3'-8"	1'-0"	97'-4"	(4) - #4'S LONGITUDINAL BOTTOM (6) - #4'S TRANSVERSE BOTT.	20 kips
F13	4'-8"	3'-8"	1'-0"	97'-4"	(4) - #4'S LONGITUDINAL BOTTOM (6) - #4'S TRANSVERSE BOTT.	20kips
F14	10'-6"	3'-8"	1'-0"	97'-4"	(4) - #4'S LONGITUDINAL BOTTOM (11) - #4'S TRANSVERSE BOTT.	30kips
F15	4'-0"	4'-0"	1'-0"	97'-4"	(4) - #4'S E.W. BOTTOM.	
F16	3'-6"	3'-6"	1'-0"	98'-0"	(4) - #4'S E.W. BOTTOM.	
F17	4'-6"	4'-6"	1'-4"	98'-0"	(5) - #4'S E.W. BOTTOM.	
F18	4'-6"	4'-6"	1'-4"	99'-4"	(5) - #4'S E.W. BOTTOM.	
F19	6'-4"	3'-8"	1'-0"	97'-4"	(4) - #4'S LONGITUDINAL BOTTOM (6) - #4'S TRANSVERSE BOTT.	20 kips

NOTES:  
 1. SEE COLUMN FOOTING DETAILS: 1/ S502.

WALL SCHEDULE			
MARK	TYPE	THICKNESS	REINFORCEMENT
W1	PRECAST 12" (4'-4" - 4'-4")	1'-0"	BY PRECASTER
W2	PRECAST 10" (3'-4" - 3'-4")	0'-10"	BY PRECASTER
W3	PRECAST 8"	0'-8"	BY PRECASTER
W4	CMU - 8" GROUT SOLID	0'-7 5/8"	#4'S @ 8" O.C. VERTICAL CENTERED, 9 GA. LADDER TYPE HORIZONTAL JOINT REINFORCING @ 16" O.C. (2) - CONT. #4'S HORIZONTAL IN KNOCKOUT BOND BEAM @ TOP OF WALL
W5	CMU - 6" GROUT SOLID	0'-5 5/8"	#4'S @ 8" O.C. VERTICAL CENTERED, 9 GA. LADDER TYPE HORIZONTAL JOINT REINFORCING @ 16" O.C. (2) - CONT. #4'S HORIZONTAL IN KNOCKOUT BOND BEAM @ TOP OF WALL
W6	CMU - 12" GROUT SOLID	0'-11 5/8"	#4'S @ 8" O.C. VERTICAL CENTERED, #5'S EA. FACE VERTICAL @ EA. JAMB OF DOOR, 9 GA. LADDER TYPE HORIZONTAL JOINT REINFORCING @ 16" O.C. (2) - CONT. #4'S HORIZONTAL IN KNOCKOUT BOND BEAM @ TOP OF WALL
W7	CONCRETE - 8"	0'-8"	4/ S503
W8	CMU - 8" GROUT SOLID	0'-7 5/8"	#4'S @ 8" O.C. VERTICAL CENTERED, 9 GA. LADDER TYPE HORIZONTAL JOINT REINFORCING @ 16" O.C. (1) - CONT. #5 HORIZONTAL IN KNOCKOUT BOND BEAM @ TOP OF WALL

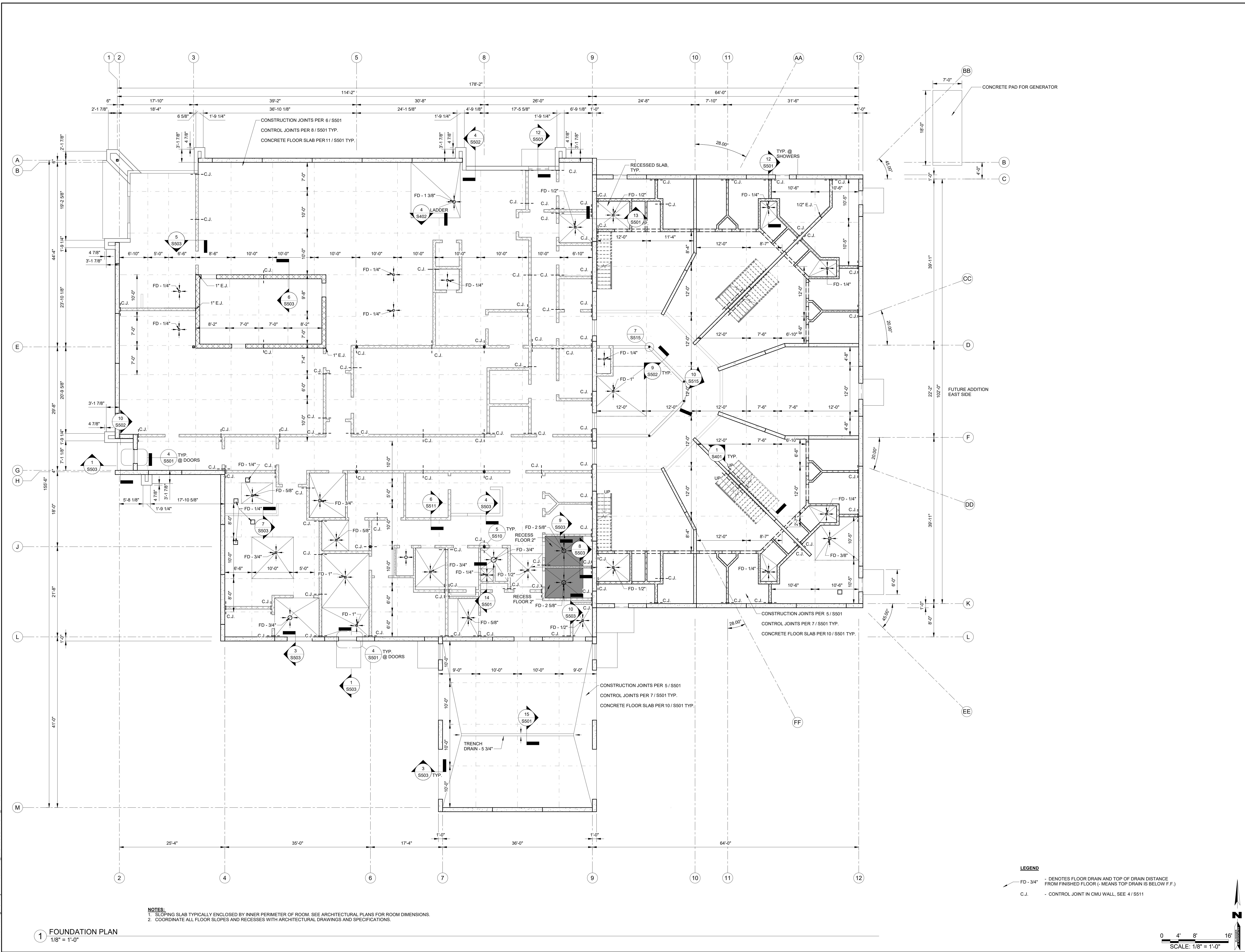
NOTES:  
 1. OPENING FOR DOORS ARE SHOWN AS NOMINAL DIMENSIONS. THE OPENINGS NEED TO BE INCREASED FOR ROUGH OPENINGS.

**1 FOUNDATION PLAN**  
 1/8" = 1'-0"



**FOUNDATION PLAN**

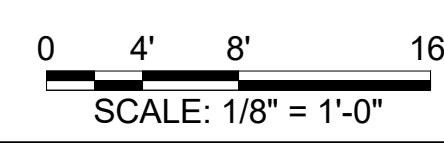
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SHEET TITLE			
PROJECT NO. 22-4048			
DRAWING ISSUED DATE: 03/01/2024			
SHEET			
<b>S101</b>			



**NOTES:**  
 1. SLOPING SLAB TYPICALLY ENCLOSED BY INNER PERIMETER OF ROOM. SEE ARCHITECTURAL PLANS FOR ROOM DIMENSIONS.  
 2. COORDINATE ALL FLOOR SLOPES AND RECESSES WITH ARCHITECTURAL DRAWINGS AND SPECIFICATIONS.

**1 FOUNDATION PLAN**  
 1/8" = 1'-0"

**LEGEND**  
 FD - 3/4" - DENOTES FLOOR DRAIN AND TOP OF DRAIN DISTANCE FROM FINISHED FLOOR (- MEANS TOP DRAIN IS BELOW F.F.)  
 C.J. - CONTROL JOINT IN CMU WALL, SEE 4 / S511



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LINTEL SCHEDULE			
MARK	SIZE	DETAIL	REINFORCING
L1	1'-0"	1/ S511 & 3/ S511	CONT. #5'S TOP & BOTTOM.
L2	1'-0"	1/ S511, 3/ S511, & 14/ S511	CONT. #5'S TOP & BOTTOM.
L3	4'-0"	13/ S511 & 5/ S511 (JAMBS)	(2) CONT. #5'S BOTTOM.
L4	1'-4"	1/ S511, 3/ S511, & 14/ S511 SIM.	CONT. #5'S TOP & BOTTOM.
L5	0'-8"	1/ S511 & 3/ S511	(2) - #4'S BOTTOM

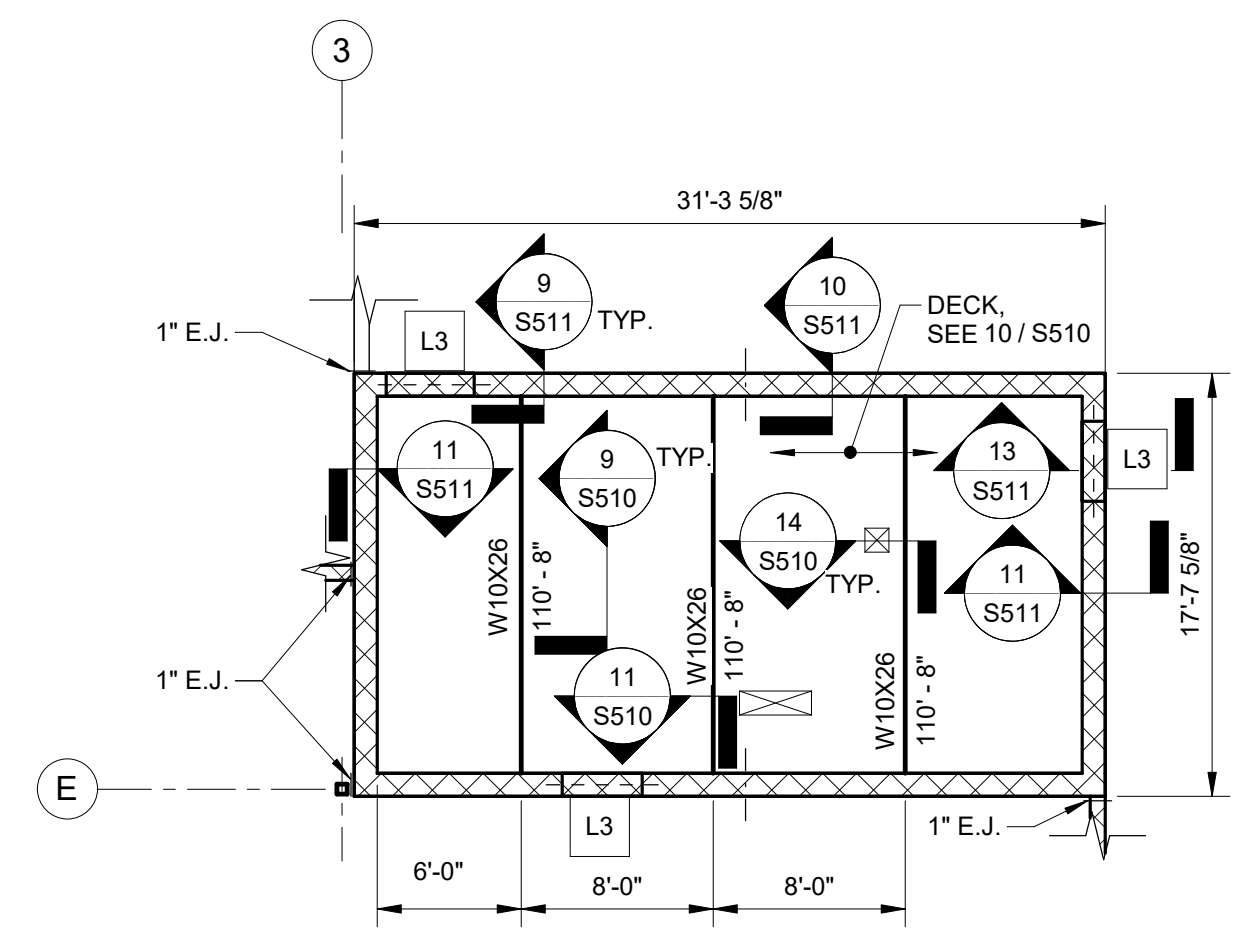
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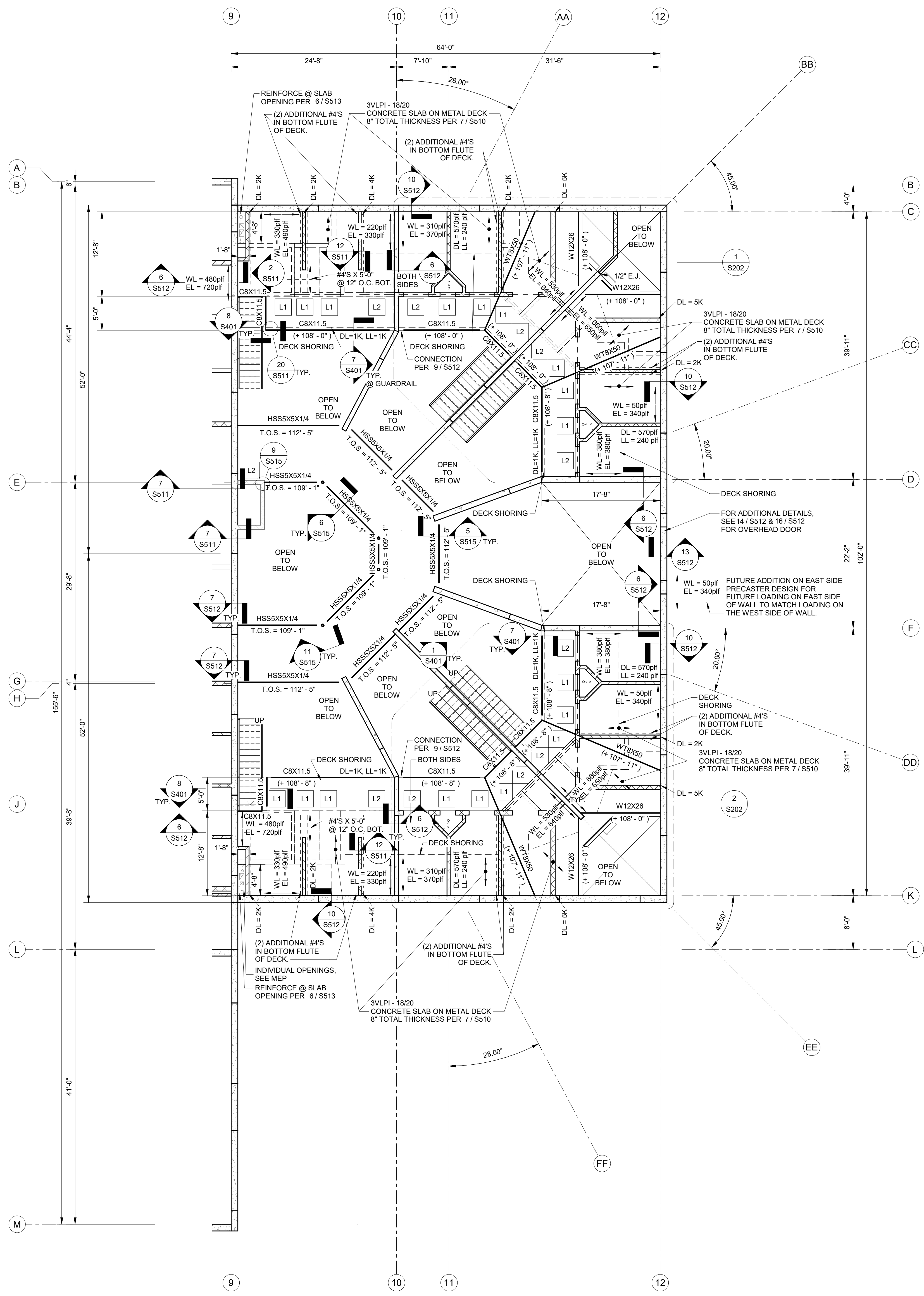
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CHECKED	ADL	ADL
DRAWN	ADL	ADL
CHECK DATE	03/01/2024	

SHEET TITLE  
**MEZZANINE FRAMING PLAN**  
 PROJECT NO. 22-4048  
 DRAWING ISSUED DATE: 03/01/2024  
**S201**

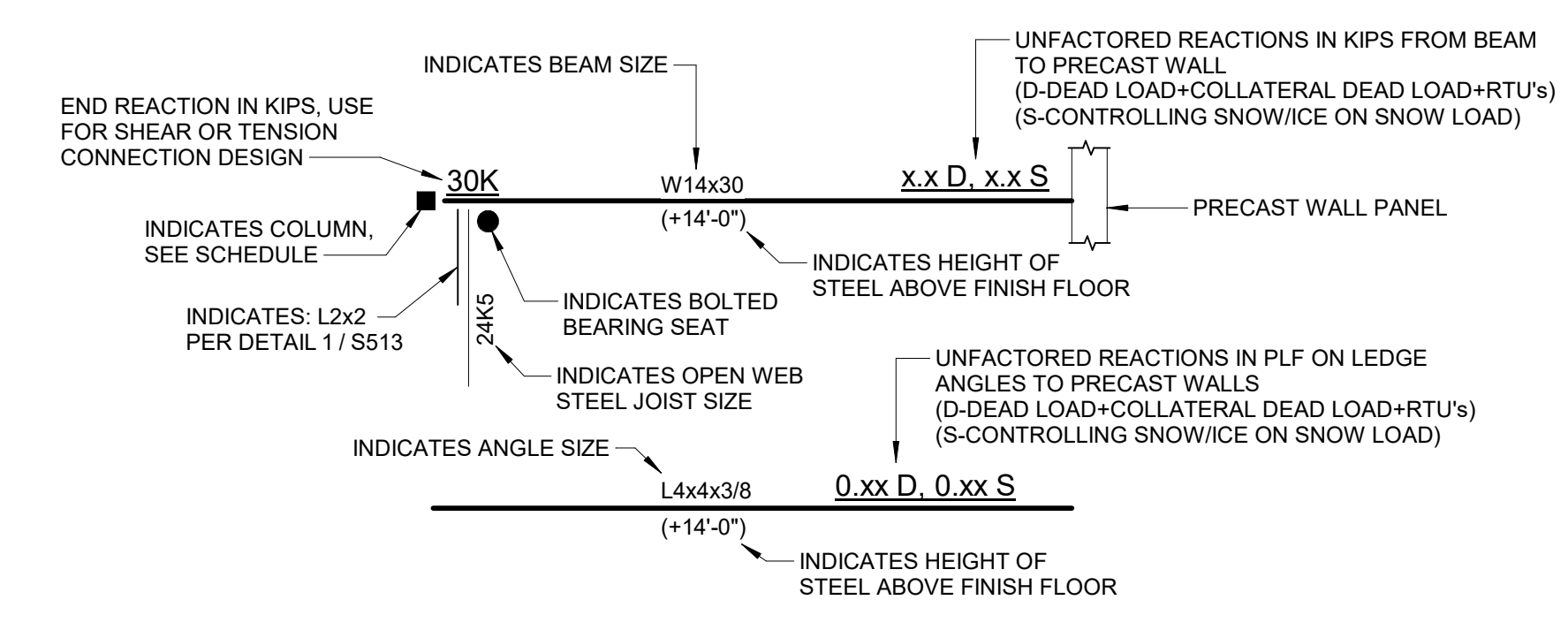


**2 STORM SHELTER FRAMING**  
 1/8" = 1'-0"

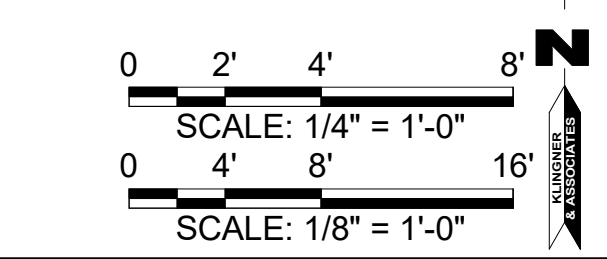


**1 T.O. MEZZ FLOOR**  
 1/8" = 1'-0"

**FRAMING LEGEND**



NOTE:  
 DECK BEARING ELEVATION = 108'-0"  
 TOP OF CONCRETE ELEVATION = 108'-8"



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PROJECT NO. 22-4048  
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SHEET  
**S202**

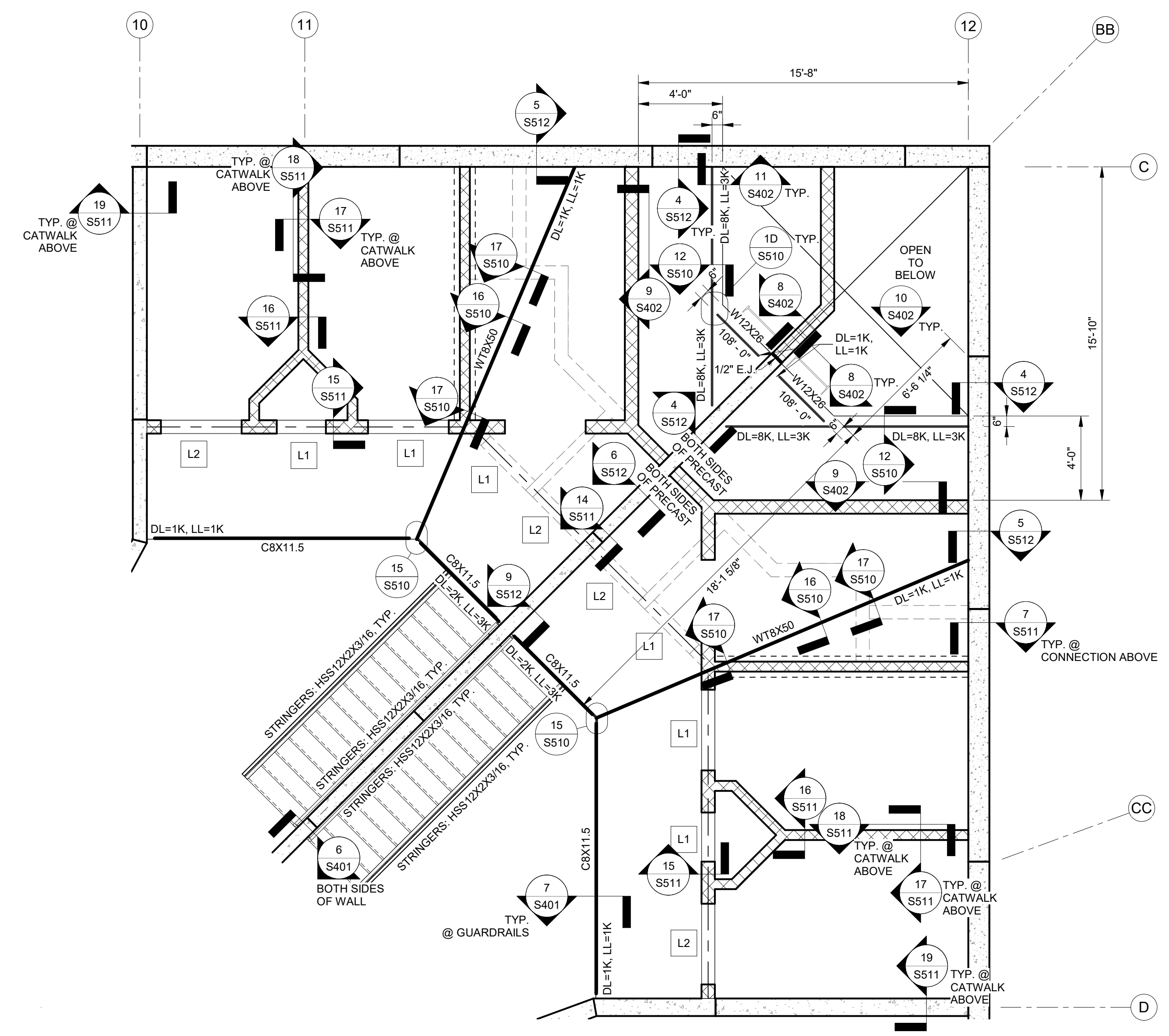
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CHECKED: ADL  
 CHECK DATE: 03/01/2024

SHEET TITLE

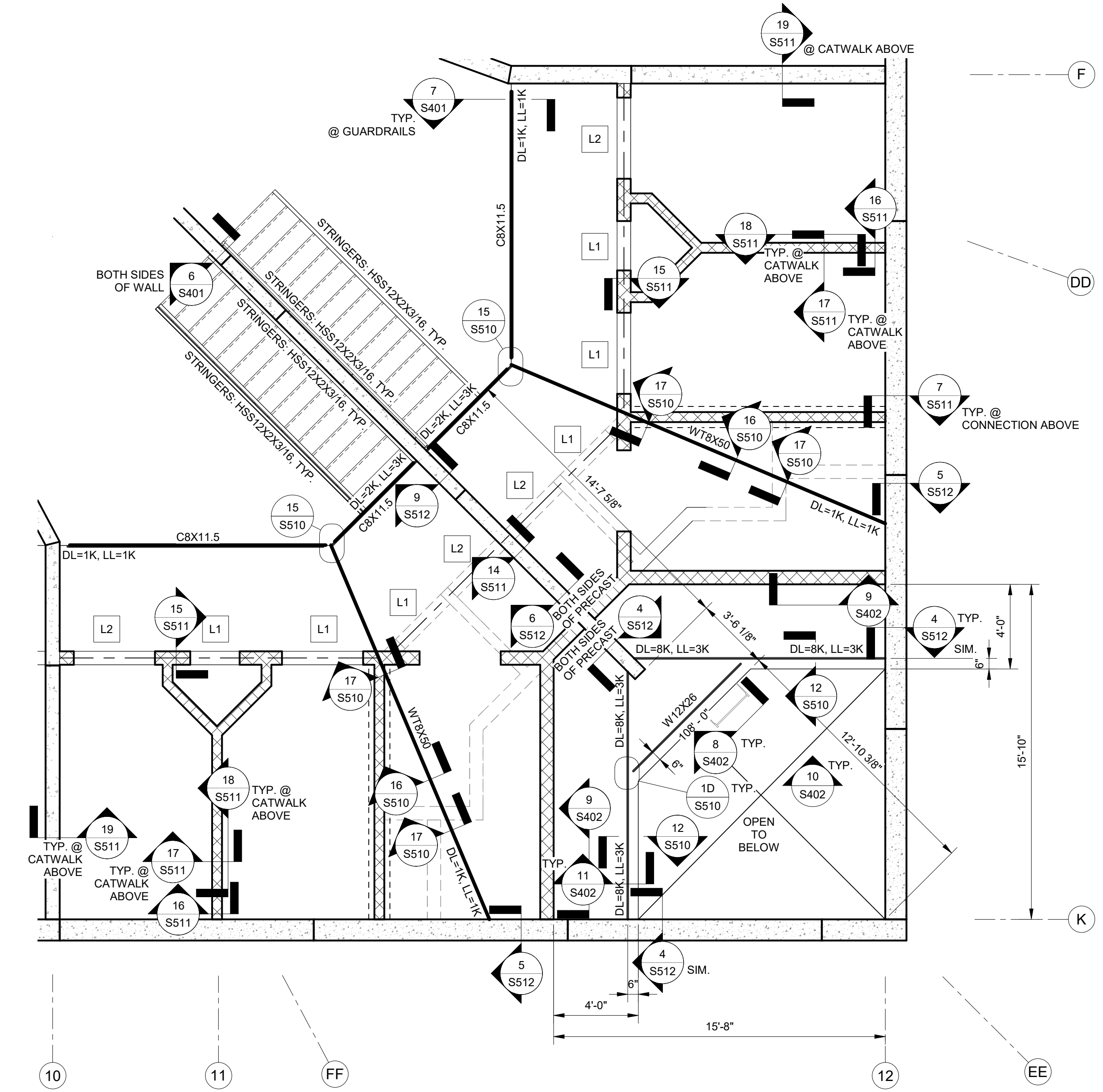
ENLARGED MEZZANINE FRAMING PLANS

SCALE: 1/4" = 1'-0"



**1 ENLARGED MEZZANINE PLAN**  
 1/4" = 1'-0"

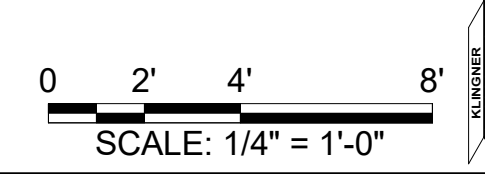
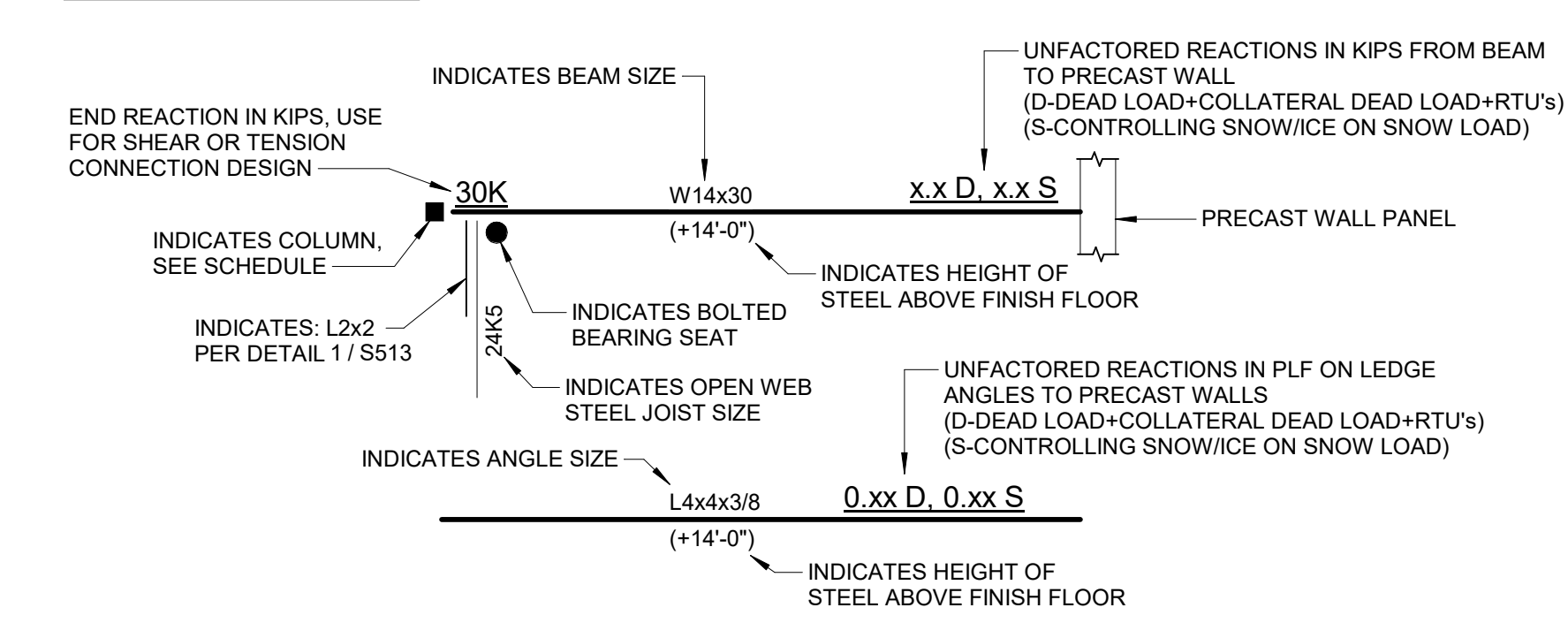
NOTE: SEE ARCHITECTURE PLANS FOR CATWALK ABOVE.

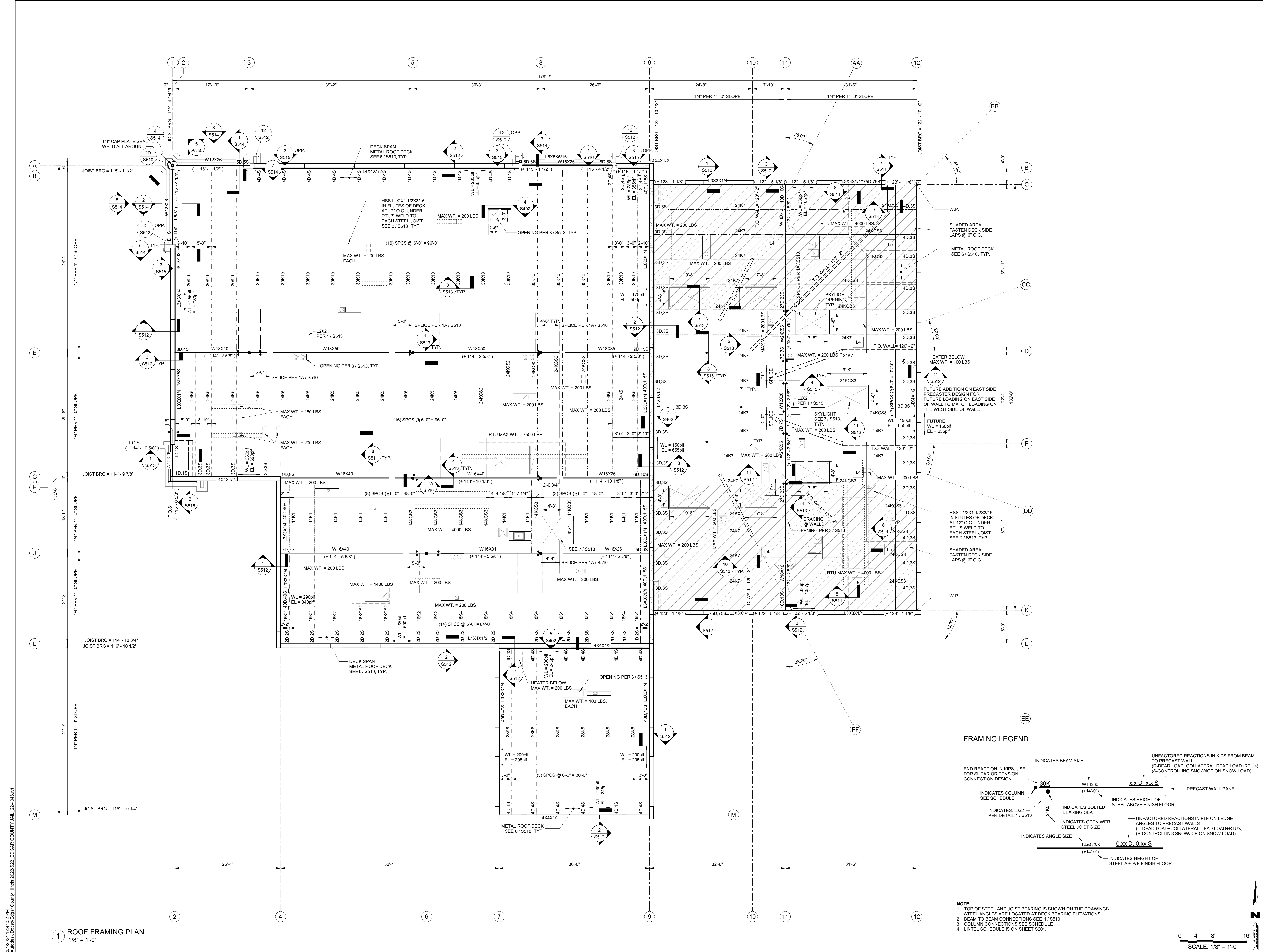


**2 ENLARGED MEZZANINE PLAN**  
 1/4" = 1'-0"

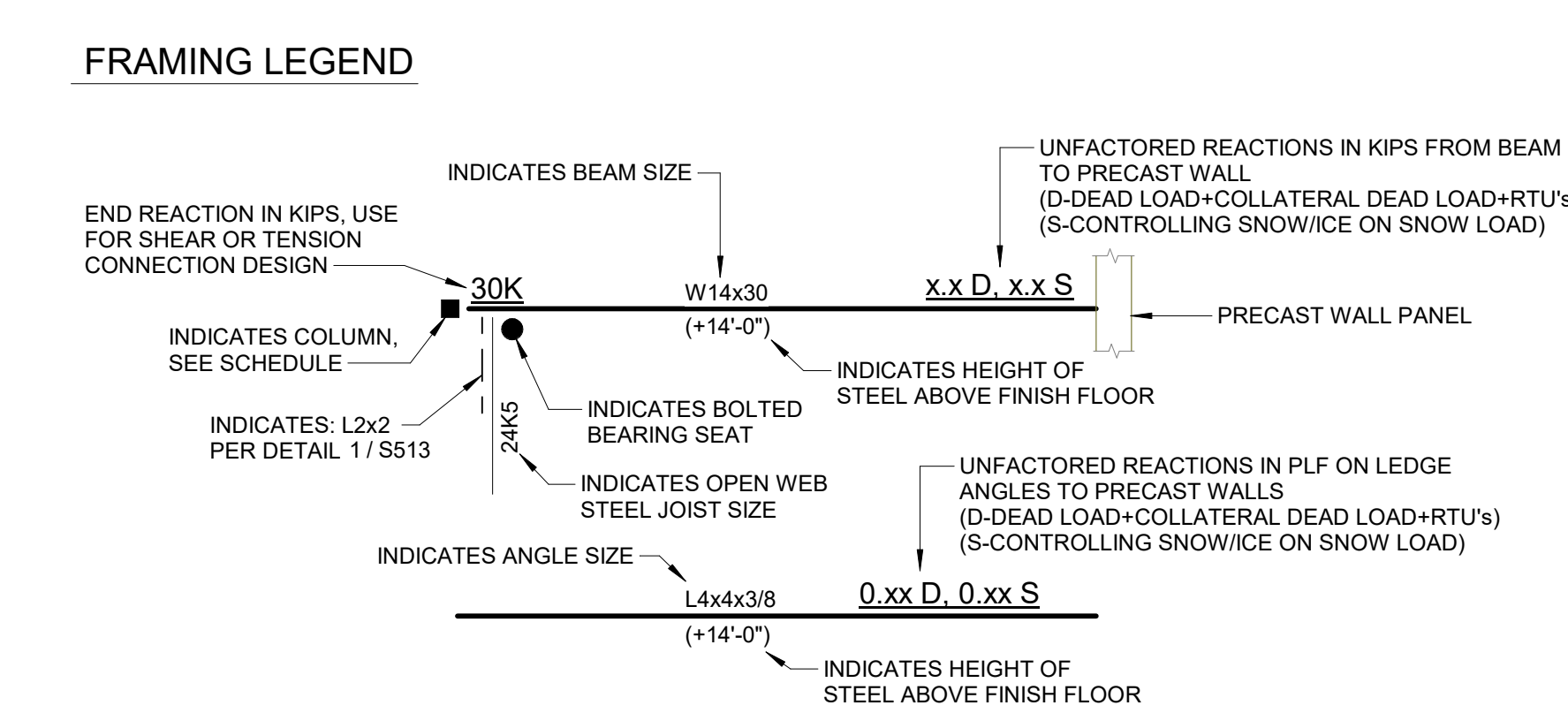
NOTE: SEE ARCHITECTURE PLANS FOR CATWALK ABOVE.

**FRAMING LEGEND**

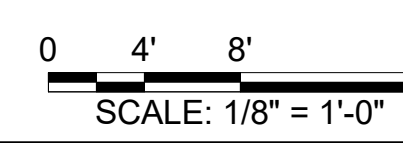




1 ROOF FRAMING PLAN  
1/8" = 1'-0"



- NOTE:**
1. TOP OF STEEL AND JOIST BEARING IS SHOWN ON THE DRAWINGS. STEEL ANGLES ARE LOCATED AT DECK BEARING ELEVATIONS.
  2. BEAM TO BEAM CONNECTIONS SEE 1 / S510
  3. COLUMN CONNECTIONS SEE SCHEDULE
  4. LINTEL SCHEDULE IS ON SHEET S201.



REVISION HISTORY

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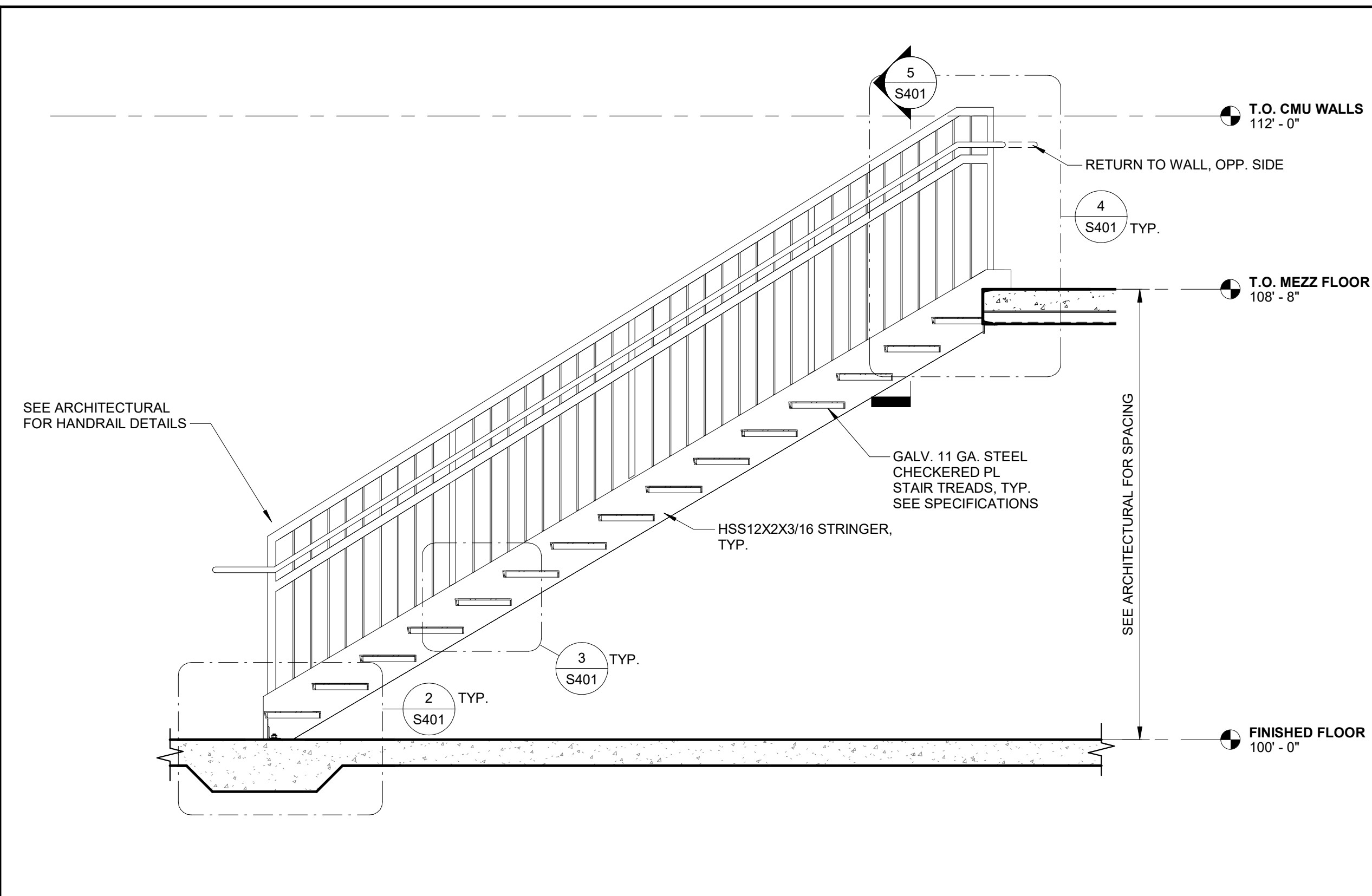
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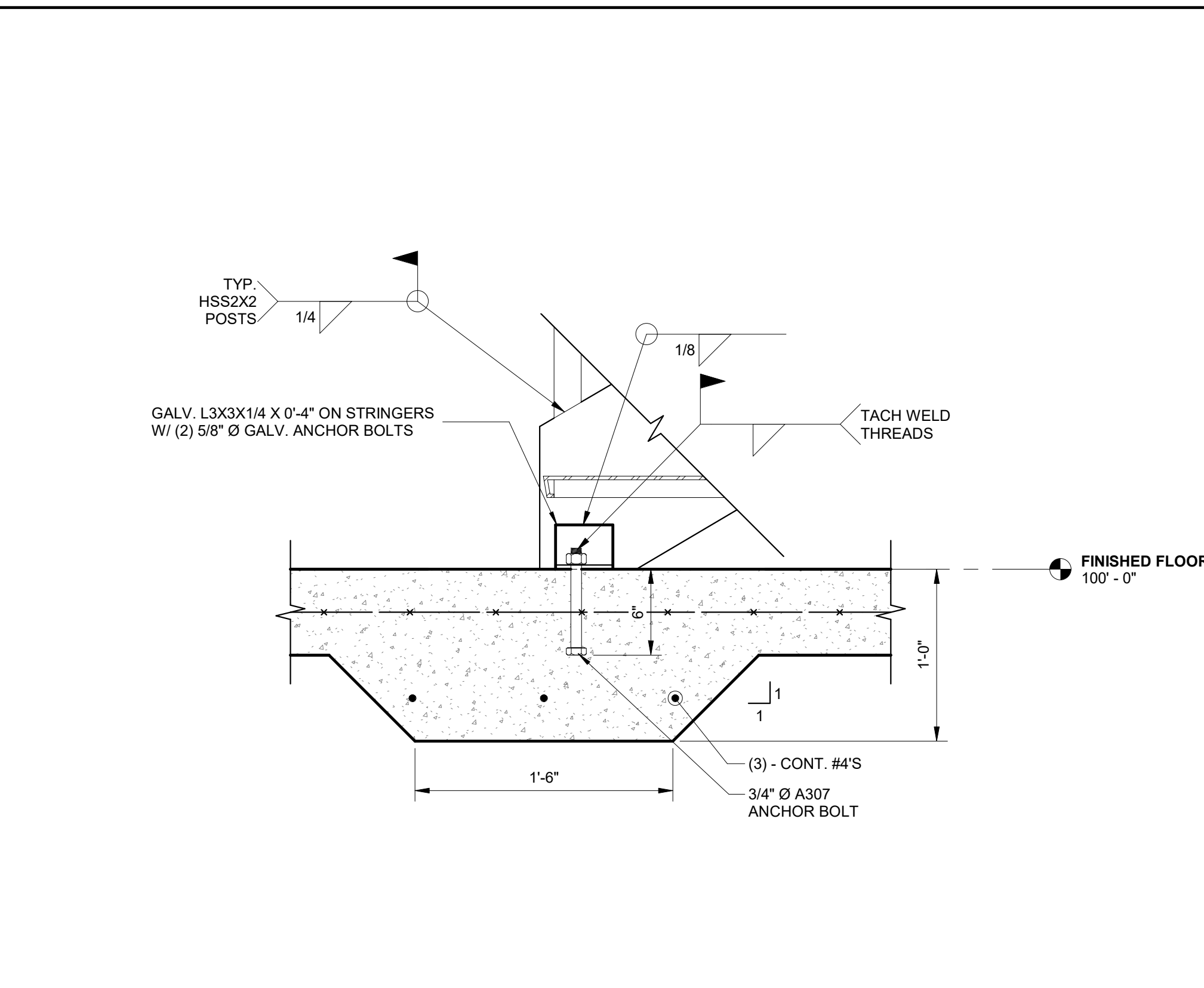
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DRAWN	ADL	NML
CHECKED	ADL	ADL
DATE	03/01/2024	
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PROJECT NO. 22-4048		
DRAWING ISSUED DATE: 03/01/2024		
SHEET		
S203		

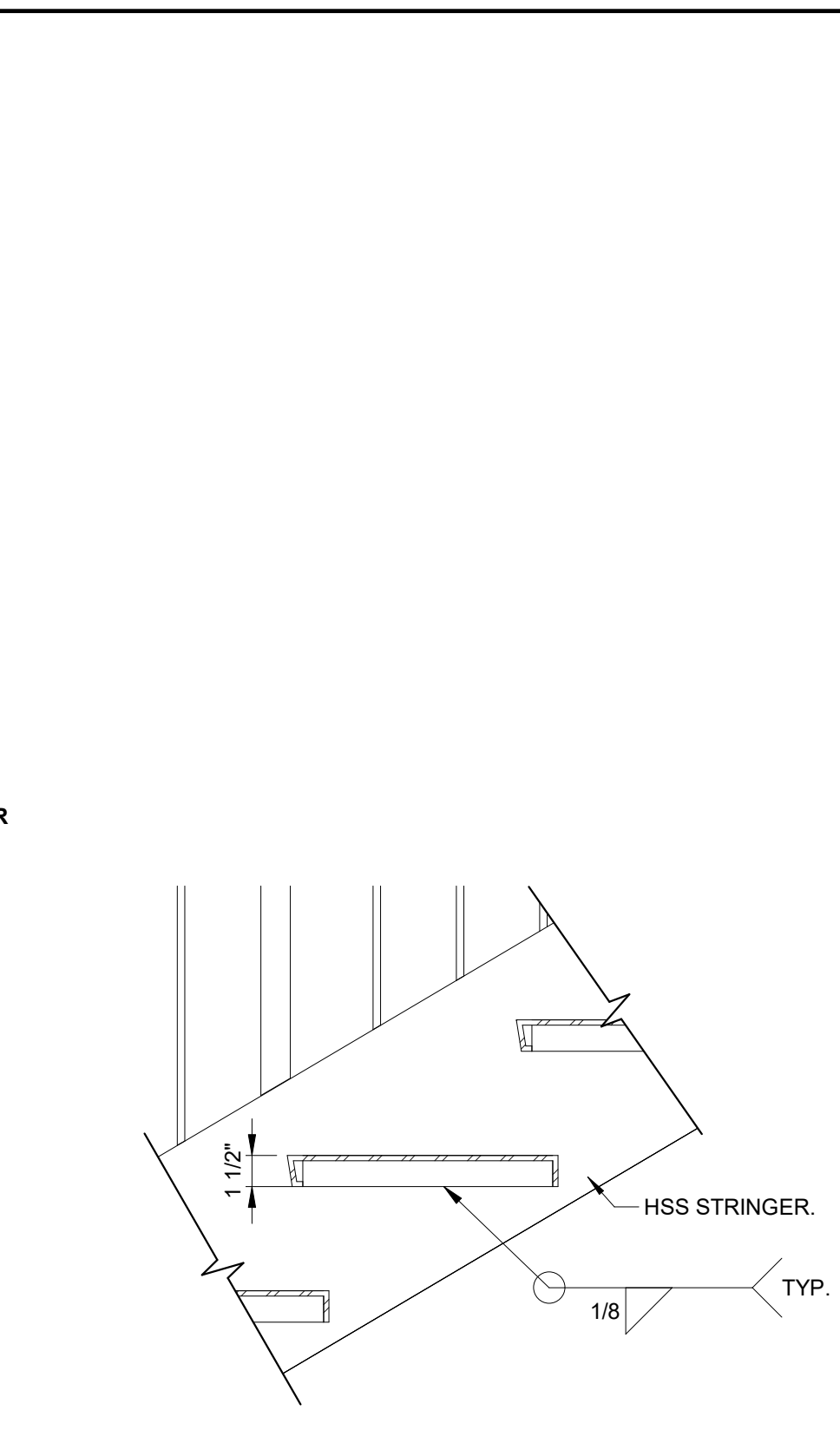
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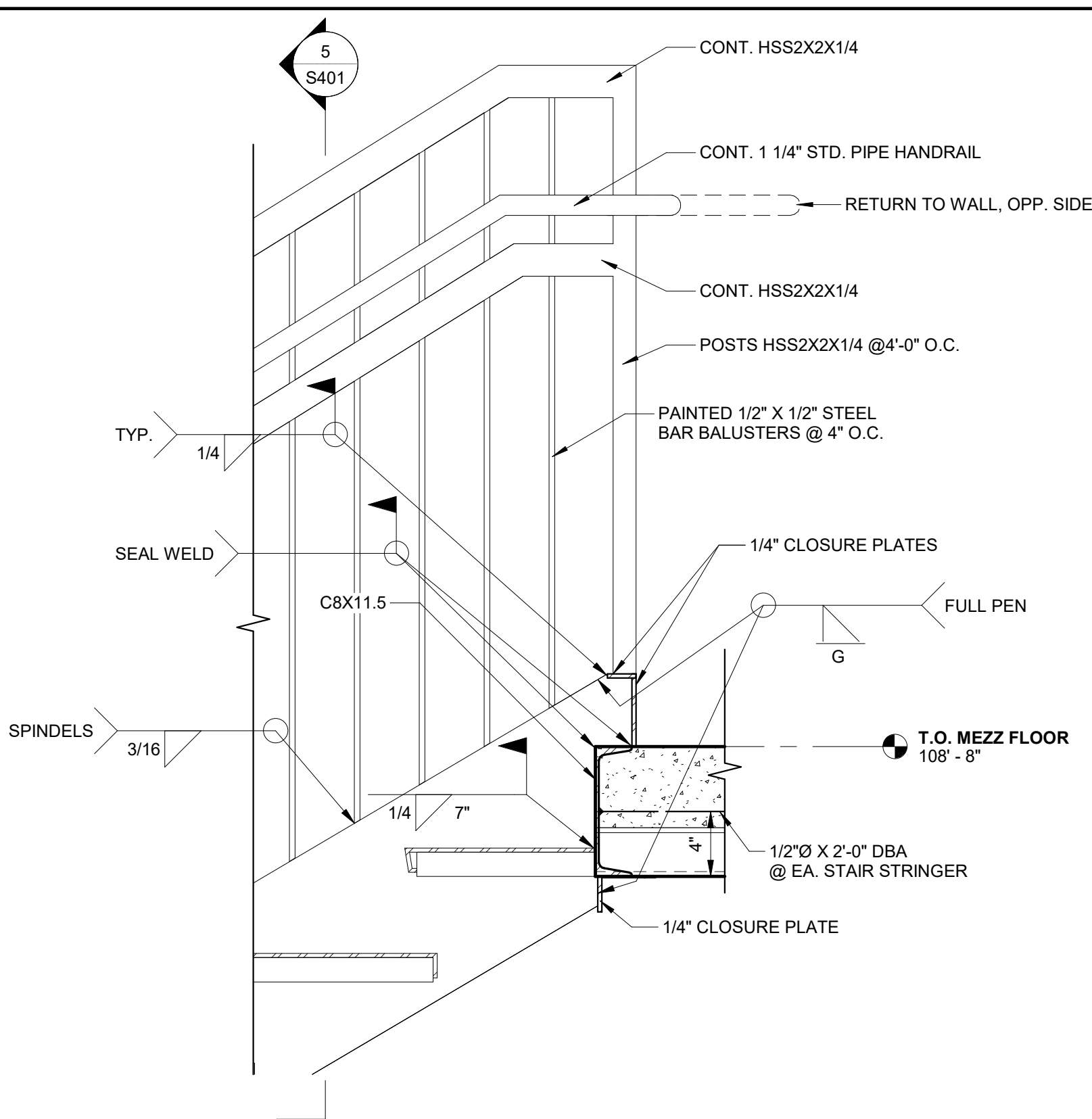
1 TYPICAL STAIR SECTION  
1/2" = 1'-0"



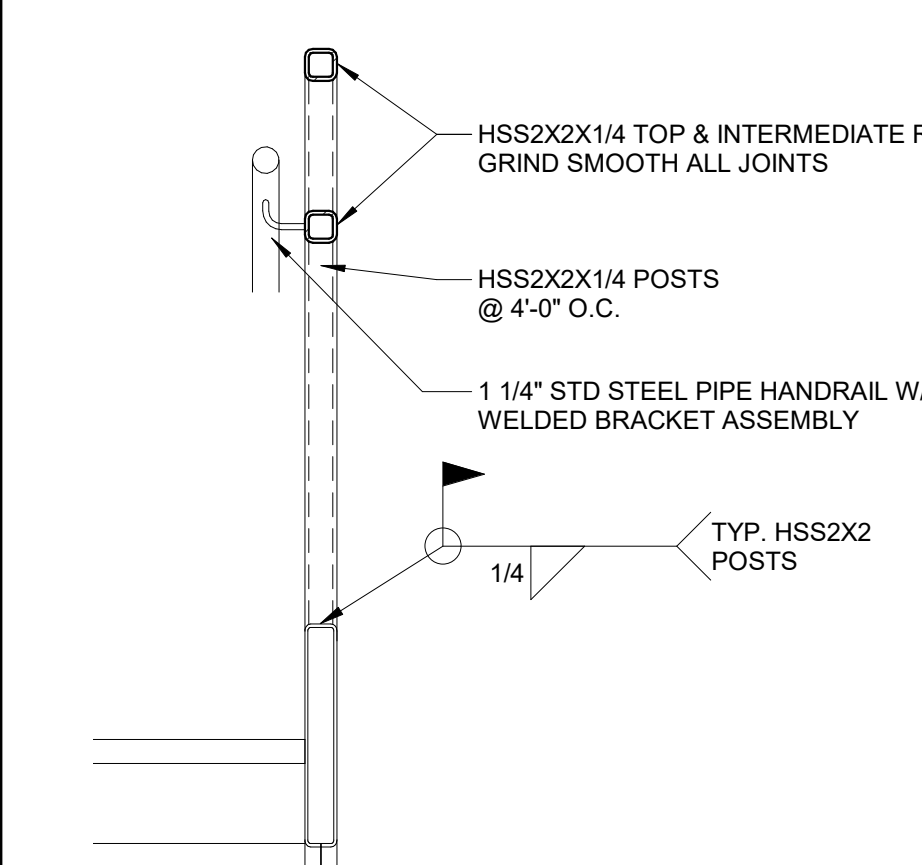
2 STAIR TO SLAB CONNECTION  
1 1/2" = 1'-0"



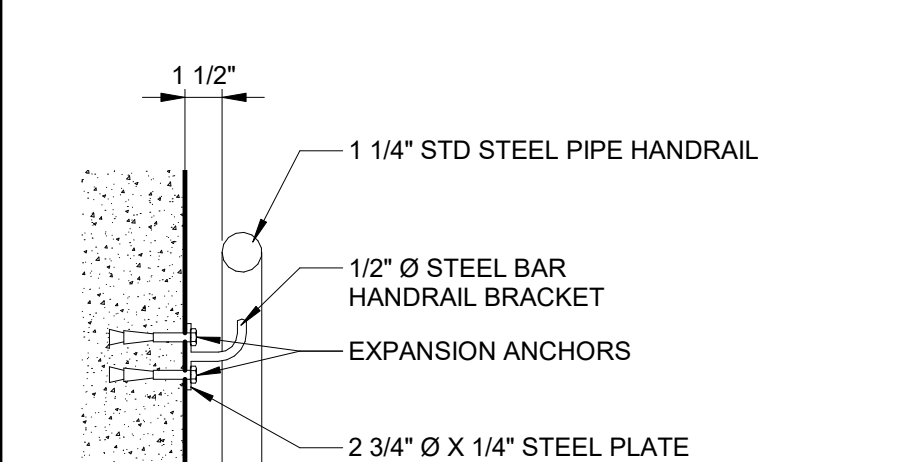
3 TYPICAL STAIR DETAIL  
1 1/2" = 1'-0"



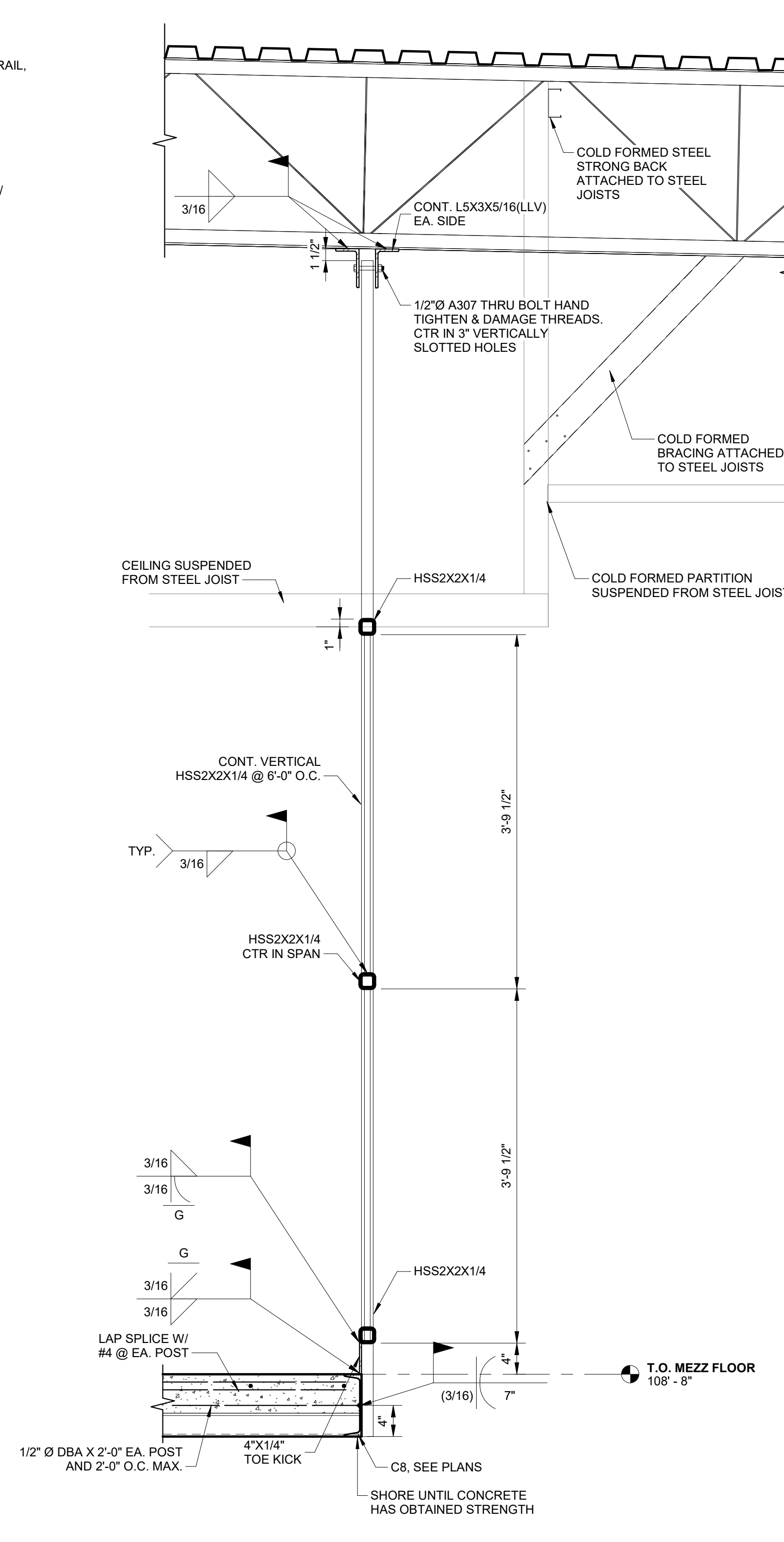
4 STAIR CONNECTION TO MEZZANINE FLOOR  
1 1/2" = 1'-0"



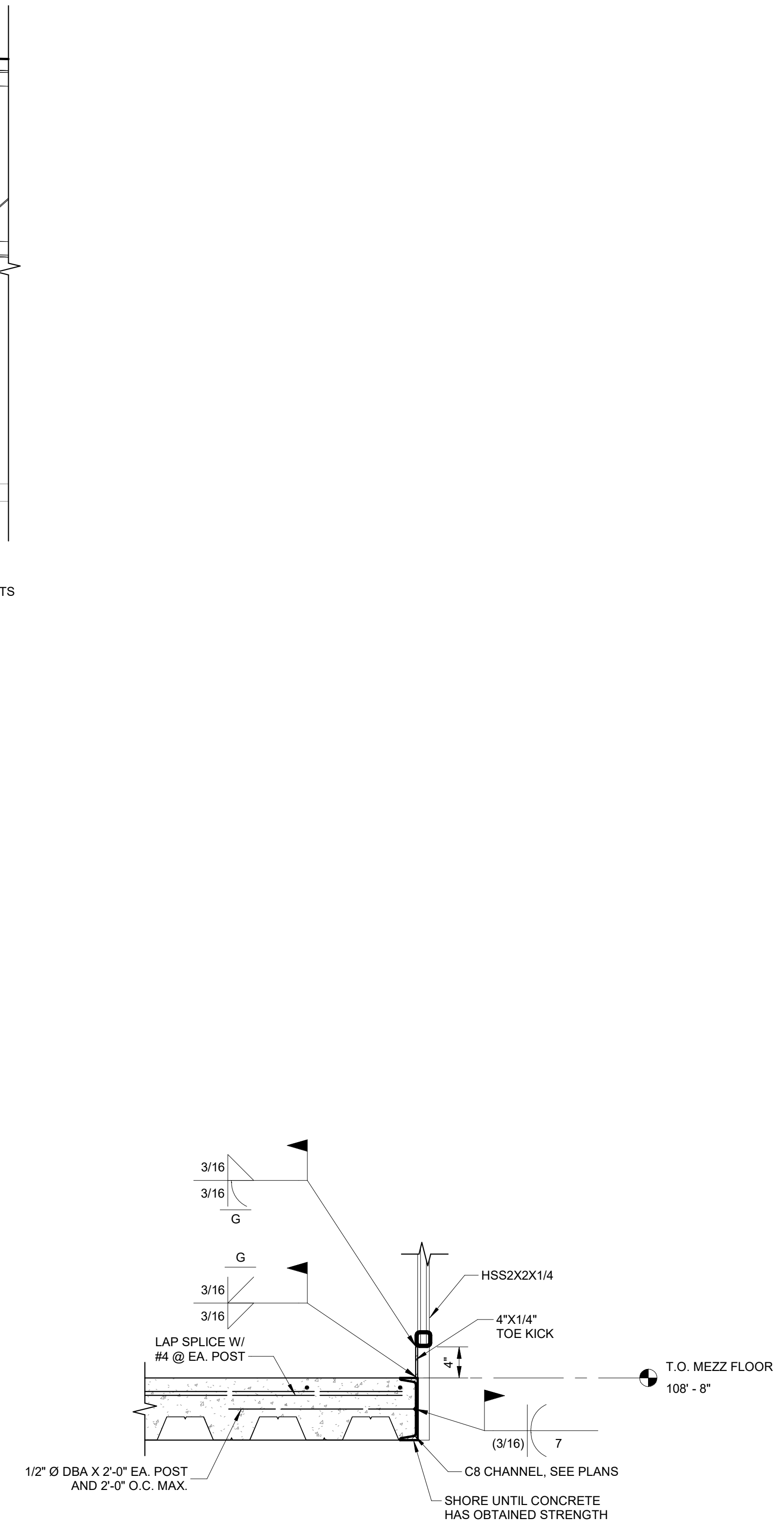
5 STAIR HANDRAIL  
1" = 1'-0"



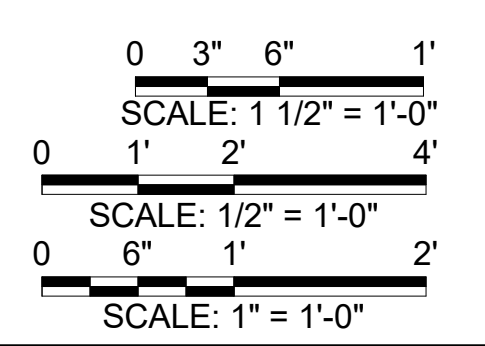
6 STAIR HANDRAIL @ WALLS  
1 1/2" = 1'-0"



7 GUARDRAIL SECTION  
1" = 1'-0"



8 GUARDRAIL SECTION NON BRG.  
1" = 1'-0"



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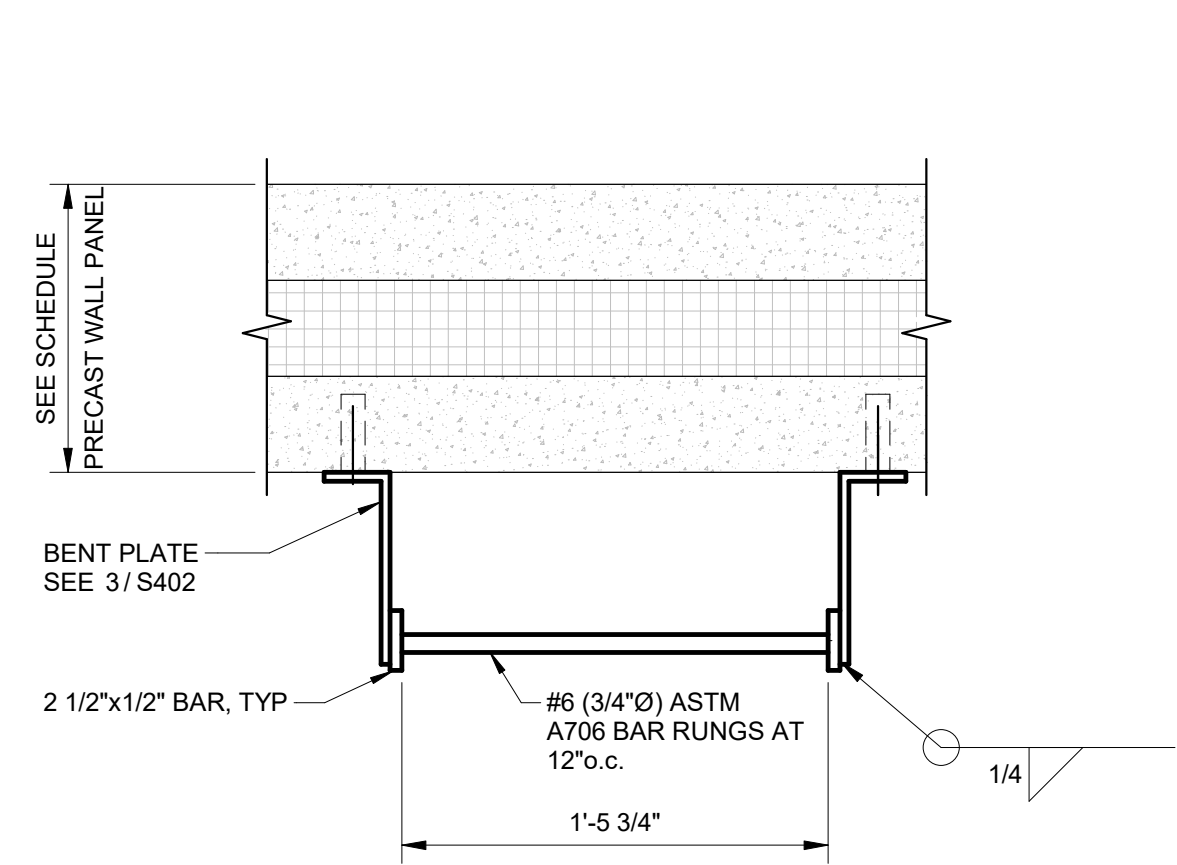
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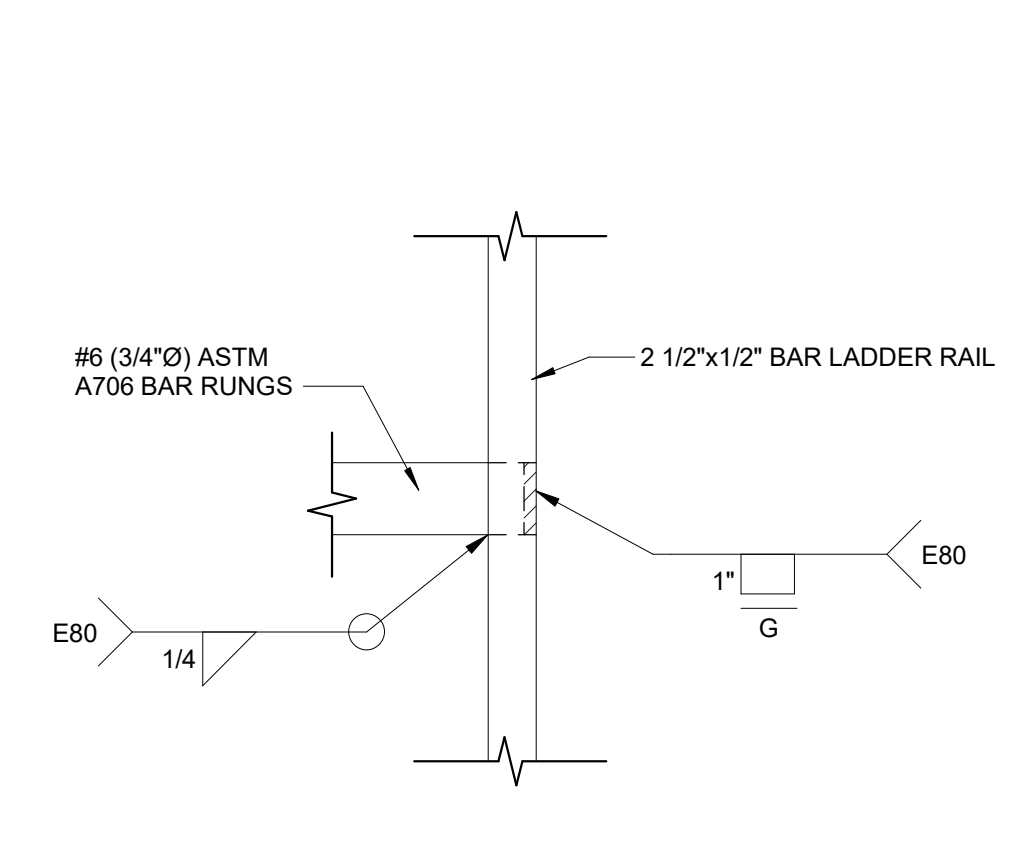
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 PROJECT NO.  
 22-4048  
 DRAWING ISSUED DATE:  
 03/01/2024  
**S401**

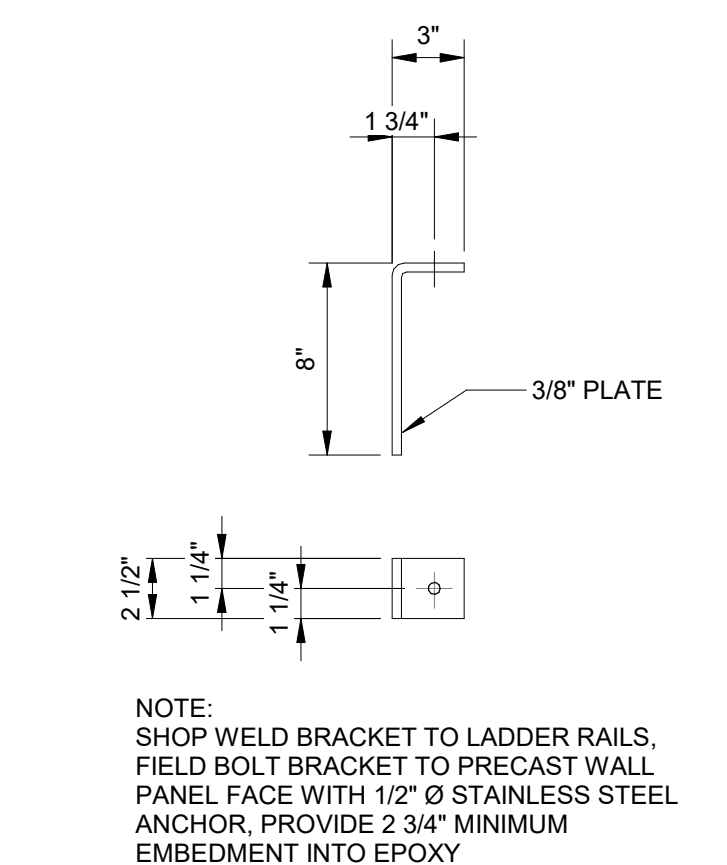




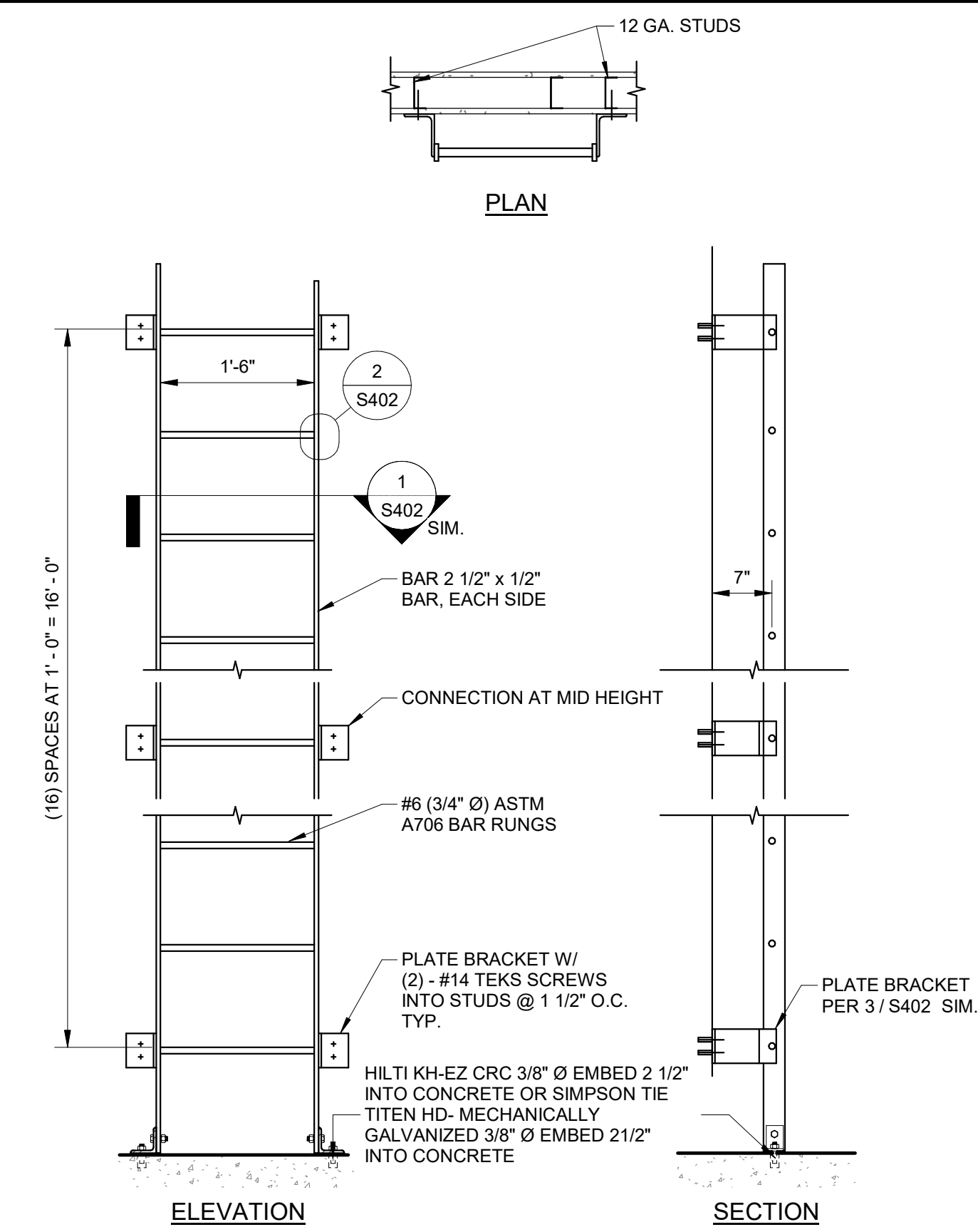
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1 1/2" = 1'-0"



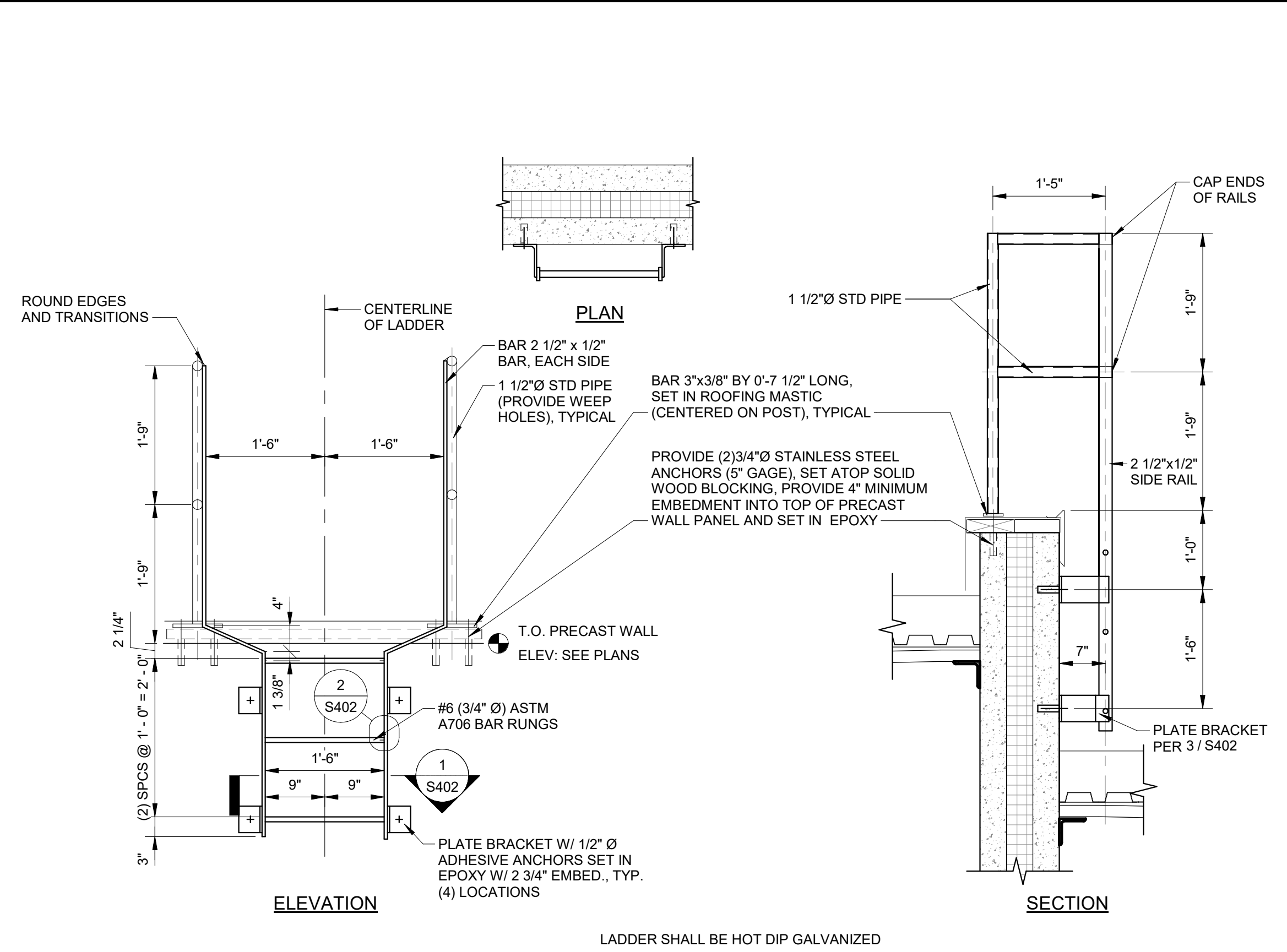
2 LADDER RUNG DETAIL  
6" = 1'-0"



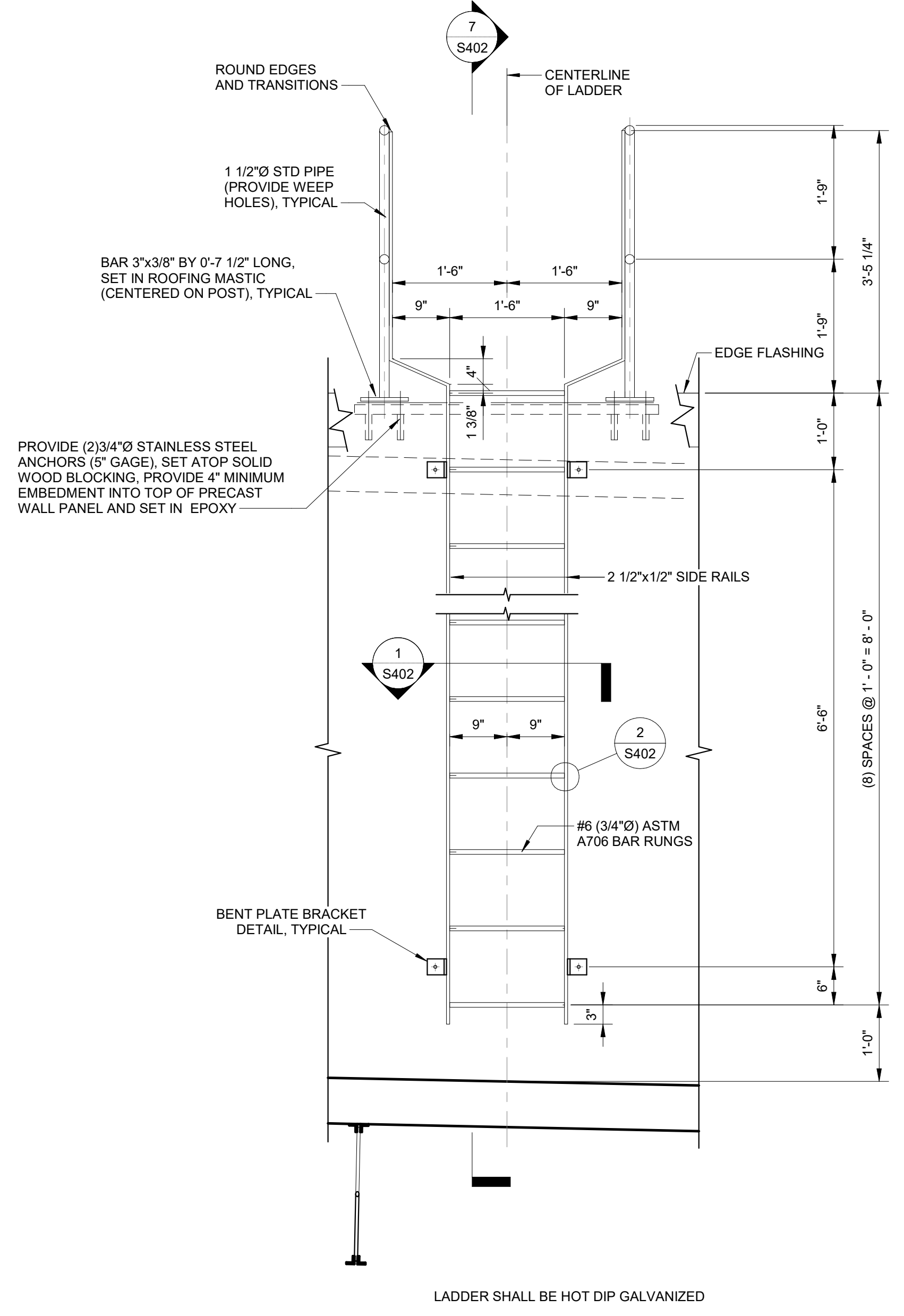
3 BENT PLATE BRACKET DETAIL  
1 1/2" = 1'-0"



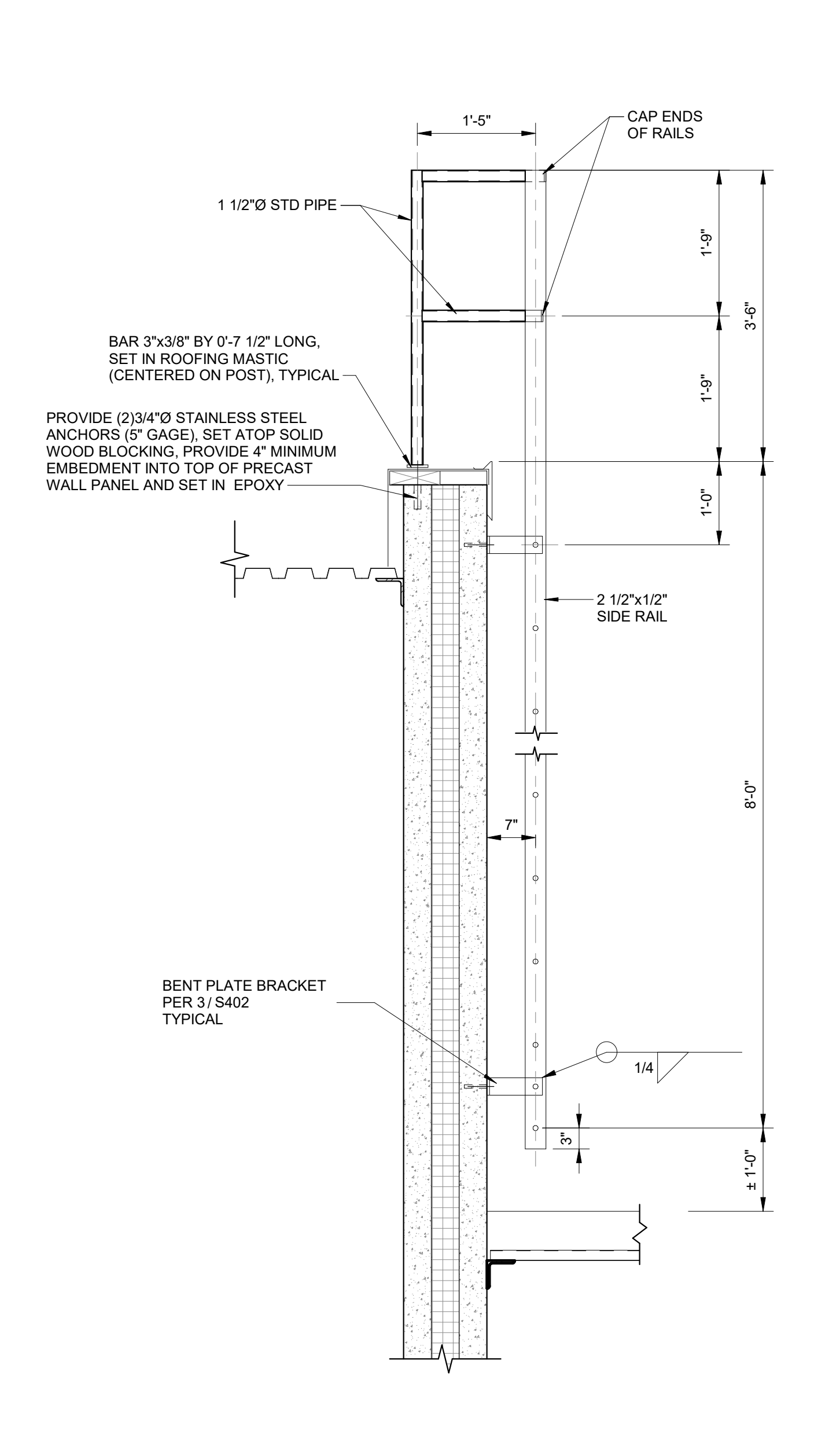
4 LADDER DETAIL  
3/4" = 1'-0"



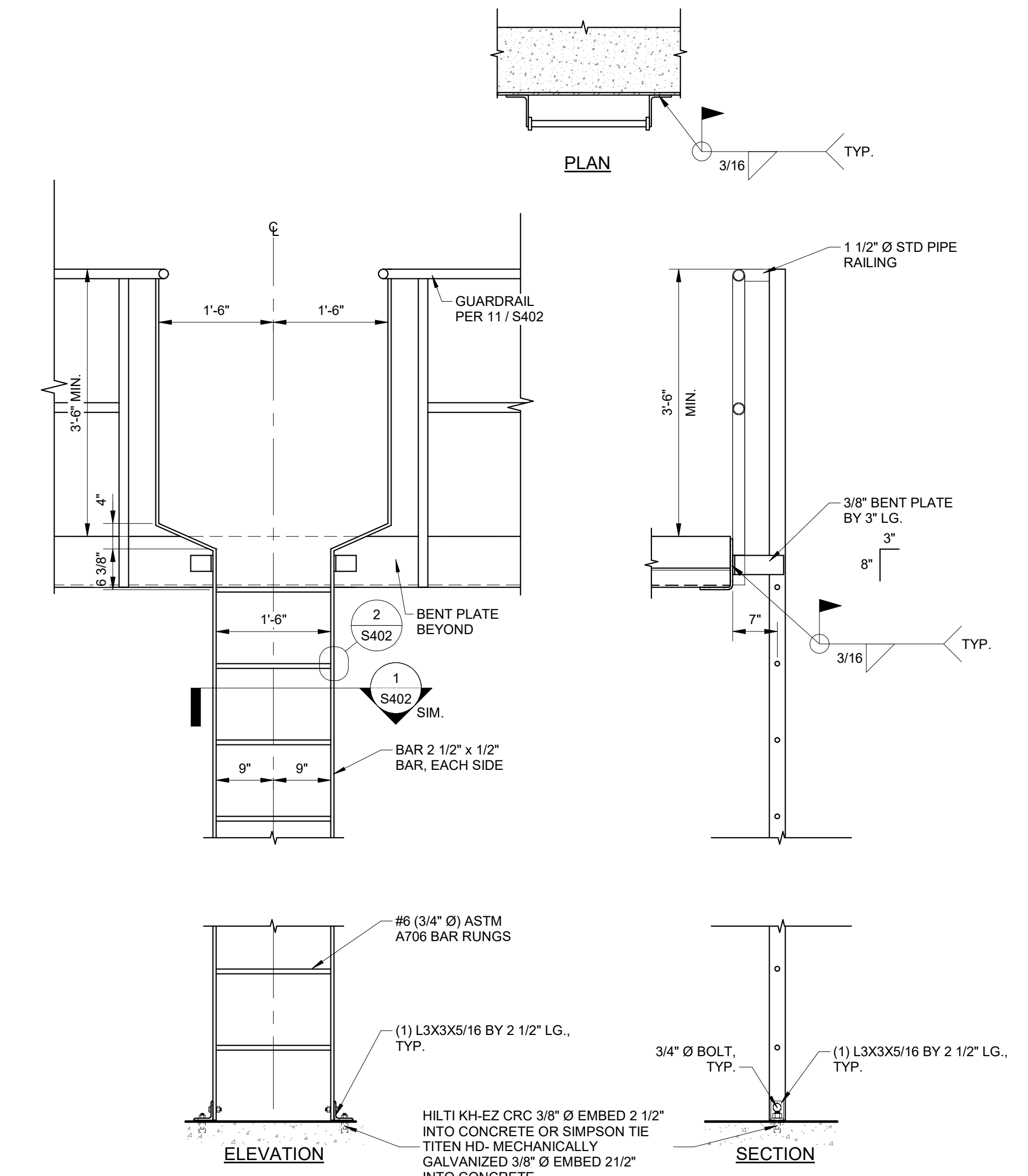
5 LADDER DETAIL  
3/4" = 1'-0"



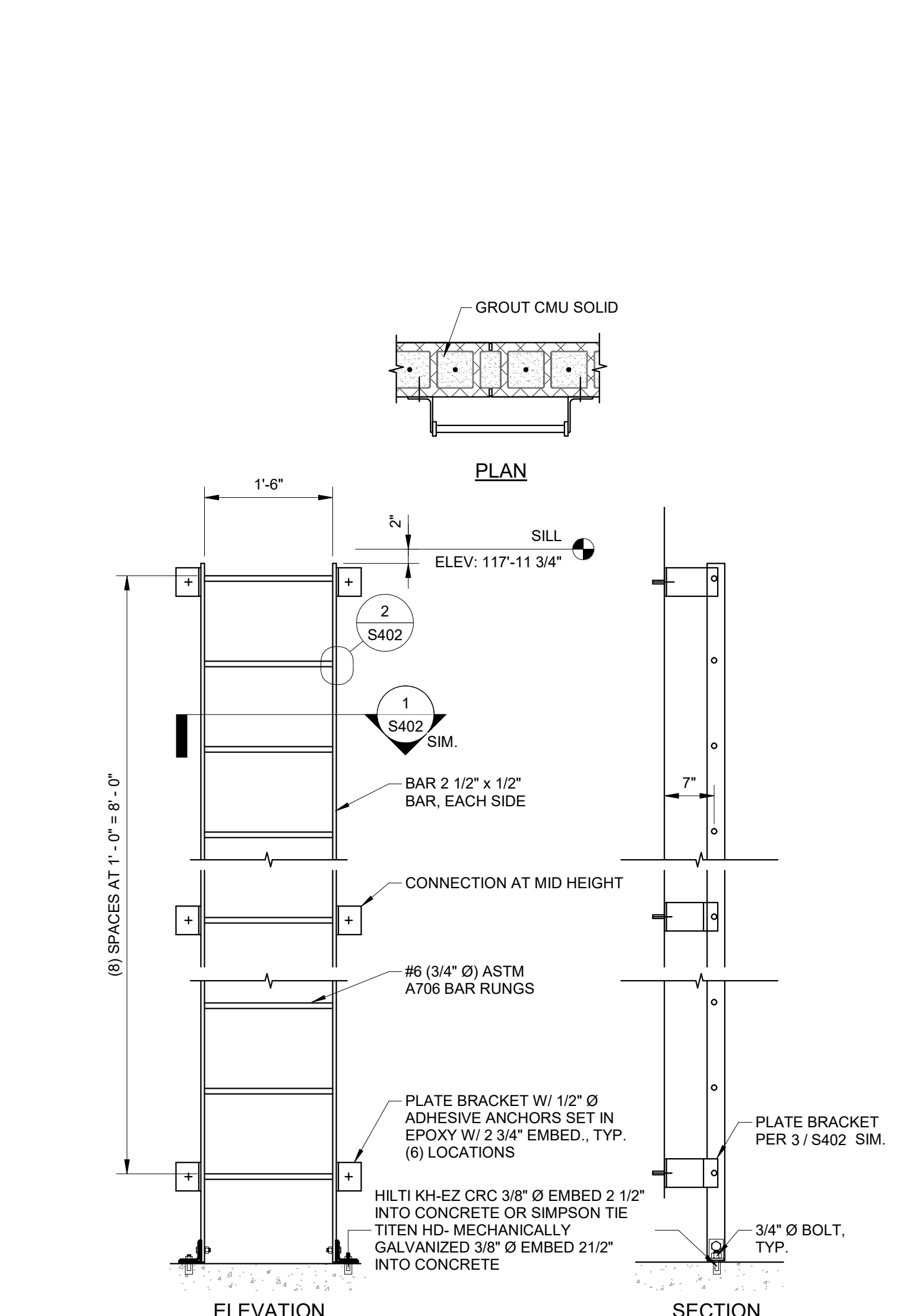
6 ROOF LADDER ELEVATION  
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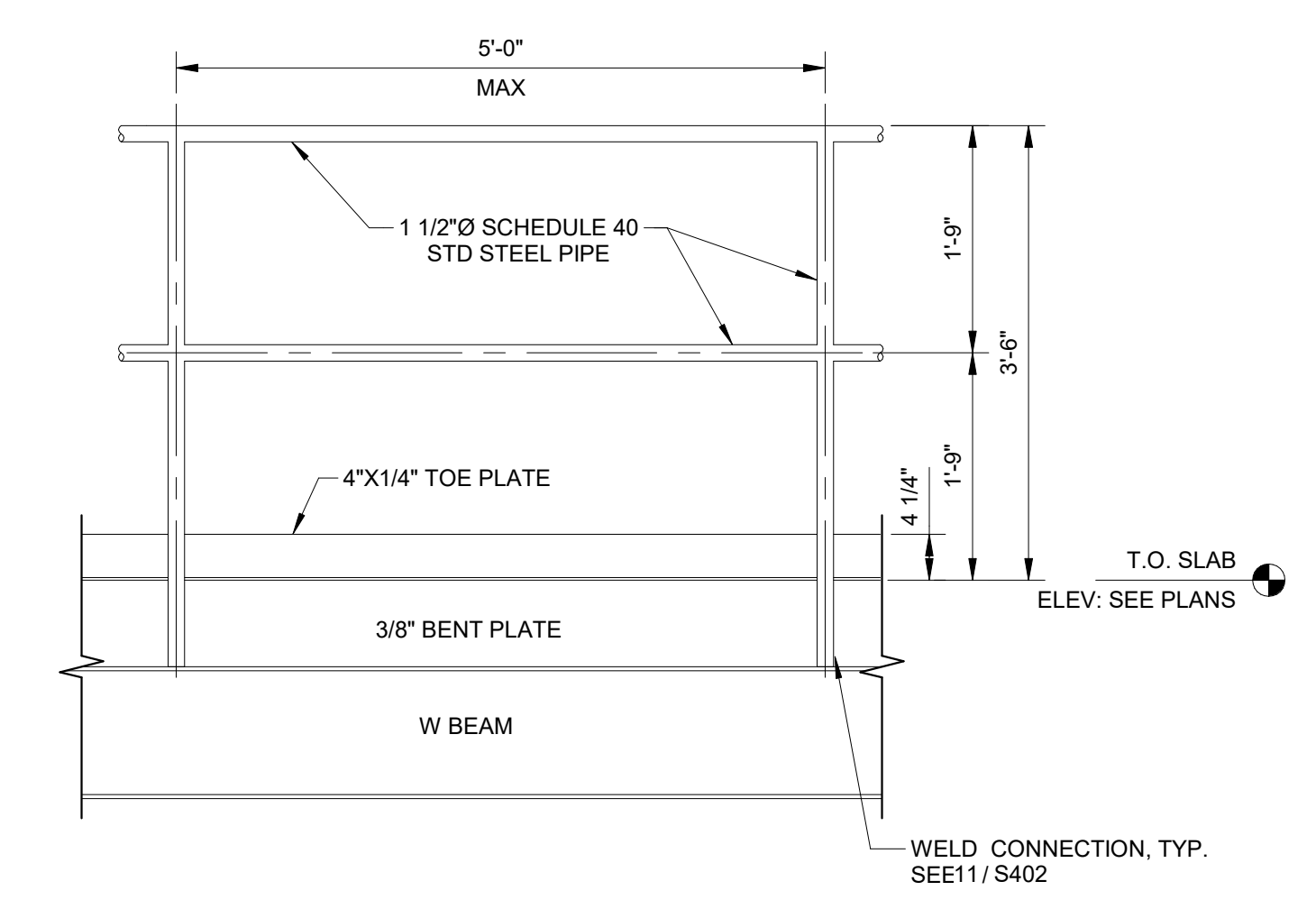
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3/4" = 1'-0"



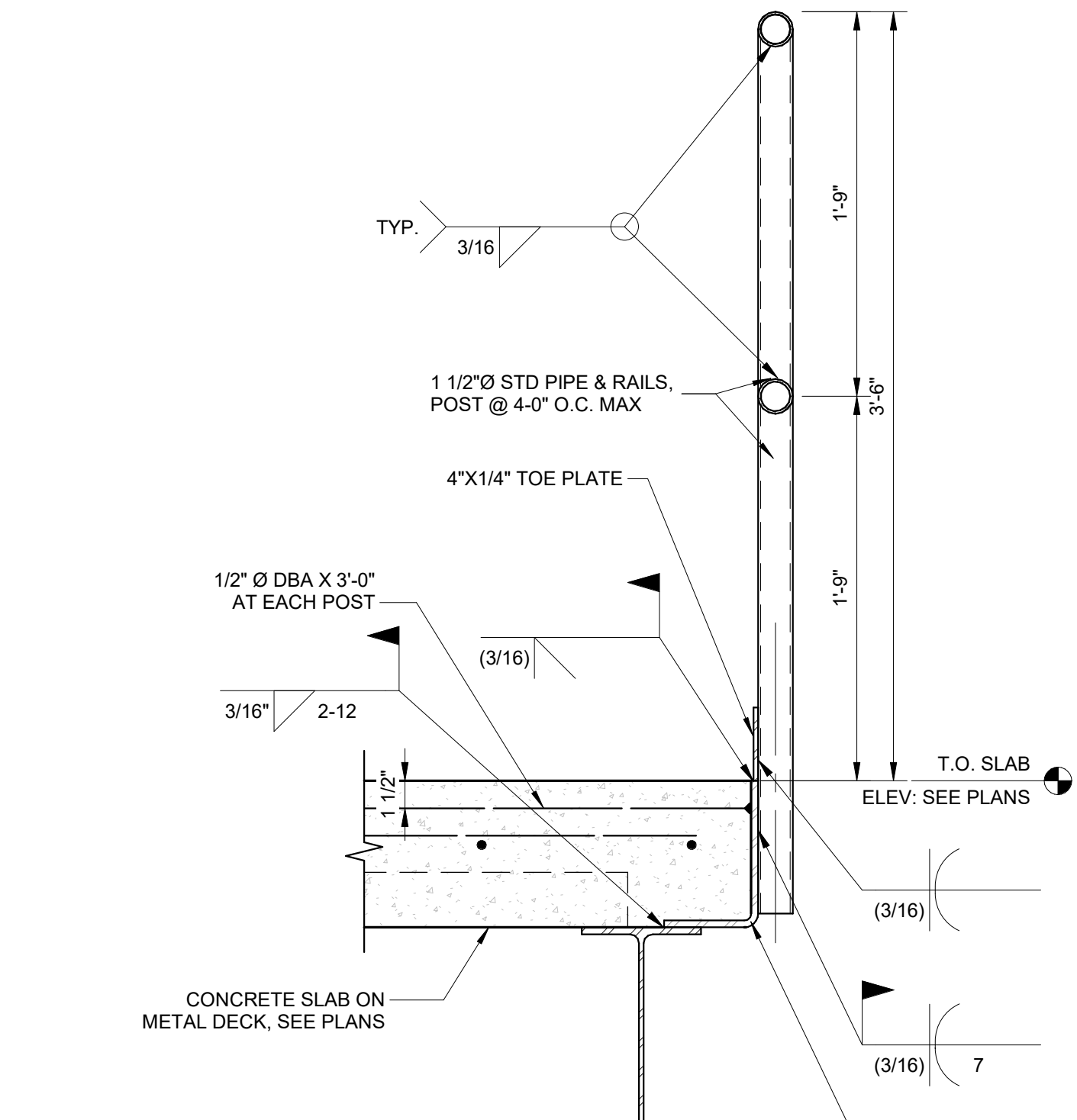
8 LADDER DETAIL  
3/4" = 1'-0"



9 LADDER DETAIL  
3/4" = 1'-0"



10 GUARDRAIL ELEVATION  
3/4" = 1'-0"



11 GUARDRAIL WELD DETAIL  
1 1/2" = 1'-0"

REVISION HISTORY

NO.	DESCRIPTION	DATE	APPR.
1	ISSUED FOR	03/01/2024	

ISSUED FOR: 03/01/2024  
**BID & PERMIT**

**EDGAR COUNTY PUBLIC SAFETY CENTER**  
 EDGAR COUNTY, ILLINOIS  
 12636 950TH ROAD  
 PARIS, ILLINOIS 61944

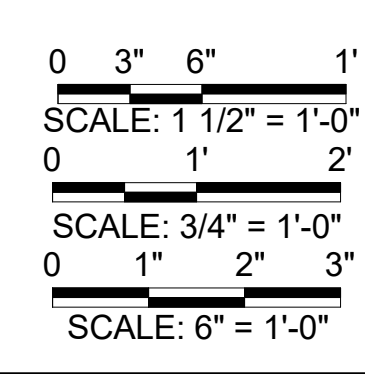
Non-Reduced Sheet Size 30" x 42"  
 Full sized plans have been prepared using standard scales.  
 Reduced sized plans may not conform to standard scales.

DESIGNED	DRAWN
JTS	NML
FIELD BOOK	FIELD BOOK

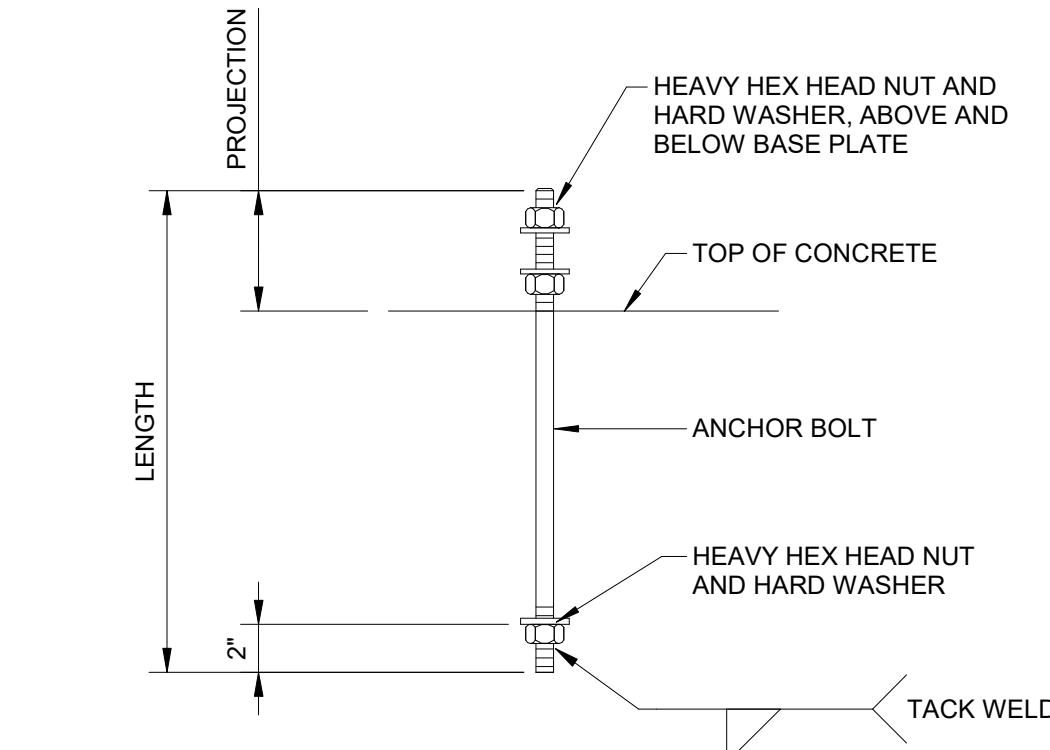
CHECKED	CHECK DATE
ADL	03/01/2024

SHEET TITLE

**LADDER SECTIONS**  
 PROJECT NO. 22-4048  
 DRAWING ISSUED DATE: 03/01/2024  
**S402**

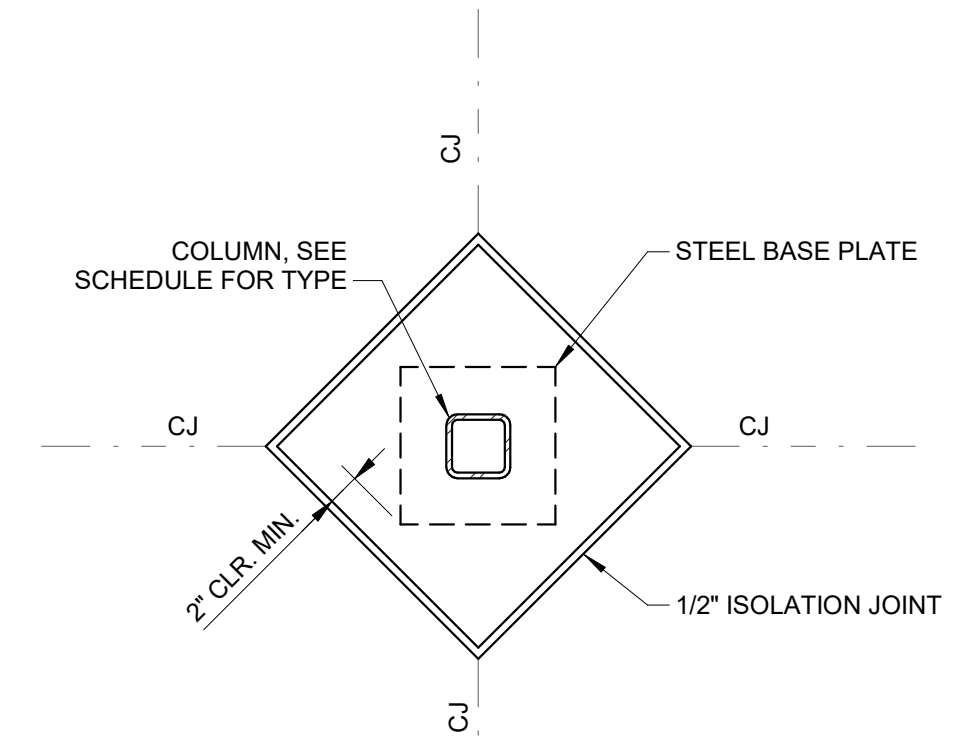


3/1/2024 12:41:56 PM Autodesk Docs\Edgar County Illinois 2022\S22 EDGAR COUNTY JAIL 22-04048.rvt

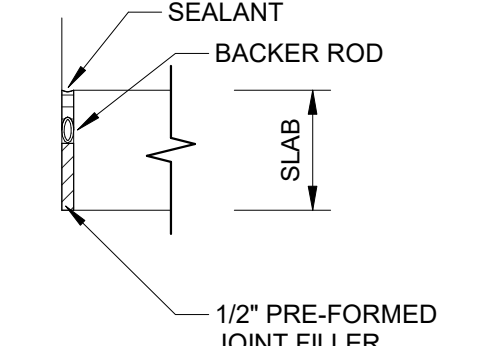


SIZE	LENGTH	PROJECTION
3/4"Ø ASTM F1554 36KSI	1'-3"	6"

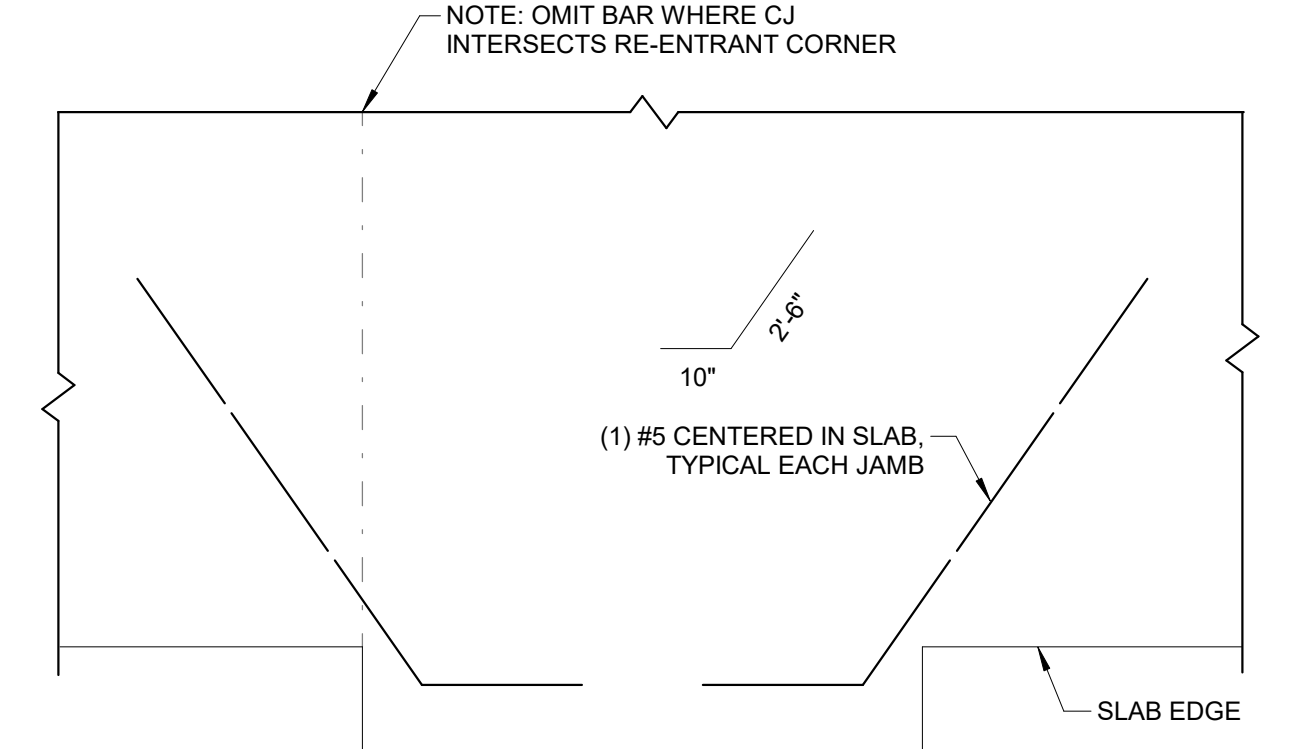
1 ANCHOR BOLT DETAIL  
1 1/2" = 1'-0"



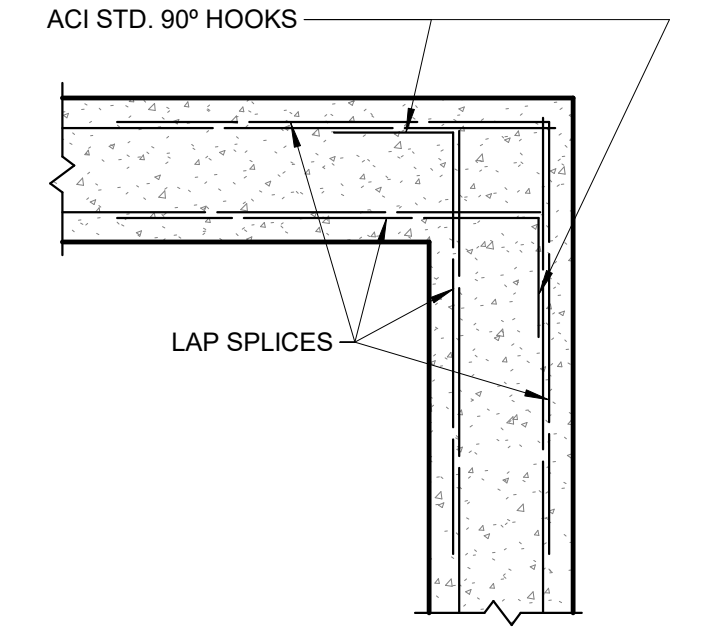
2 ISOLATION AT COLUMN SQUARE  
1" = 1'-0"



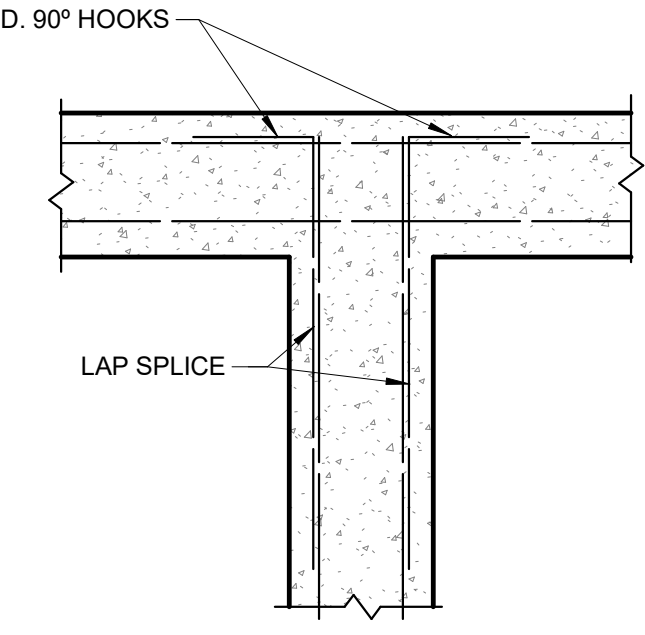
3 EXPANSION JOINT-SLAB TO WALL  
1 1/2" = 1'-0"



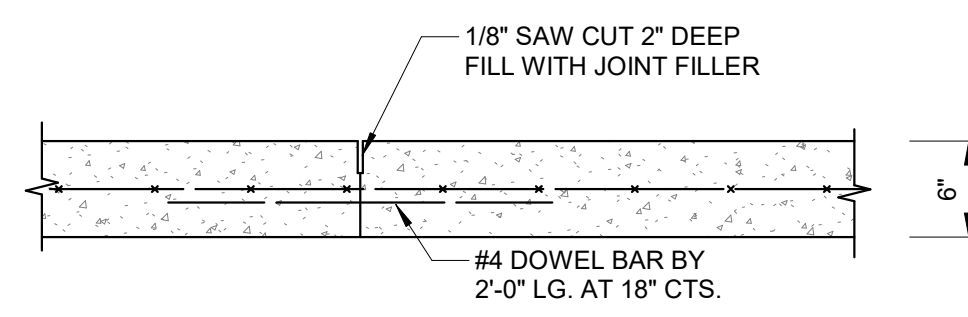
4 REINF AT DOOR OPENING  
1" = 1'-0"



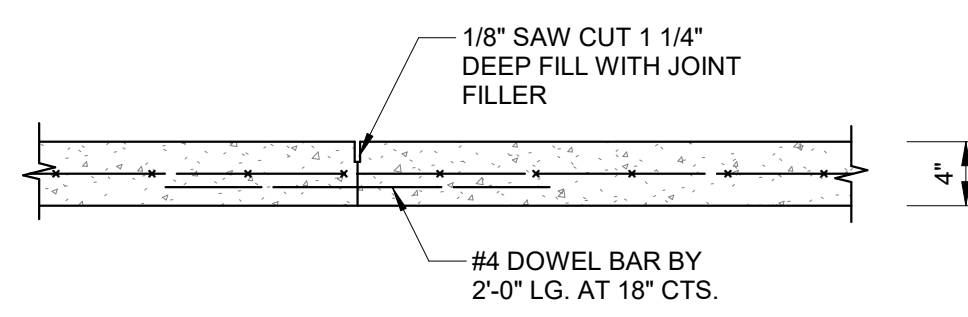
90° CORNER FTG., BEAM, AND WALL REINF. EACH FACE



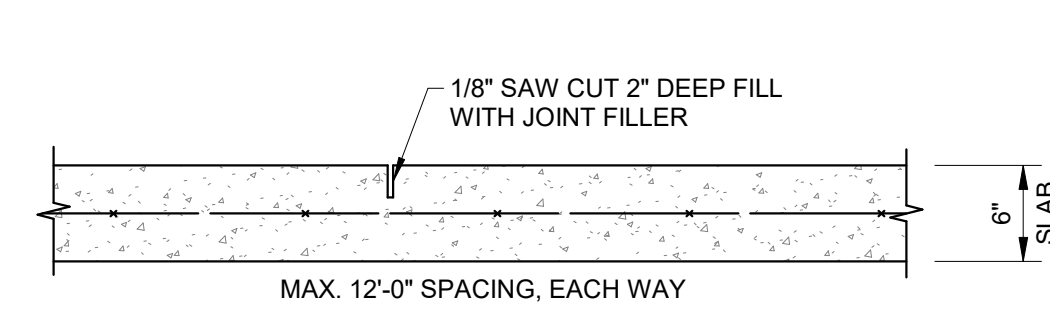
TEE CORNER FTG., BEAM, AND WALL REINF. EACH FACE



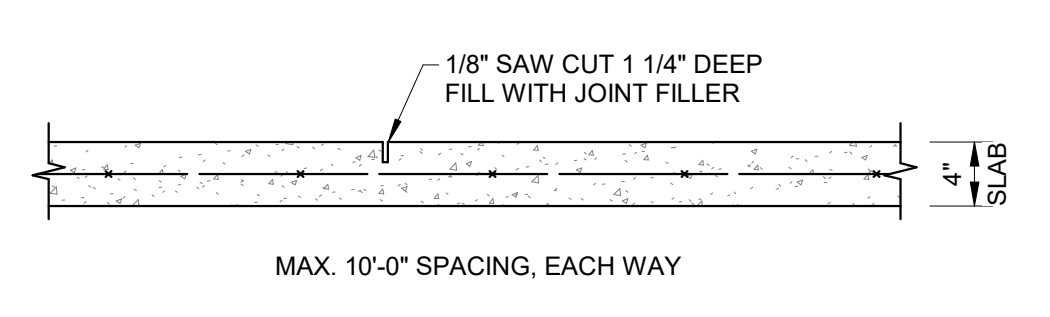
5 FLOOR SLAB CONSTRUCTION JOINT  
1" = 1'-0"



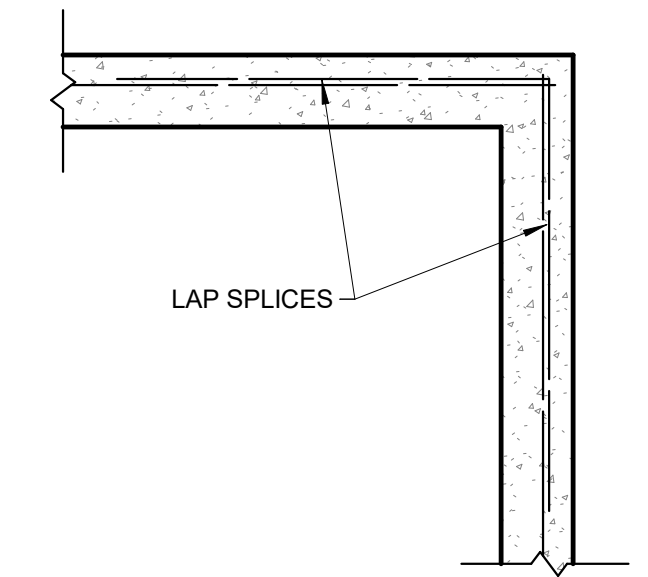
6 FLOOR SLAB CONSTRUCTION JOINT  
1" = 1'-0"



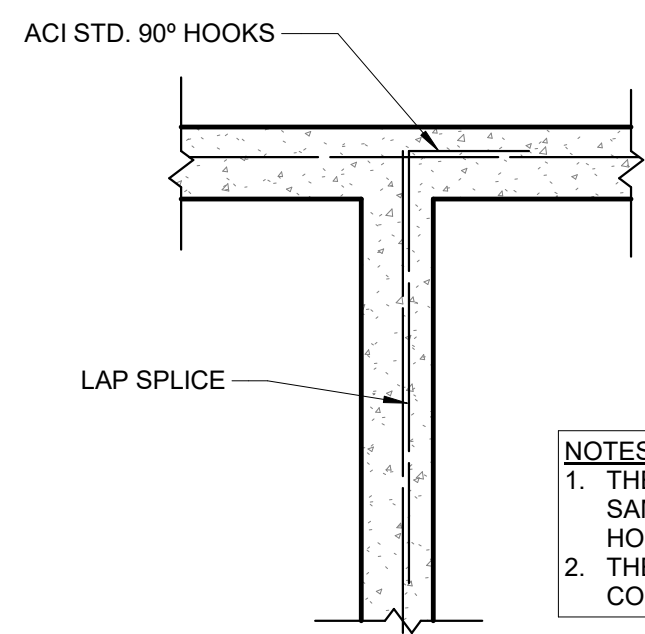
7 FLOOR SLAB CONTRACTION JOINT  
1" = 1'-0"



8 FLOOR SLAB CONTRACTION JOINT  
1" = 1'-0"



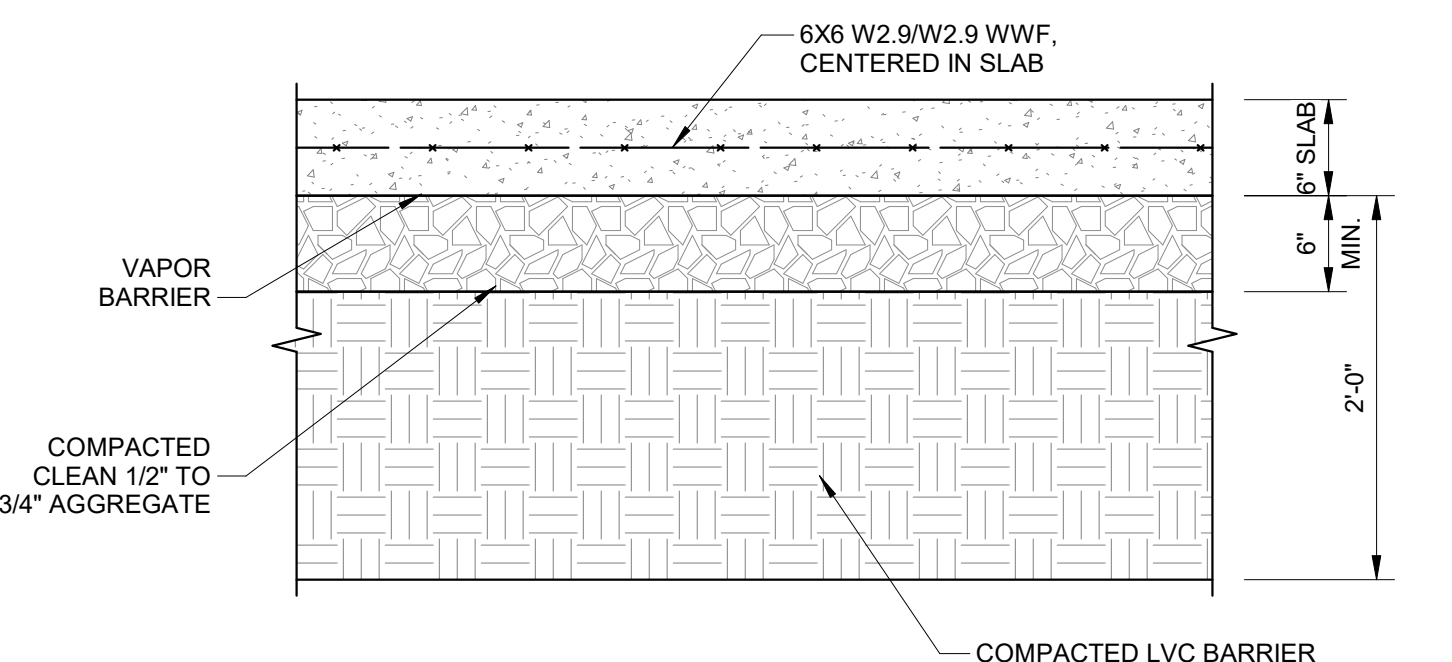
90° CORNER WALL REINF. CENTERED



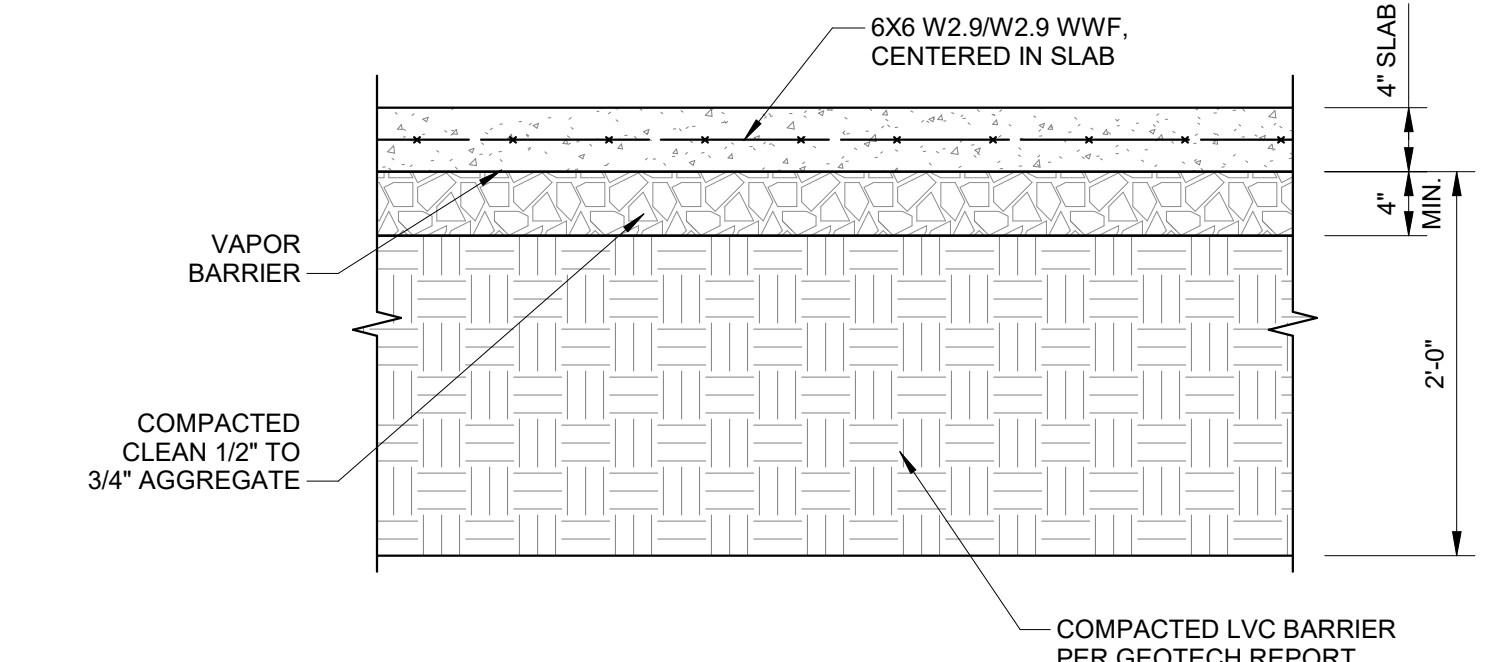
TEE CORNER WALL REINF. CENTERED

NOTES:  
1. THE CORNER REINFORCING SHOWN IS THE SAME SIZE AND SPACING AS THE HORIZONTAL REINFORCING.  
2. THESE DETAILS APPLY TO FOOTINGS, CONCRETE WALLS, AND CMU WALLS.

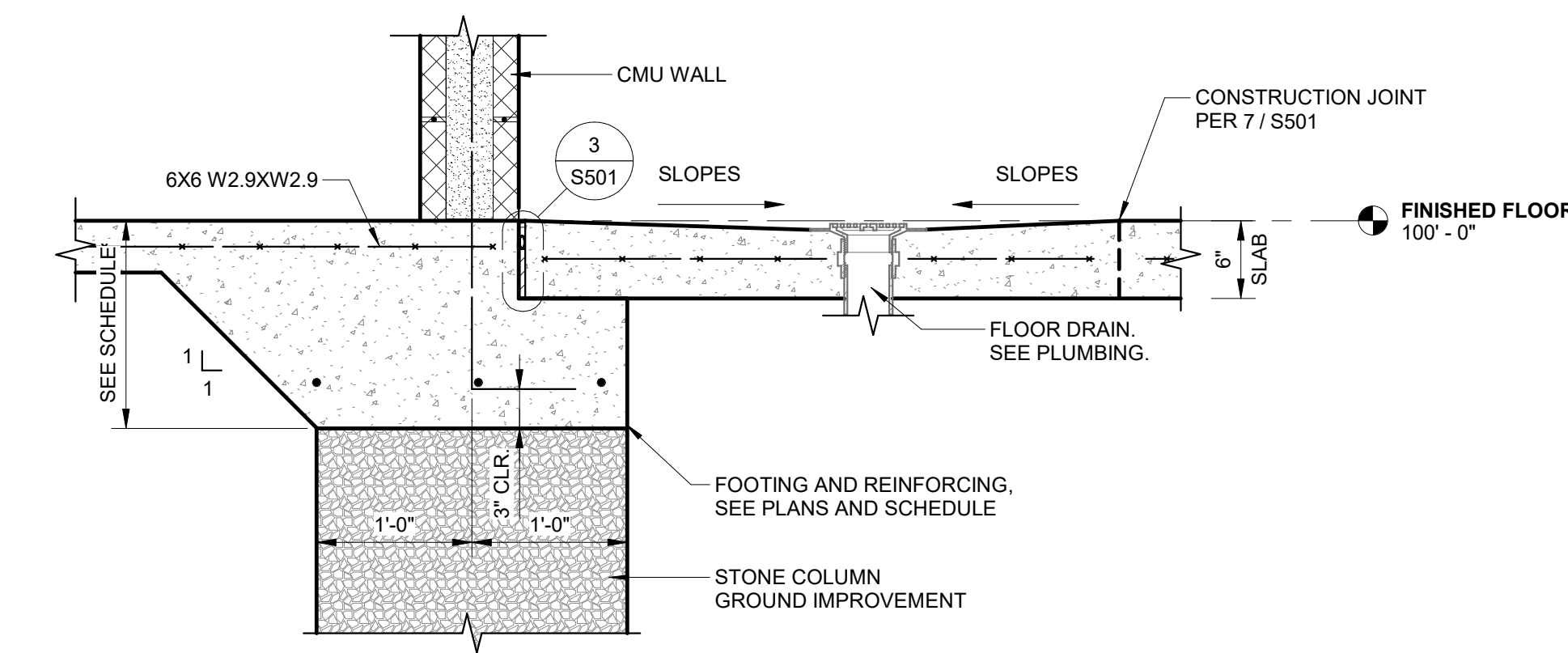
9 CORNER REINFORCING DETAILS  
3/4" = 1'-0"



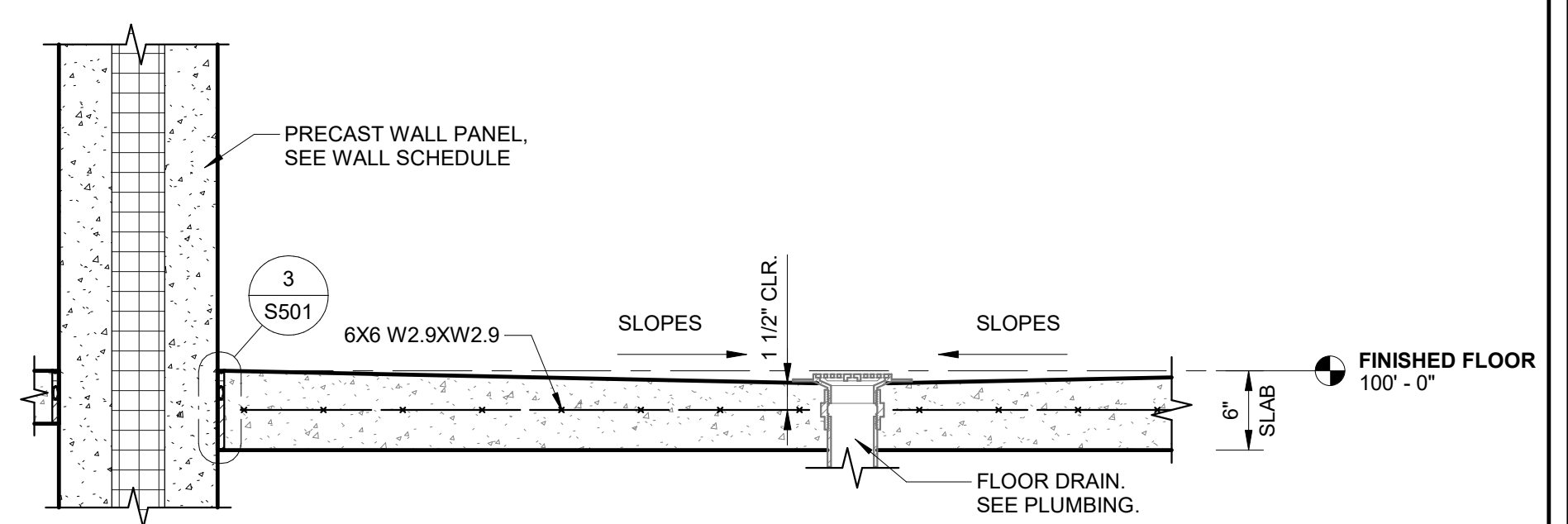
10 6" SLAB SECTION  
1" = 1'-0"



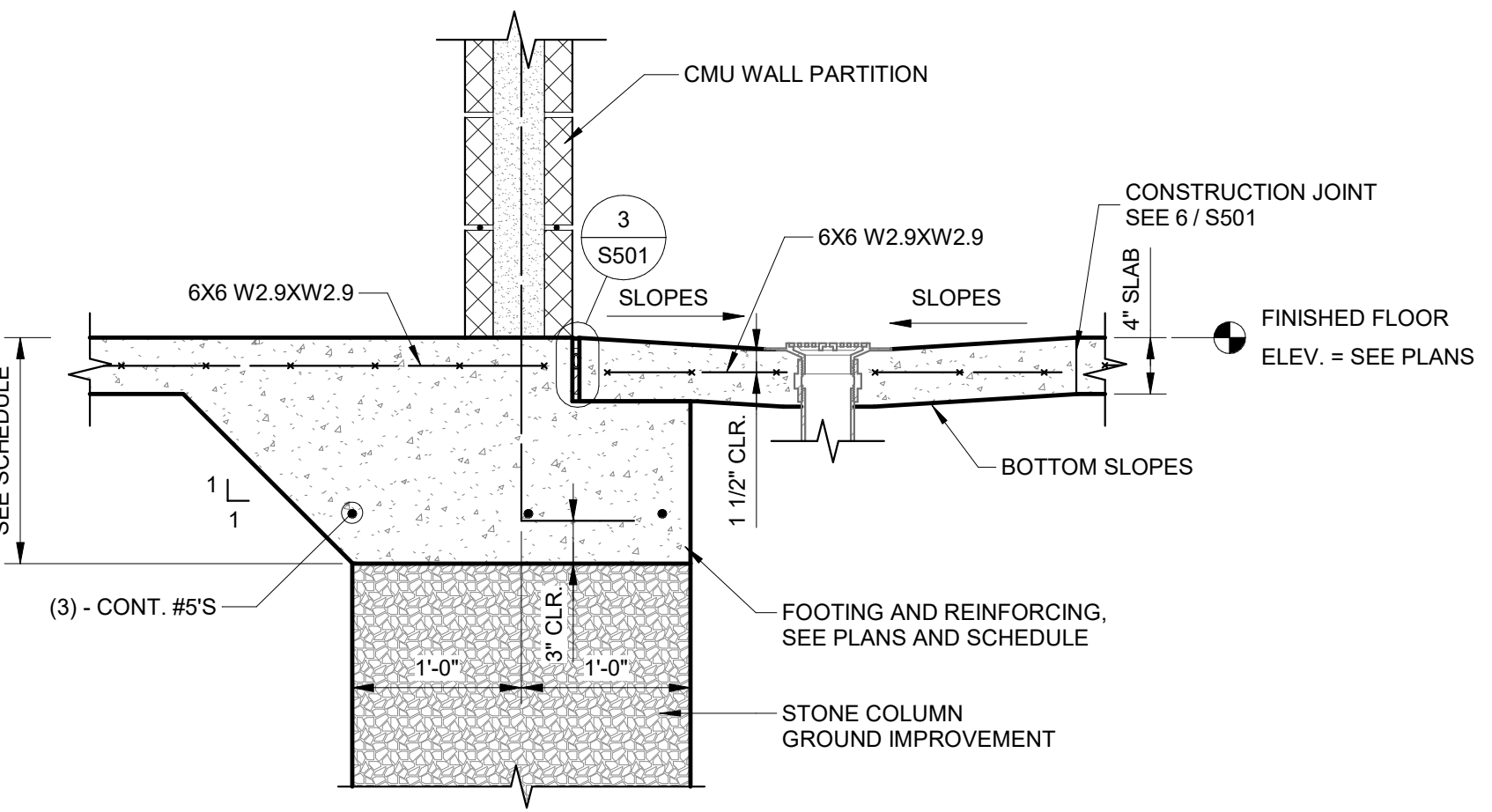
11 4" SLAB SECTION  
1" = 1'-0"



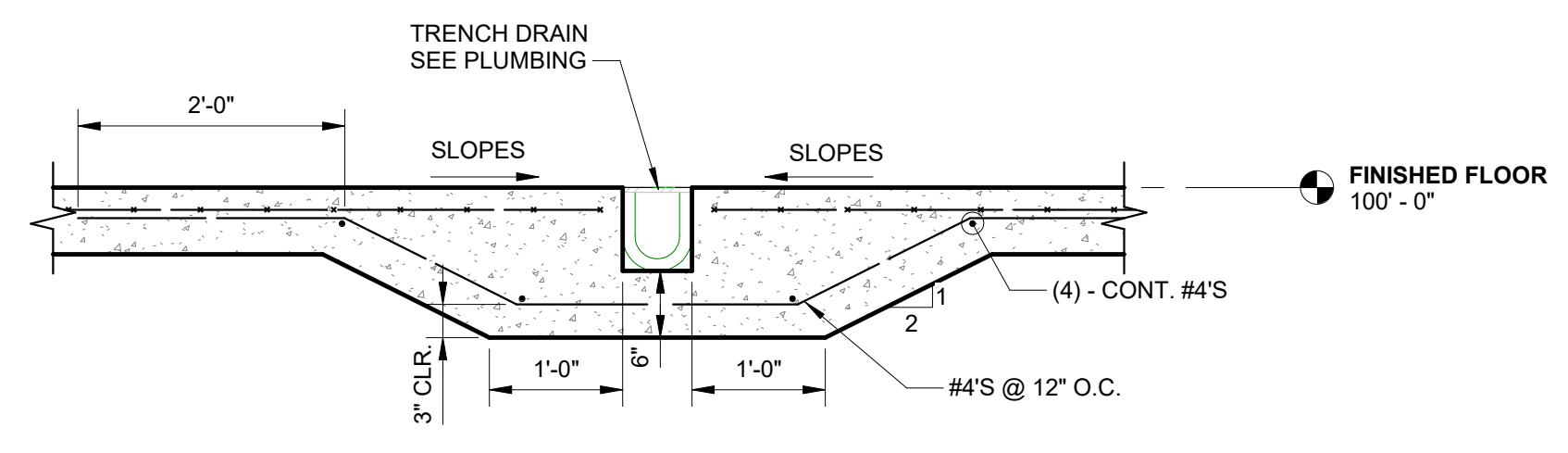
12 SHOWER SLOPED SLAB AT CMU  
1" = 1'-0"



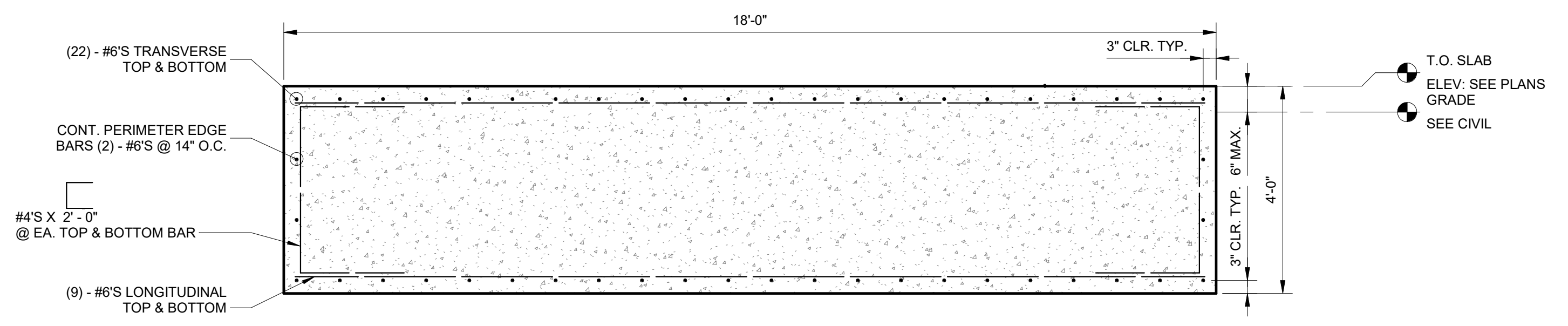
13 SHOWER SLOPED SLAB AT PRECAST  
1" = 1'-0"



14 SLOPED SLAB AT FLOOR DRAINS & SHOWERS  
1" = 1'-0"



15 TRENCH DRAIN  
3/4" = 1'-0"



16 CONCRETE PAD UNDER GENERATOR SECTION  
1/2" = 1'-0"

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REVISION HISTORY		
DESCRIPTION	DATE	APPR

ISSUED FOR: 03/01/2024

BID & PERMIT

EDGAR COUNTY PUBLIC SAFETY CENTER  
EDGAR COUNTY, ILLINOIS  
12636 950TH ROAD  
PARIS, ILLINOIS 61944

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JTS	NML
FIELD	FIELD BOOK

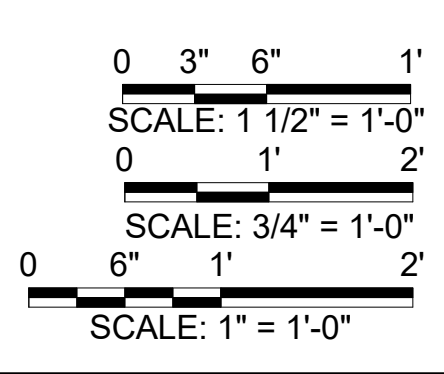
CHECKED	CHECK DATE
ADL	03/01/2024

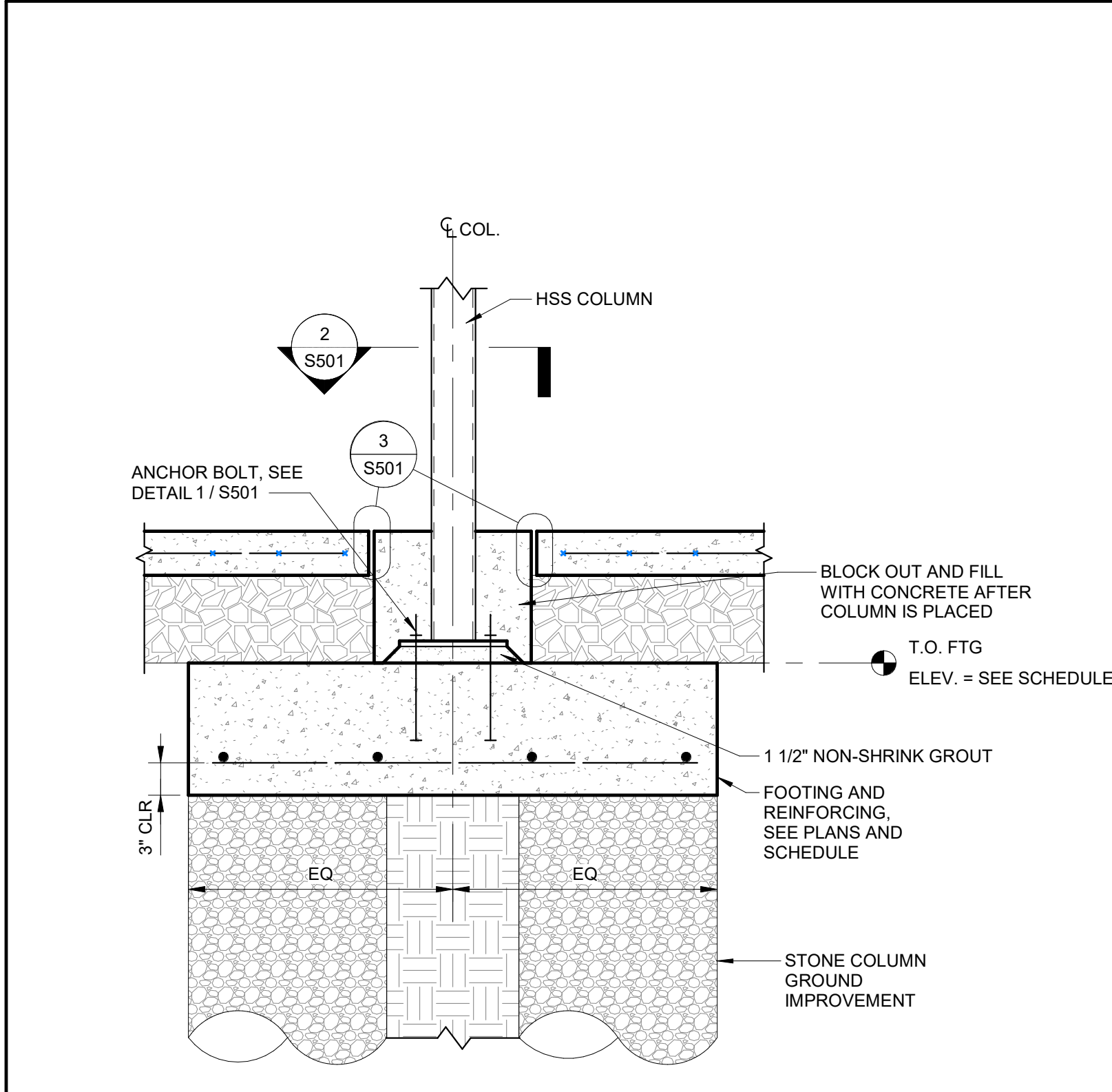
SHEET TITLE

FOUNDATION SLAB DETAILS

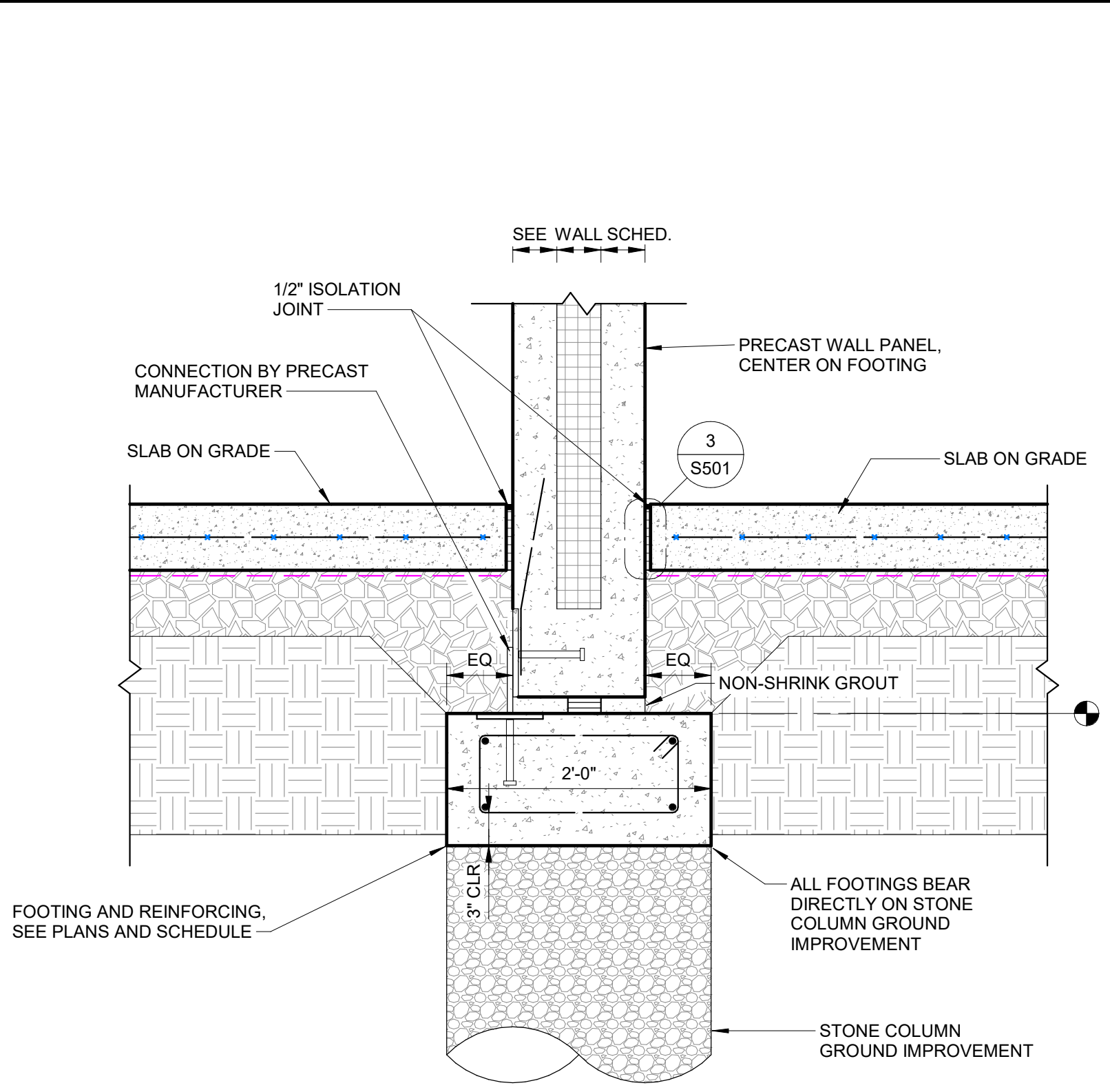
PROJECT NO. 22-4048  
DRAWING ISSUED DATE: 03/01/2024

SHEET S501

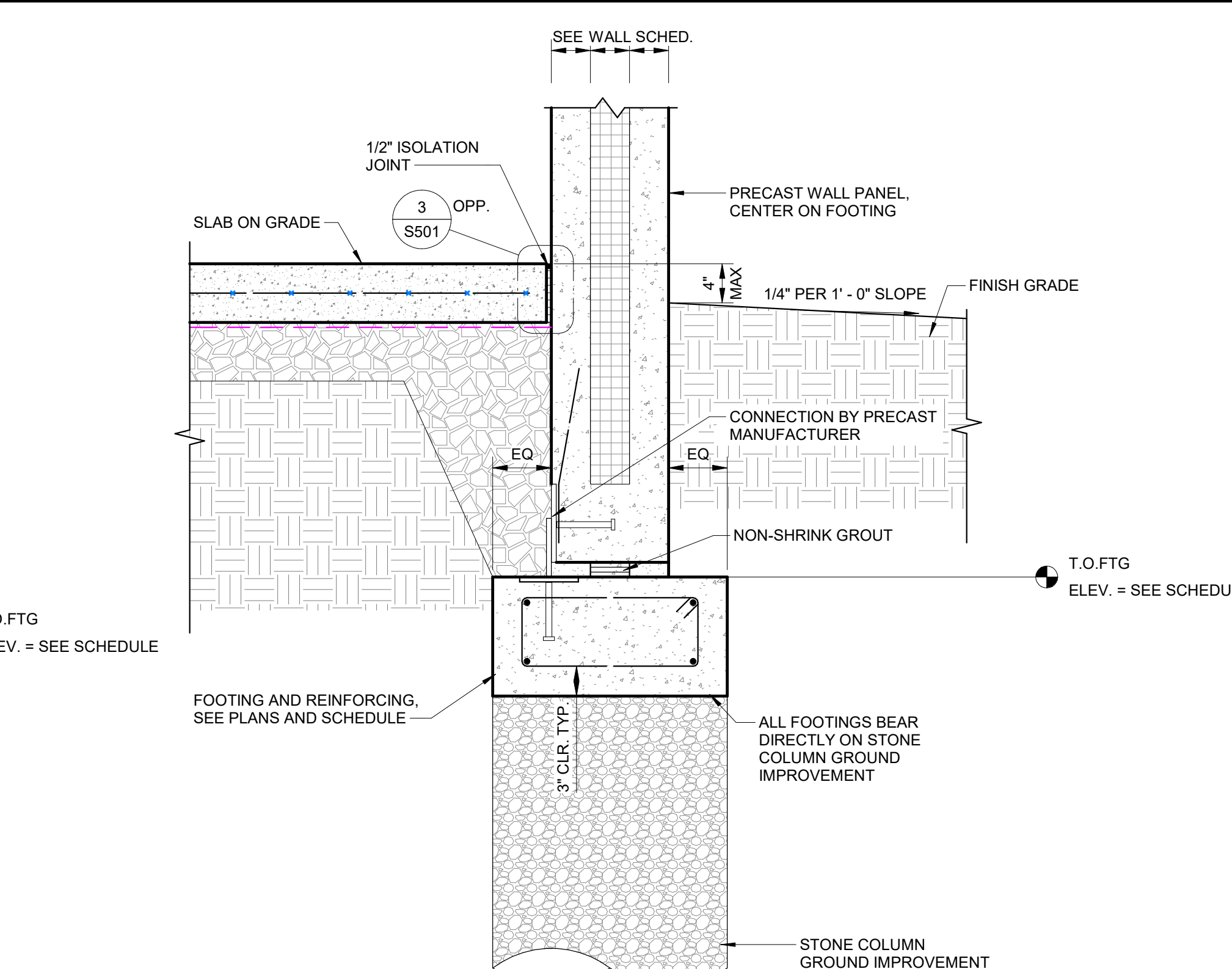




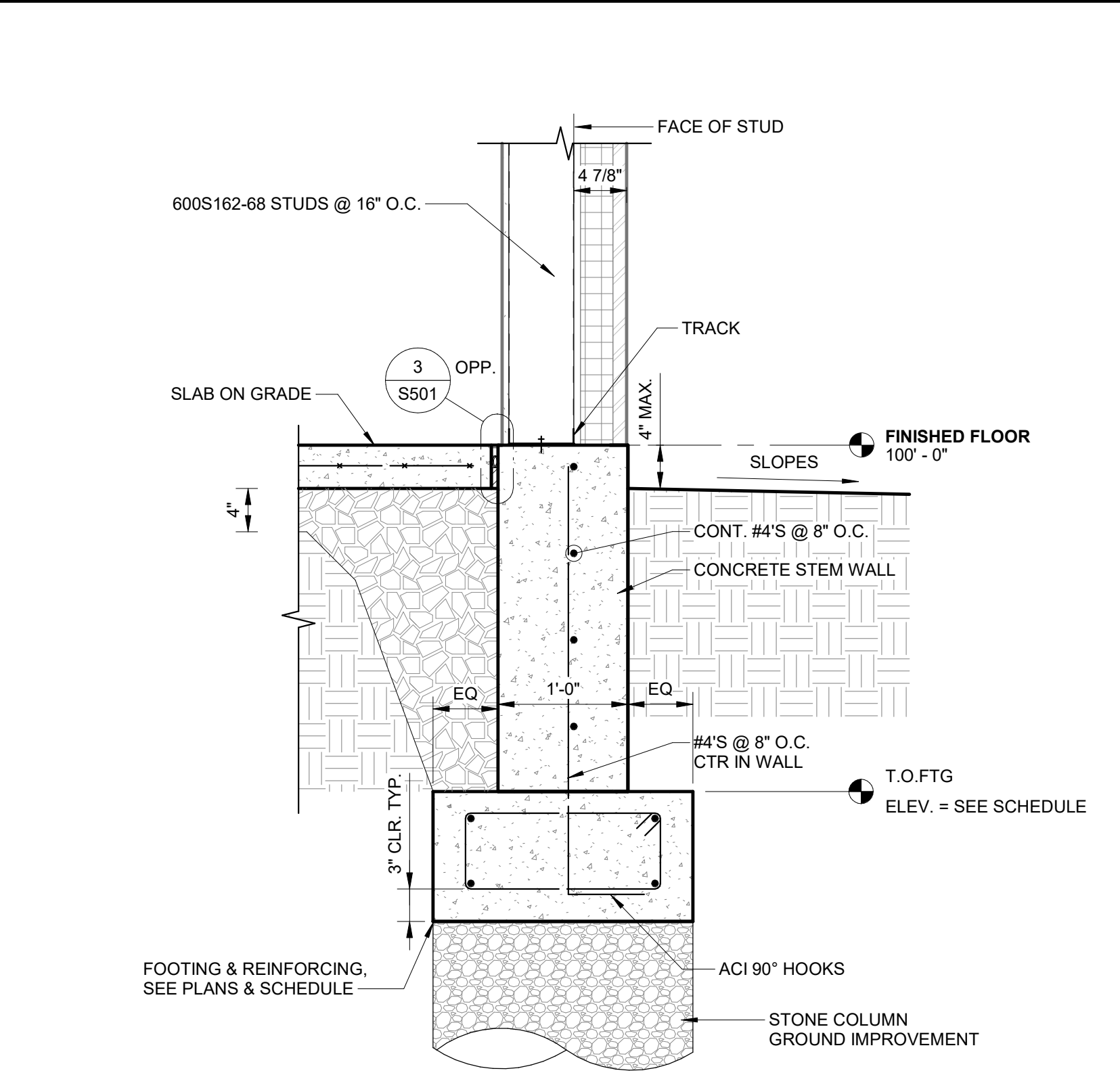
**1 COLUMN FTG.**  
1" = 1'-0"



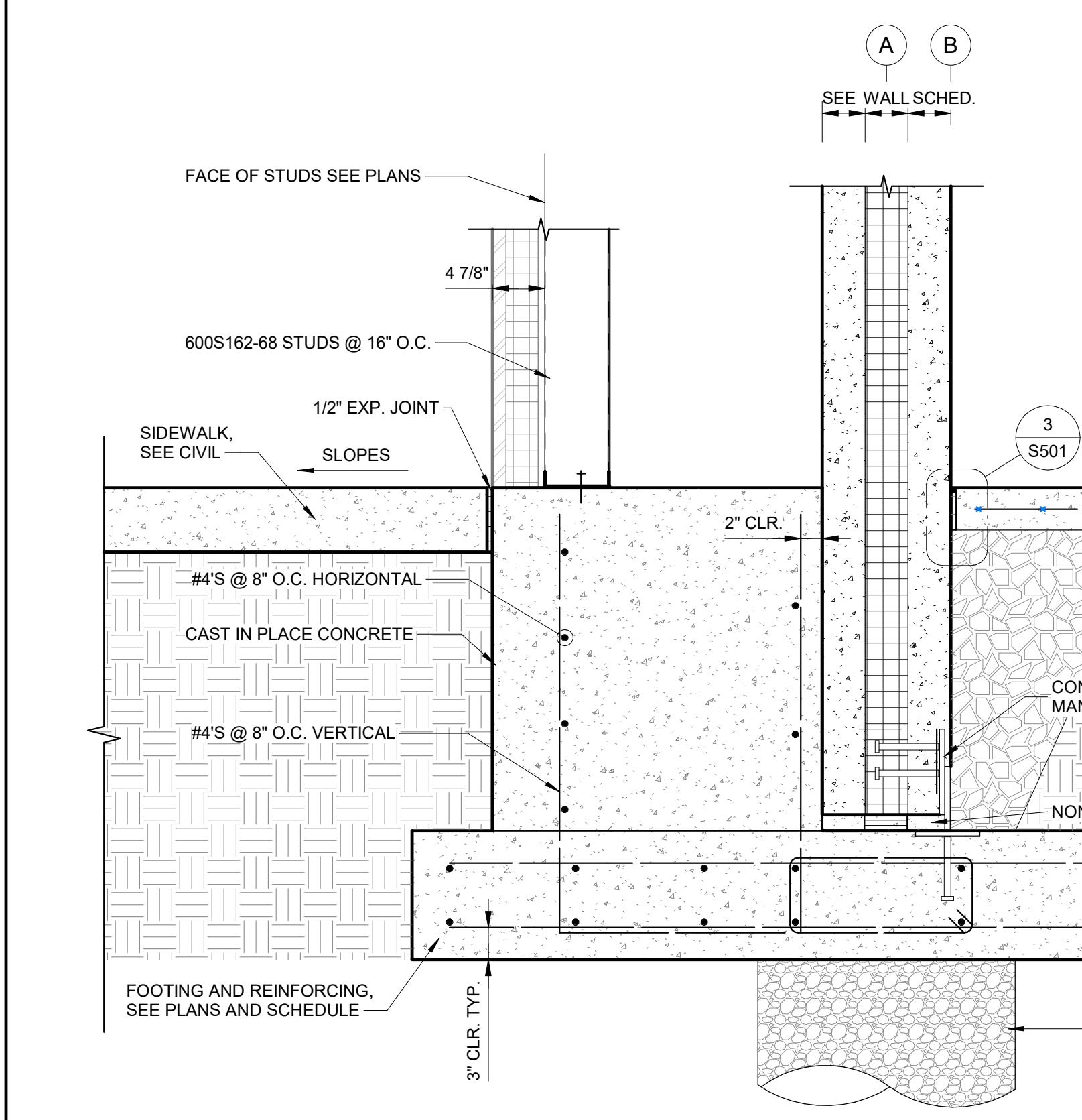
**2 TYPICAL SECTION THROUGH INTERIOR PRECAST WALL FOOTING**  
1" = 1'-0"



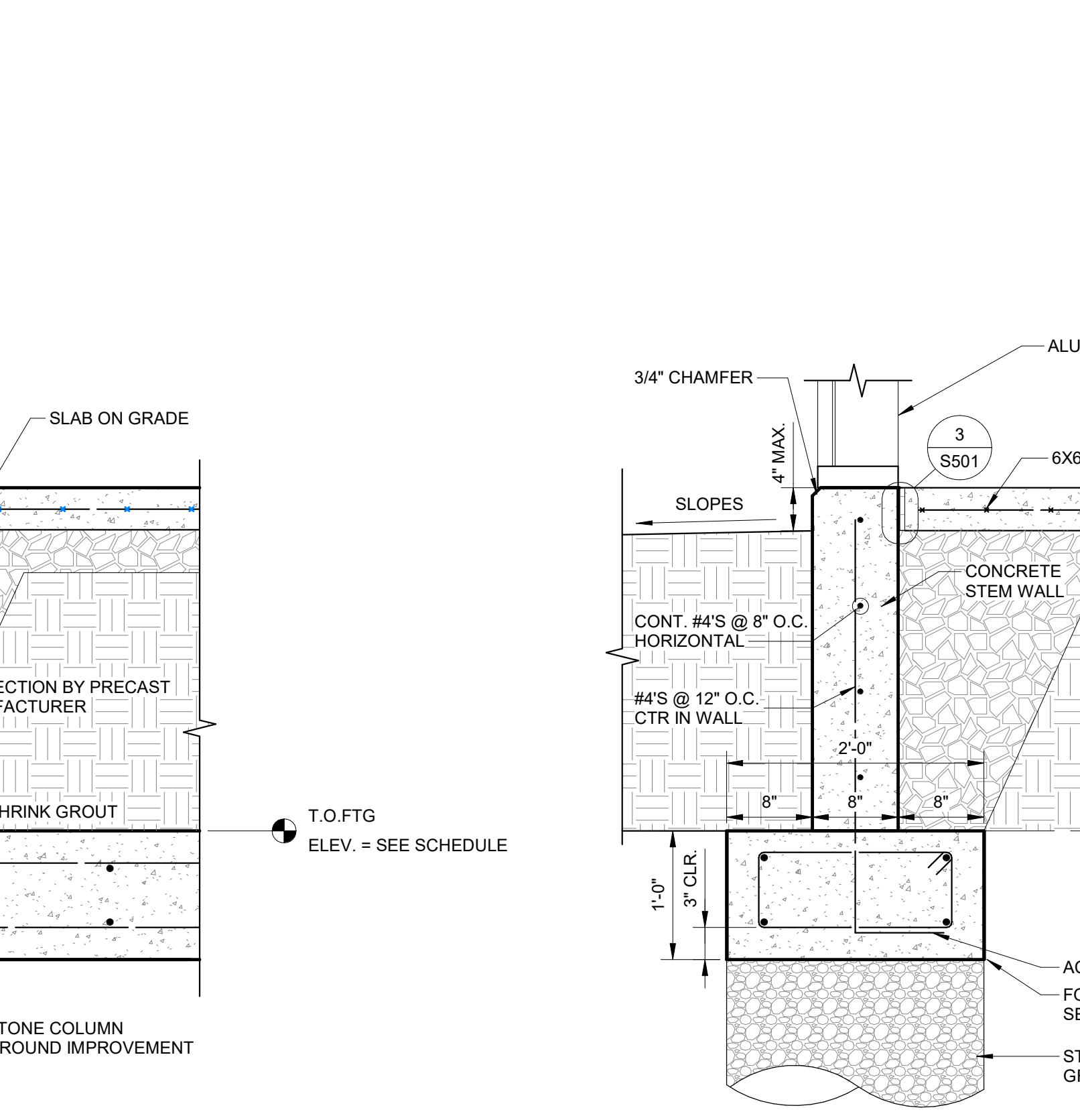
**3 TYPICAL SECTION THROUGH EXTERIOR WALL FOOTING**  
1" = 1'-0"



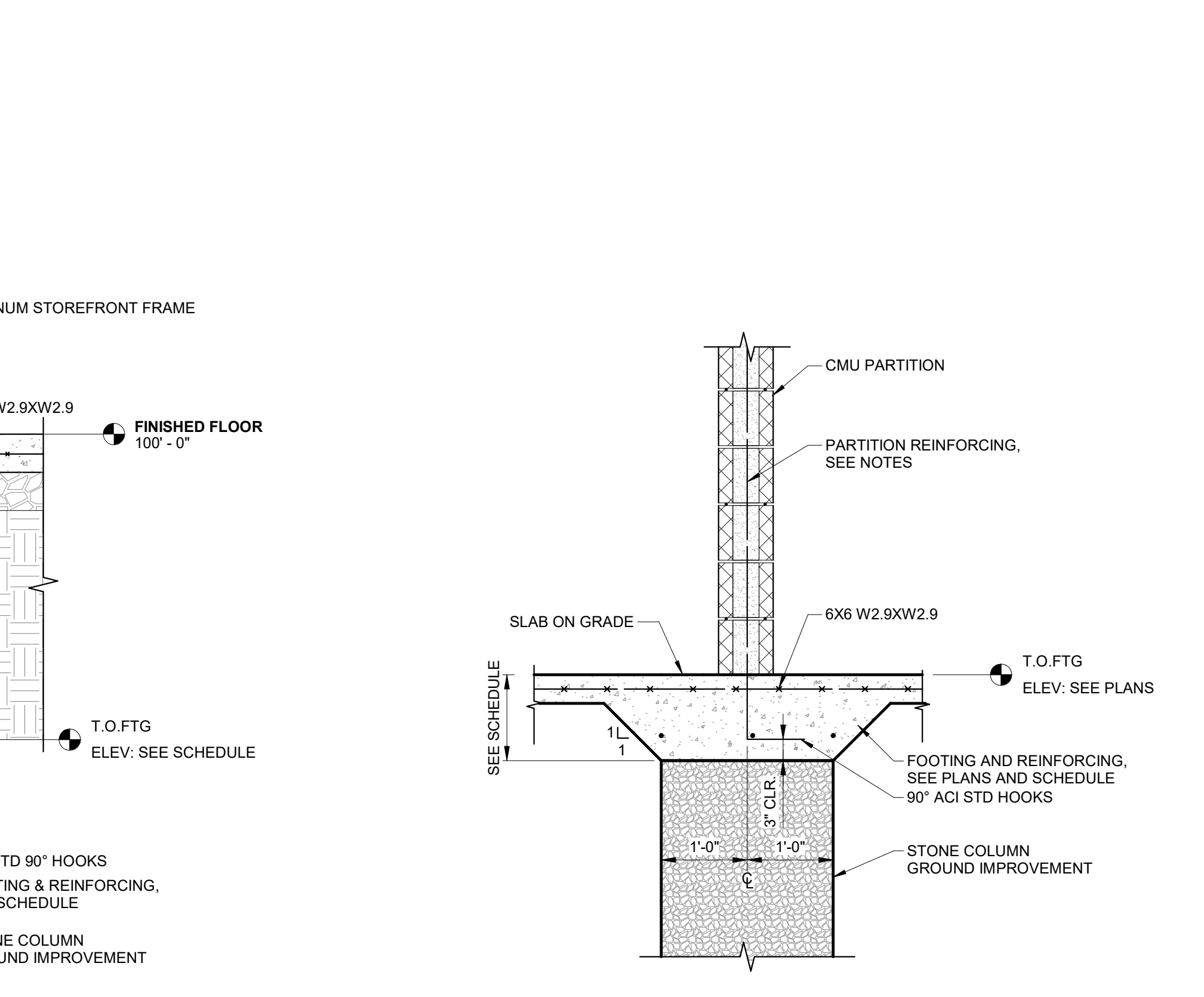
**4 SECTION**  
1" = 1'-0"



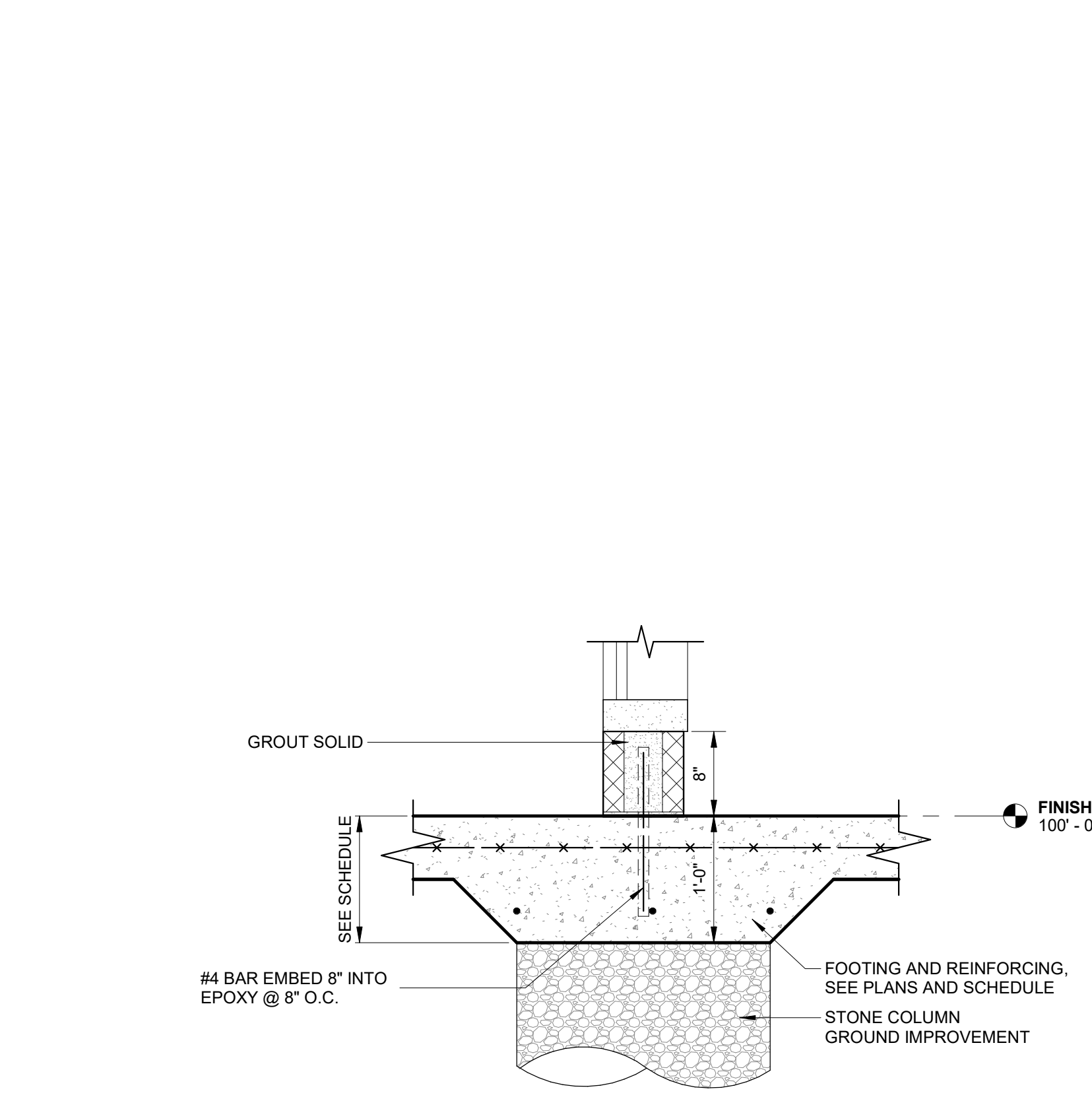
**5 SECTION**  
1" = 1'-0"



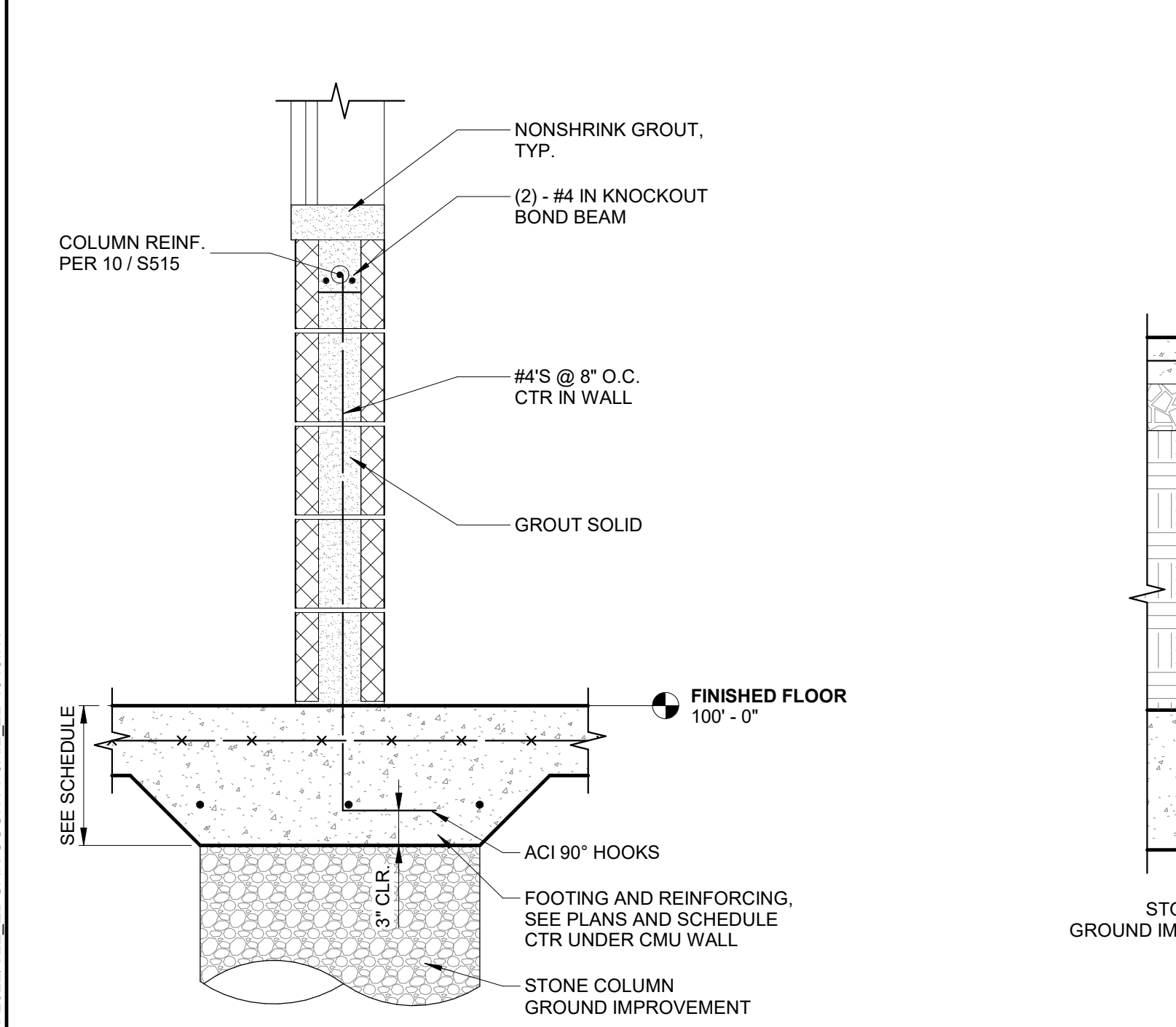
**6 SECTION**  
1" = 1'-0"



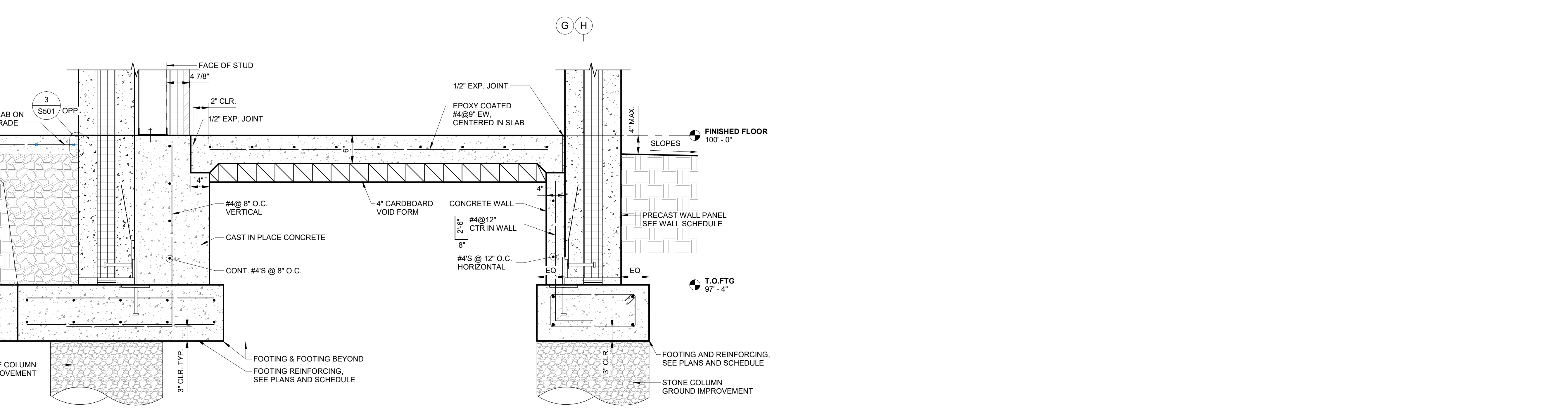
**7 TYPICAL THICKENED SLAB DETAIL**  
3/4" = 1'-0"



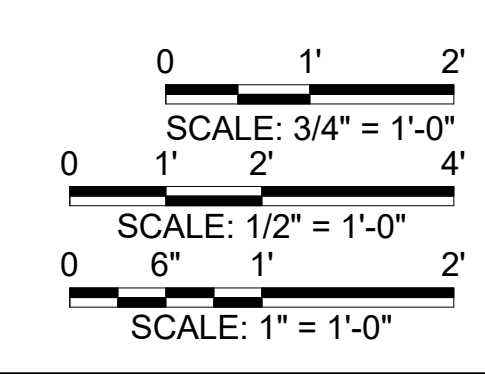
**8 DAYROOM SECURITY - SECTION**  
1" = 1'-0"

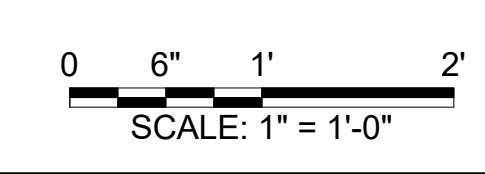
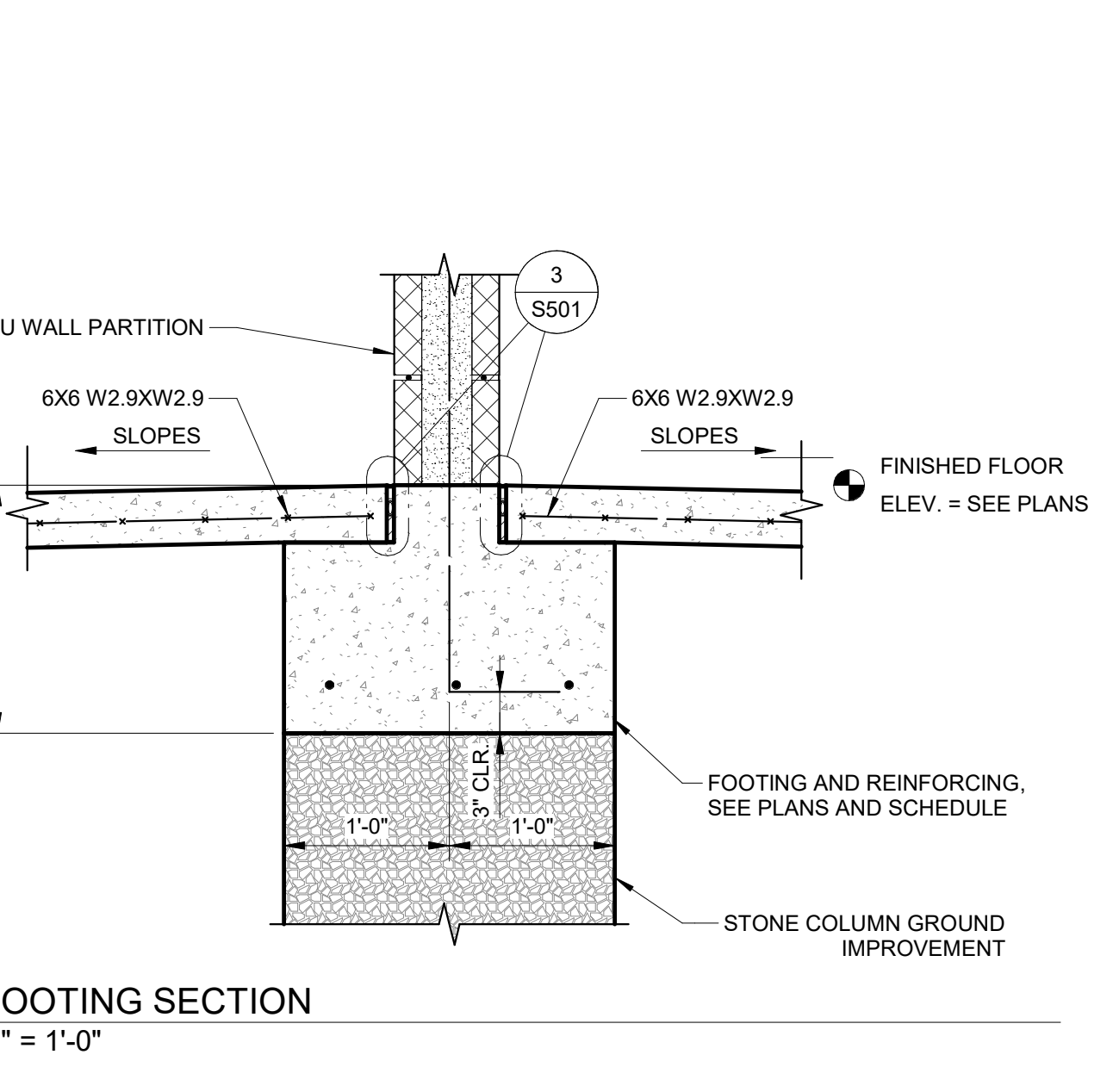
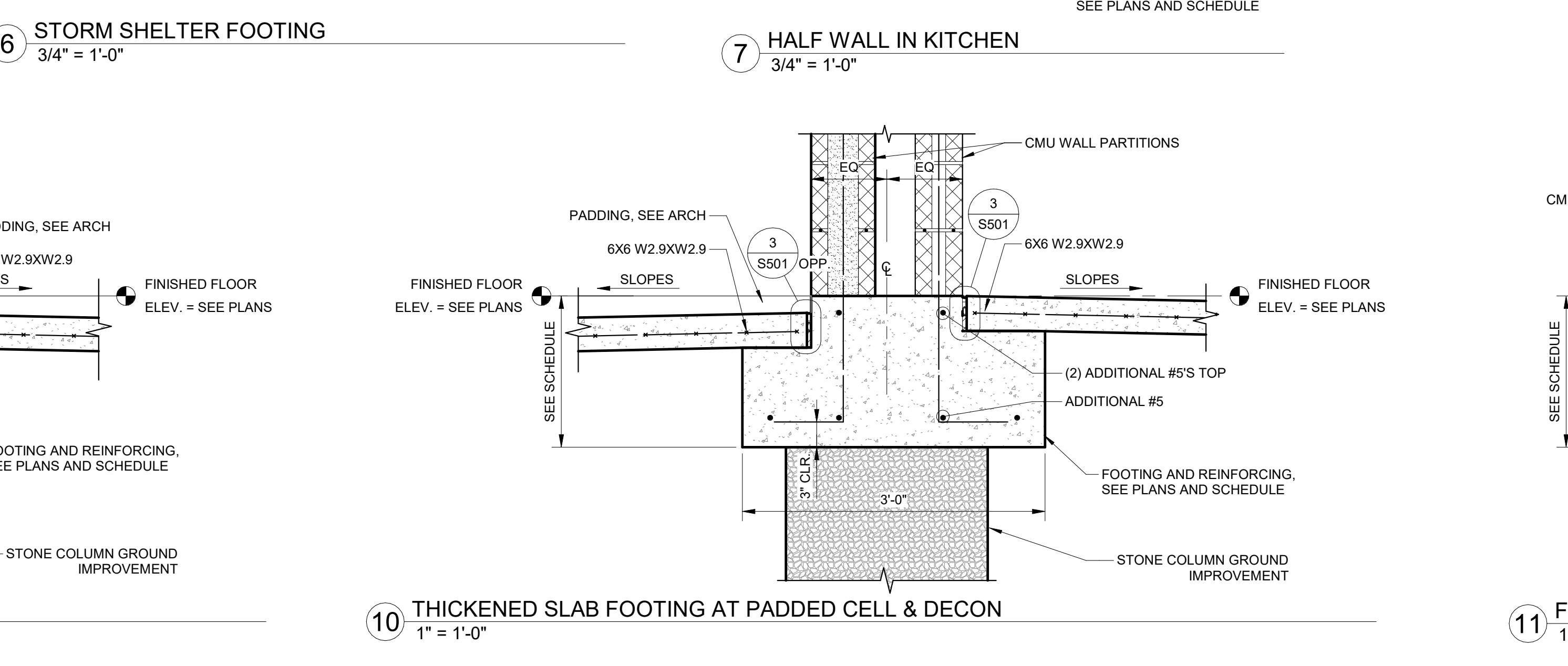
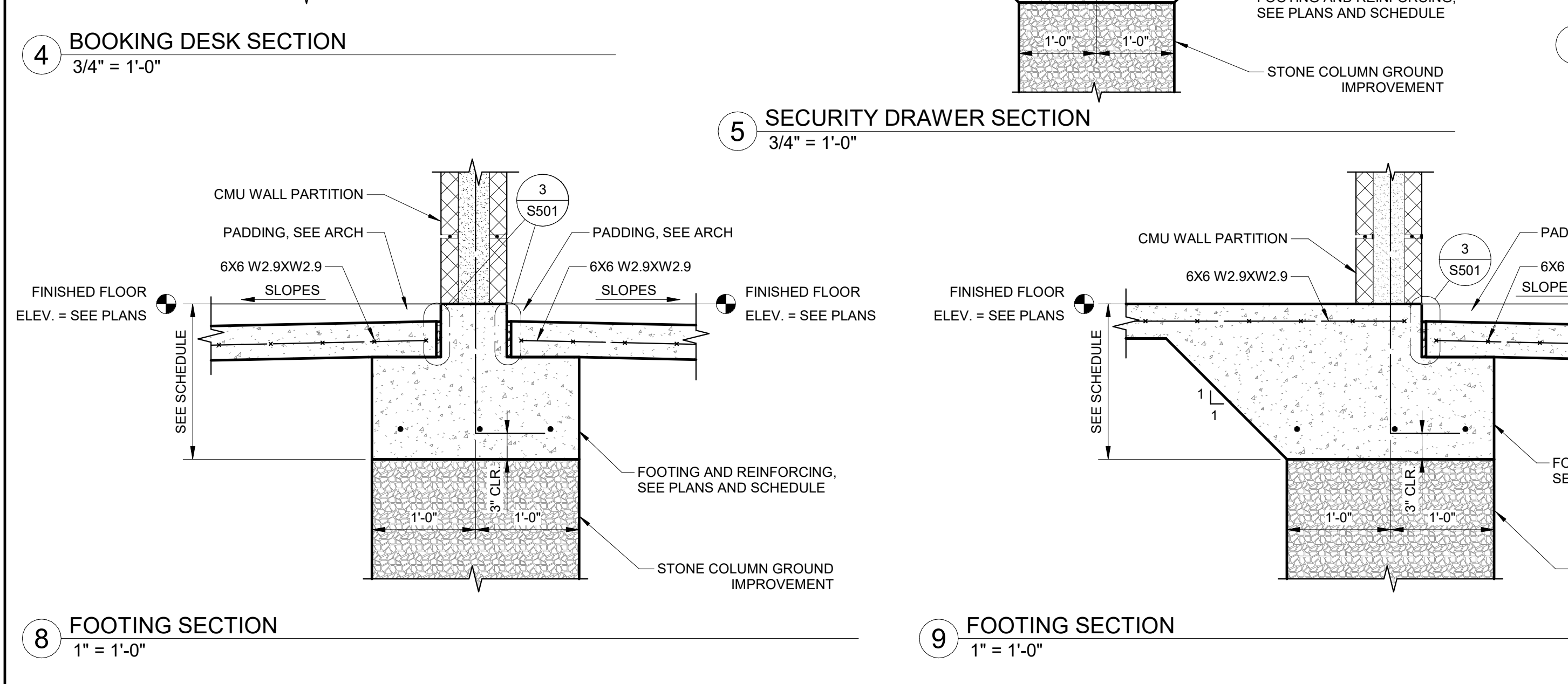
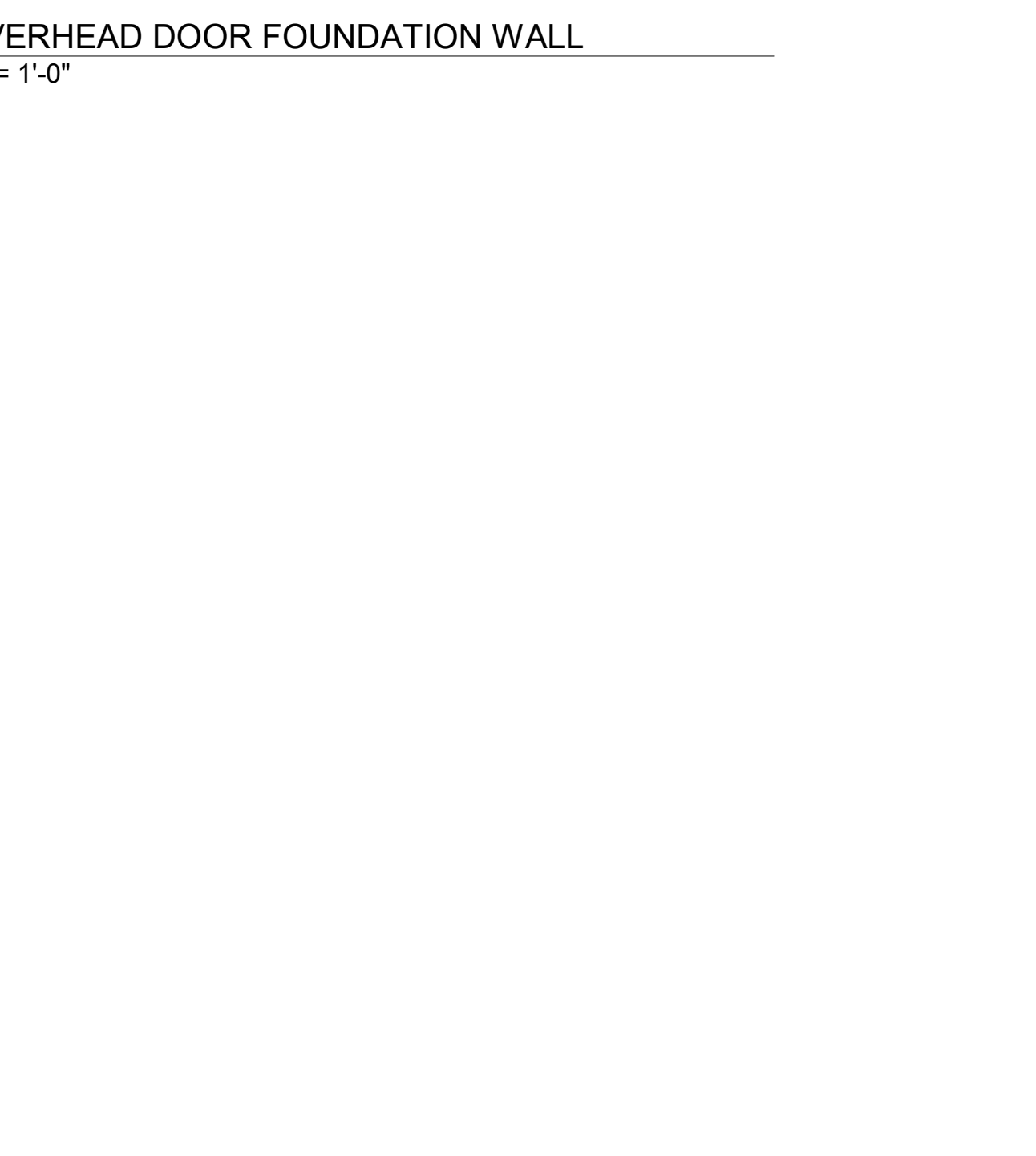
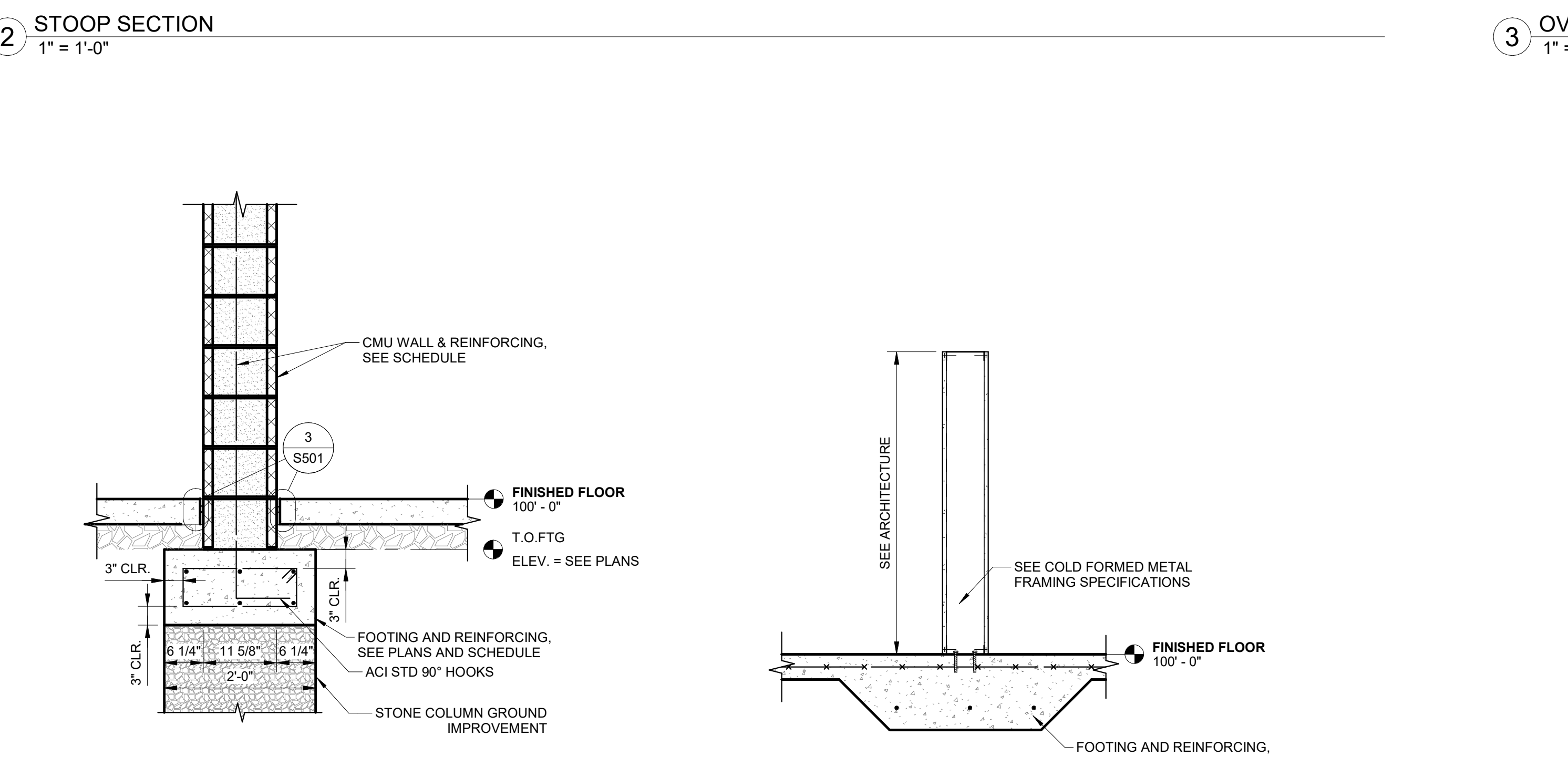
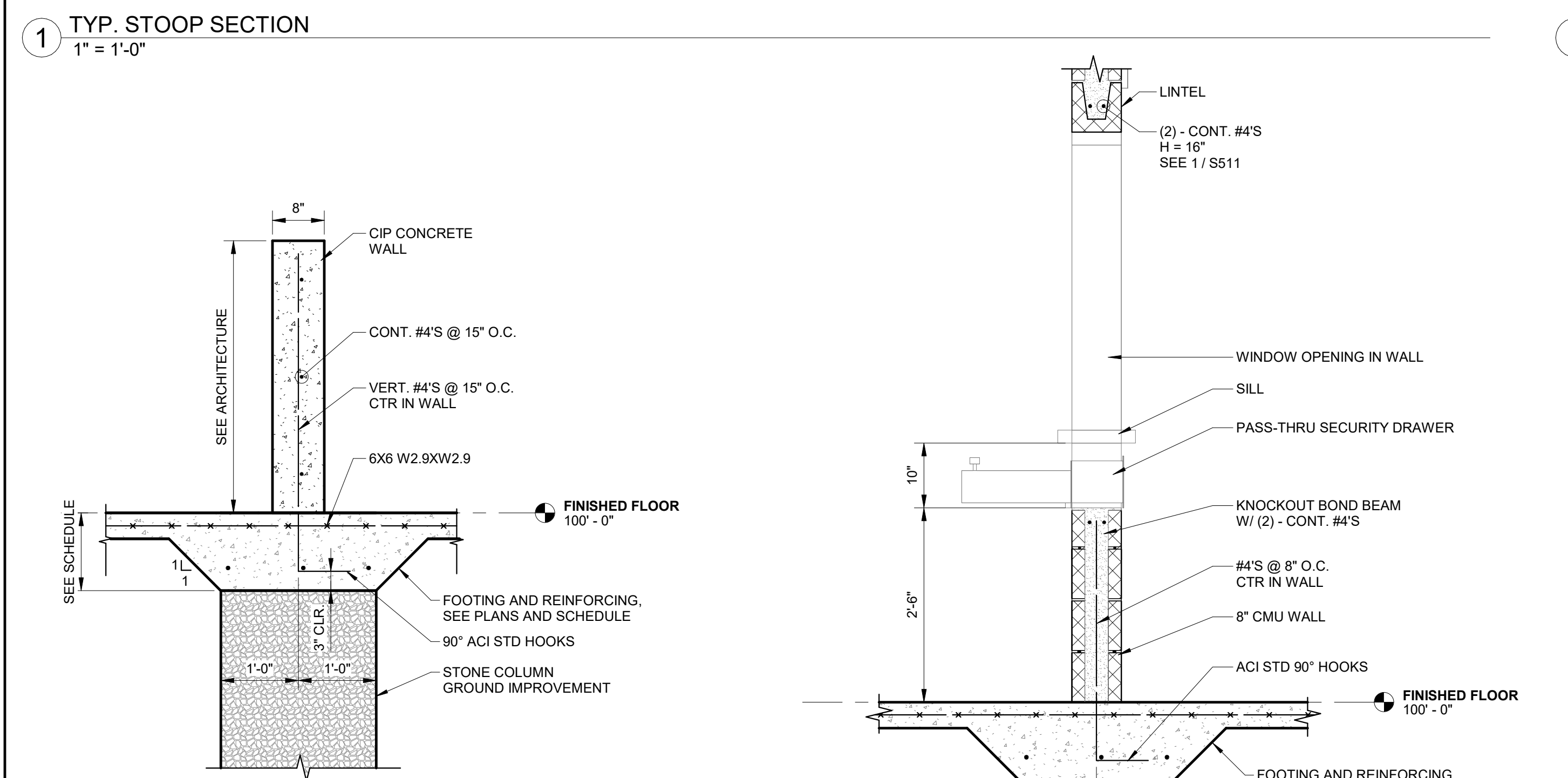
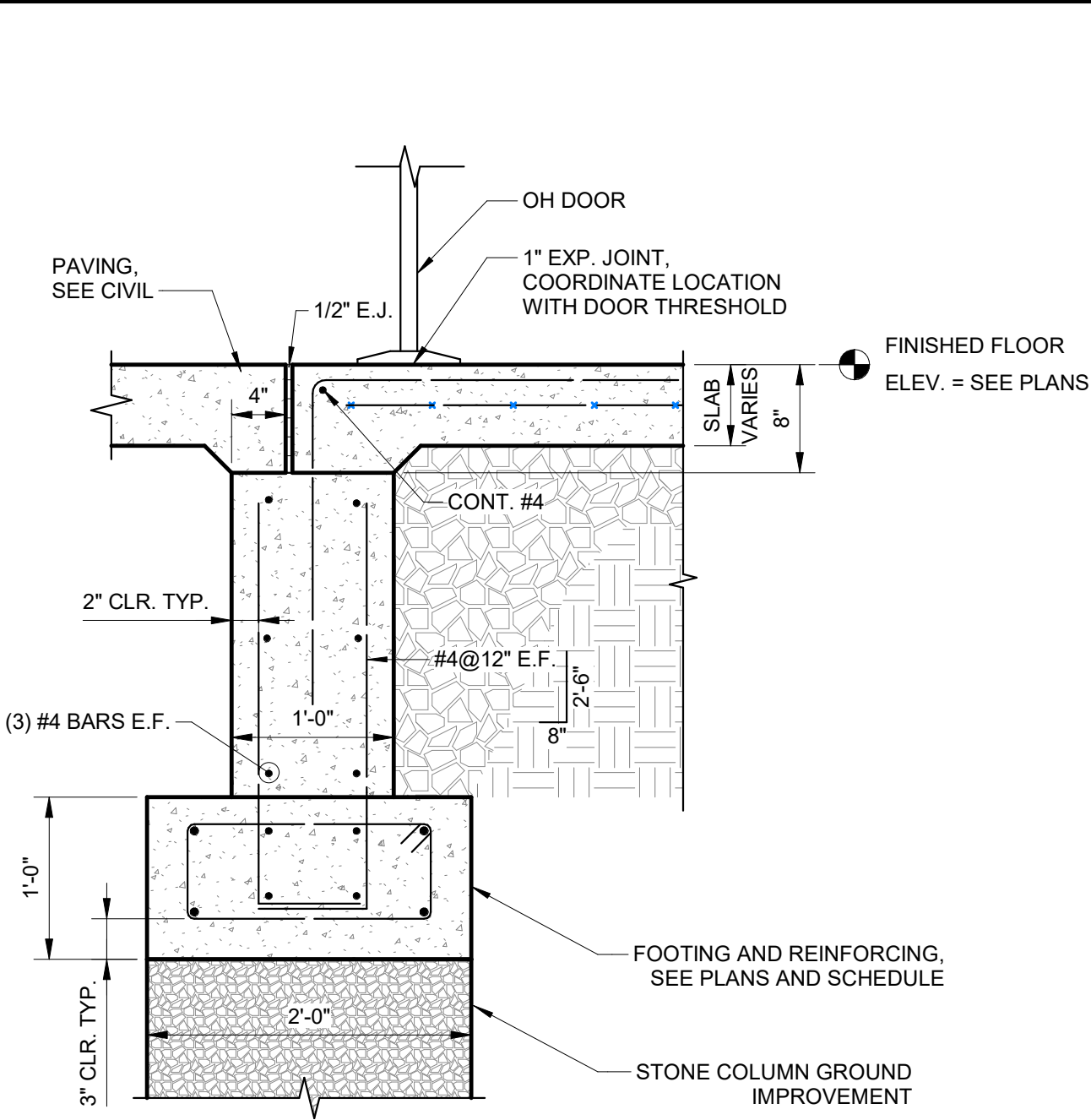
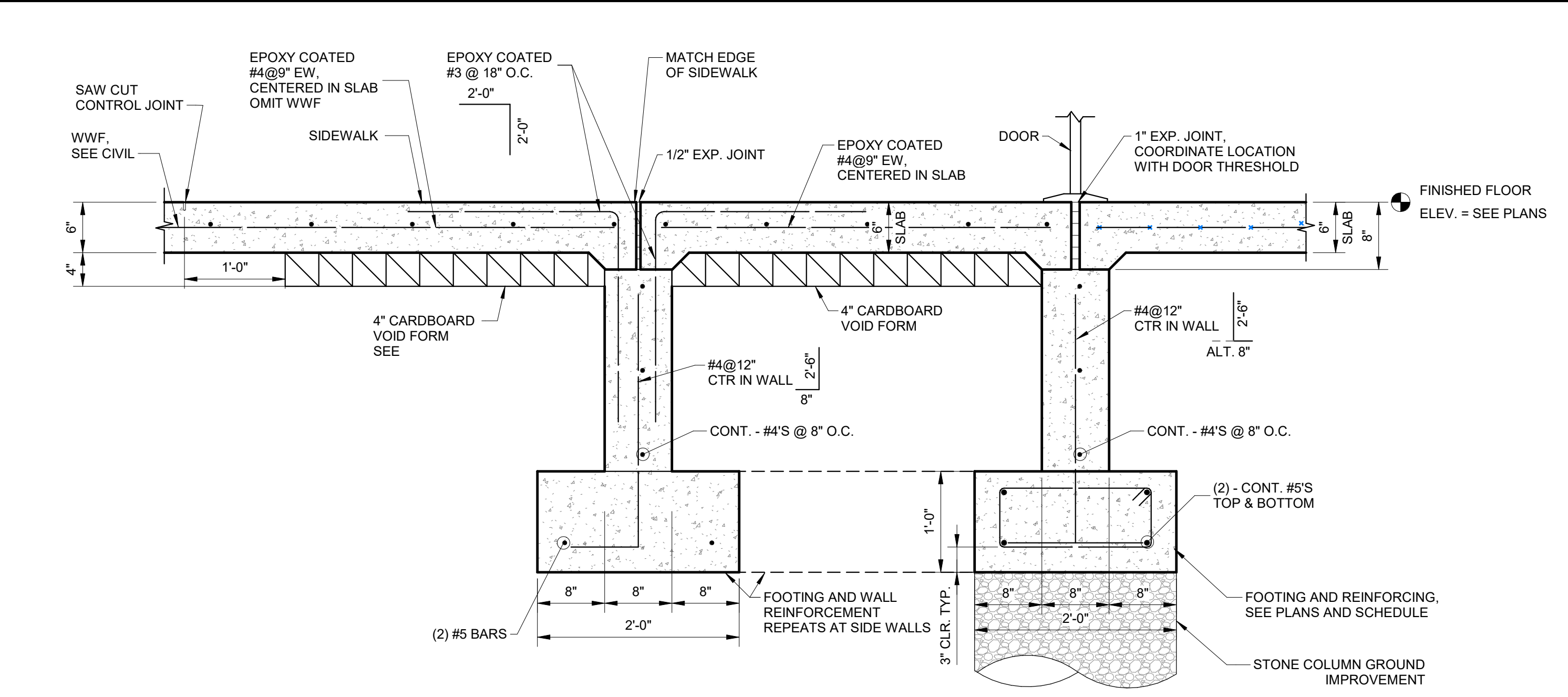
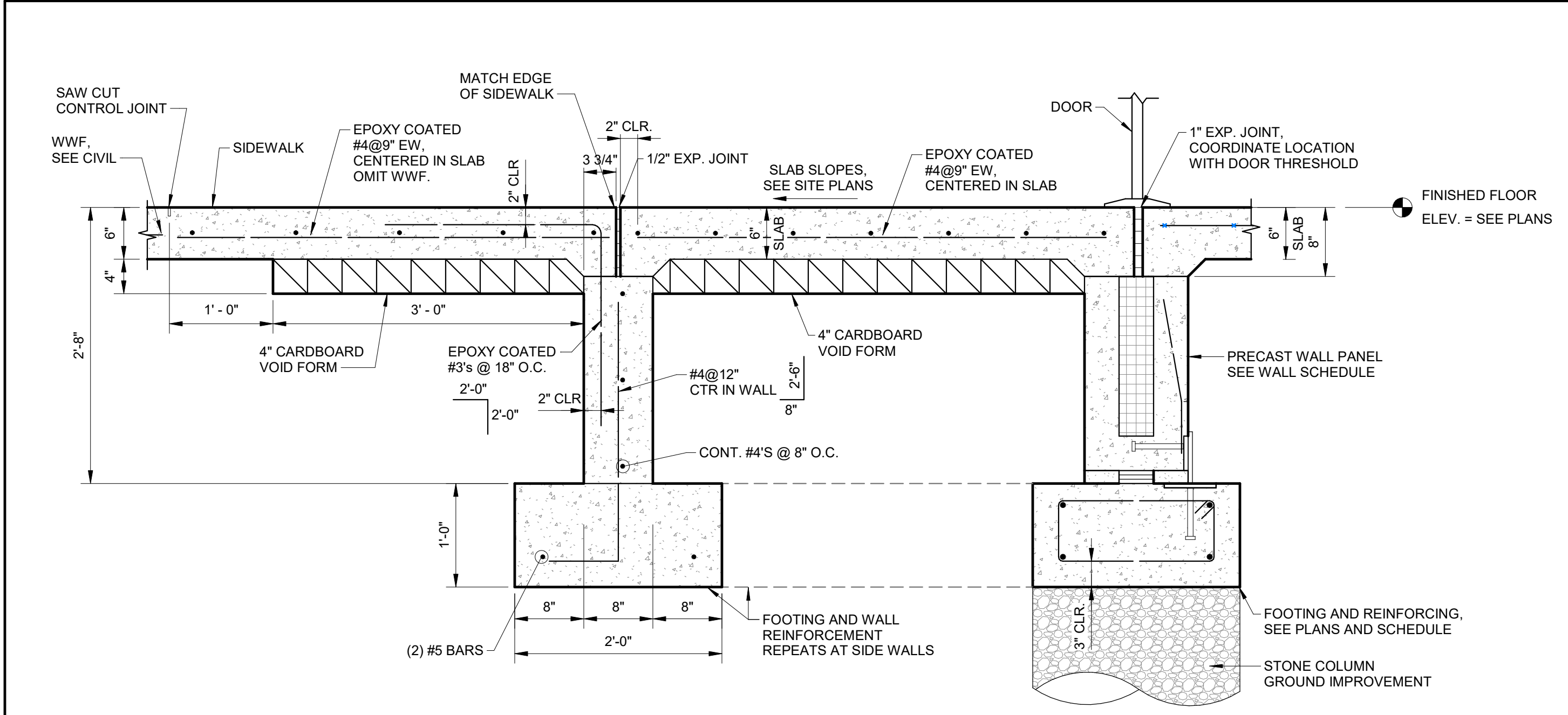


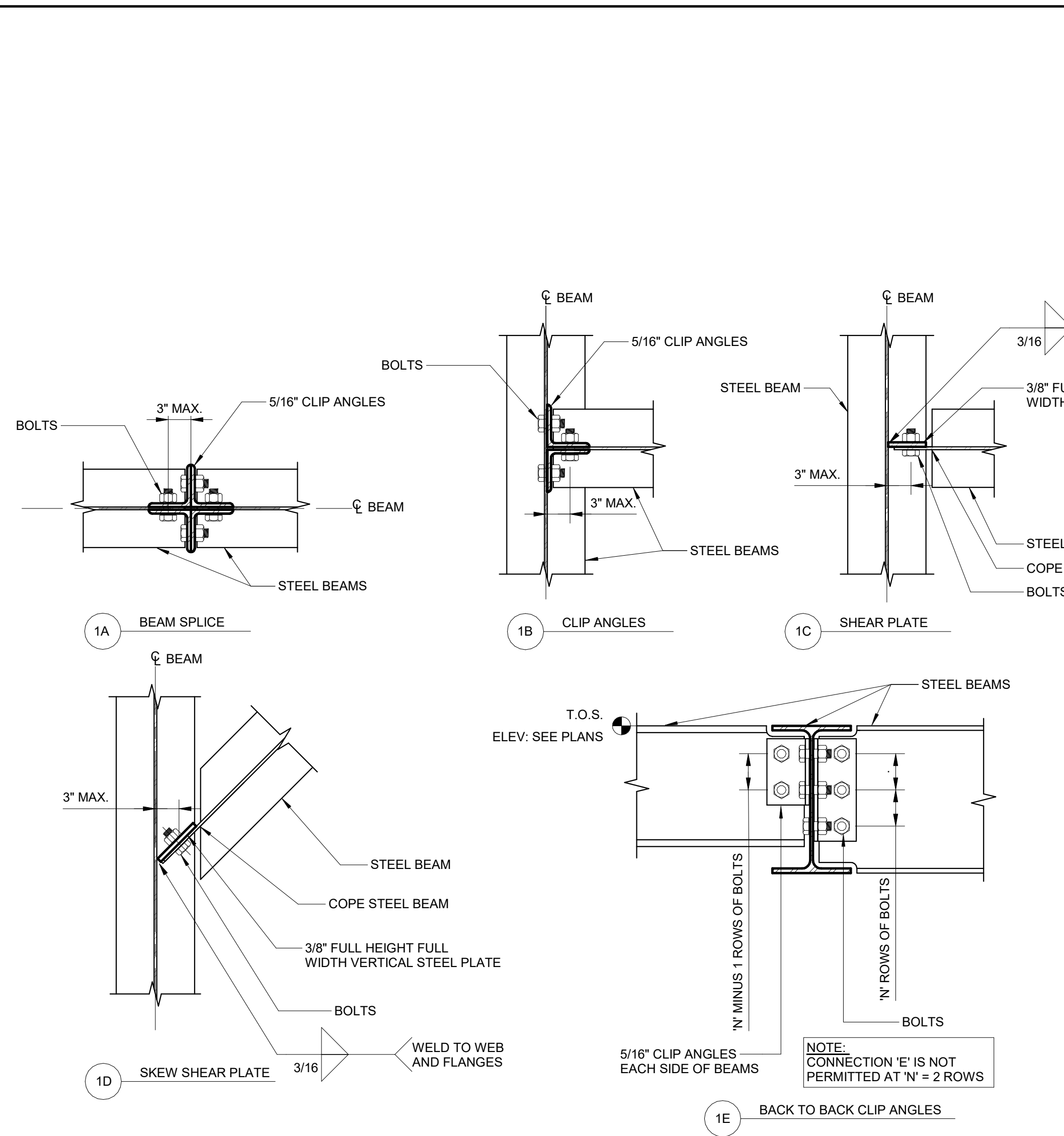
**9 CONTROL ROOM SECTION**  
1" = 1'-0"



**10 SECTION**  
1" = 1'-0"



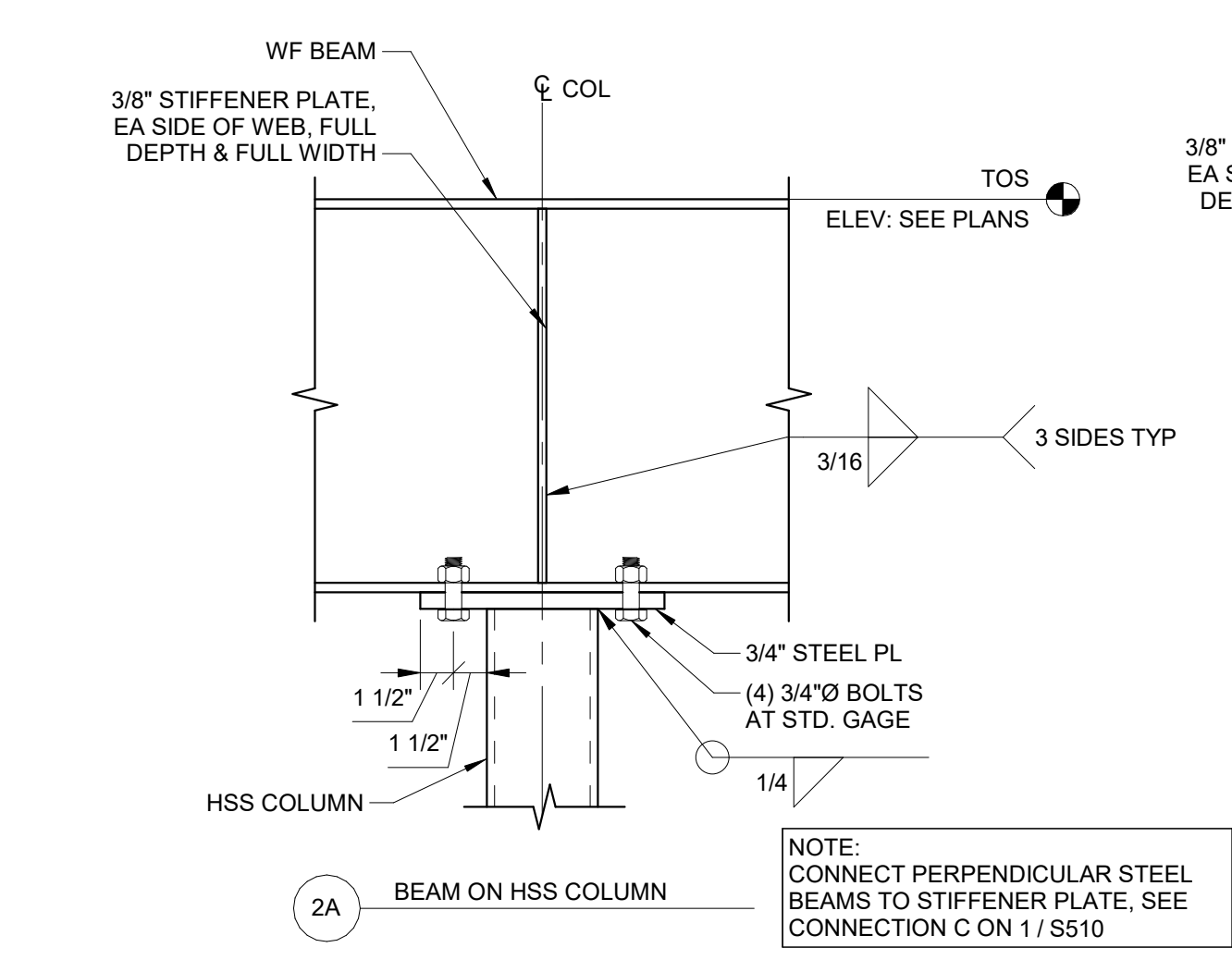




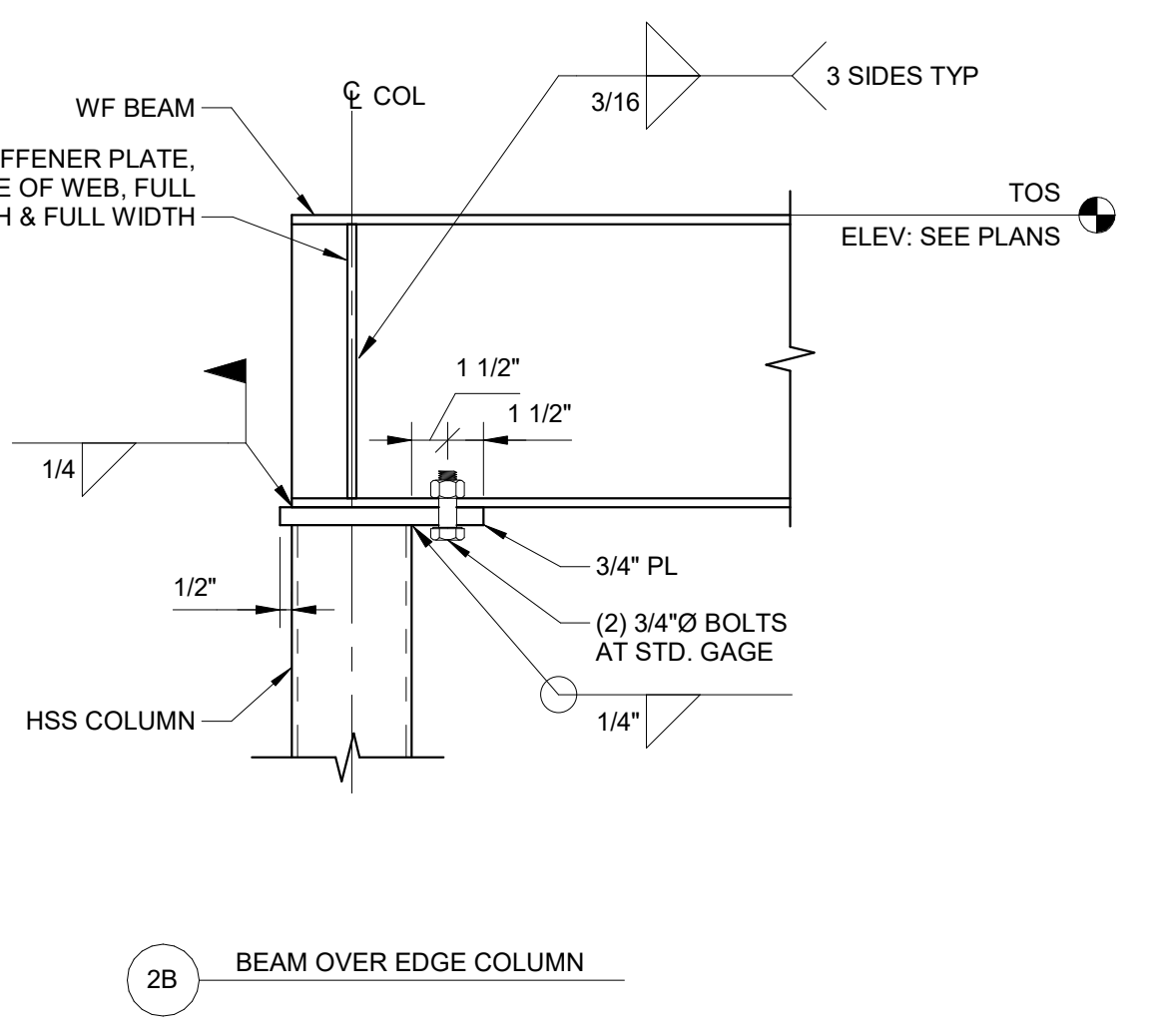
**1 BEAM TO BEAM SHEAR CONNECTION DETAILS**  
1 1/2" = 1'-0"

BEAM SIZE	'N' ROWS OF BOLTS	ANGLE LENGTH
W8 & W10	2 ROWS	0' 5 1/2"
W12 & W14	3 ROWS	0' 8 1/2"
W16	4 ROWS	0' 11 1/2"
W18	5 ROWS	1' 2 1/2"
W21 & W24	6 ROWS	1' 5 1/2"
W27 & W30	7 ROWS	1' 8 1/2"

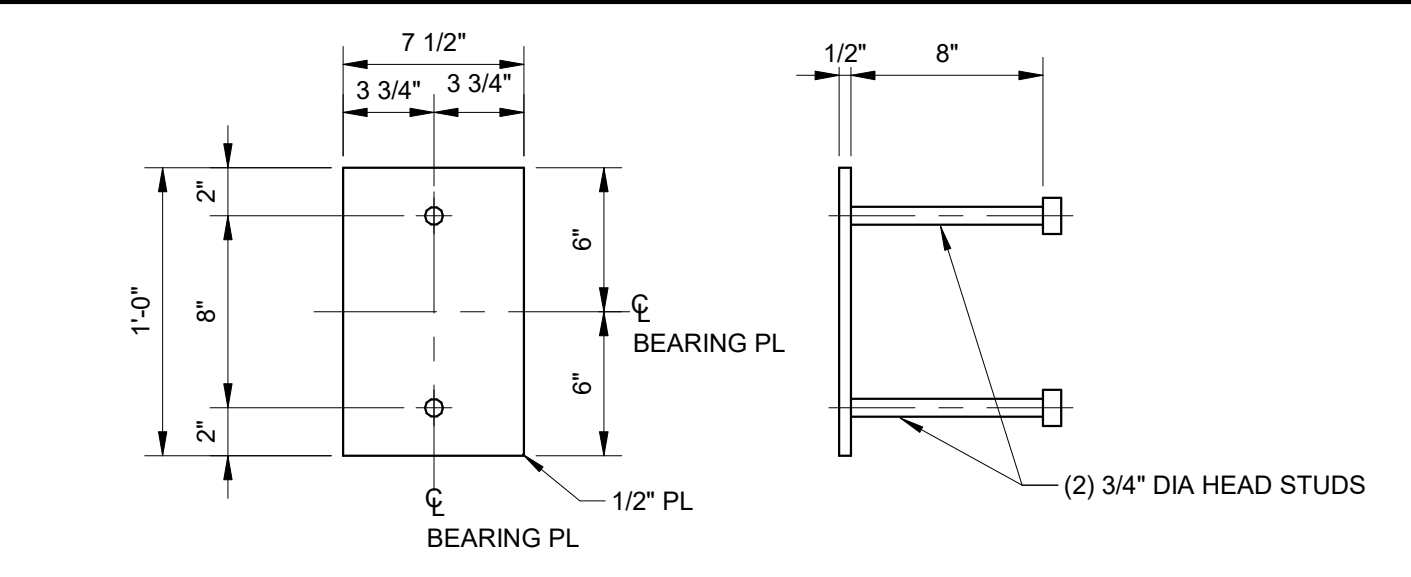
- NOTES:**
- ALL BOLTS ARE SNUG-TIGHT 3/4" Ø GRADE A325-N BOLTS IN 13/16" Ø HOLES. SHORT SLOTTED HOLES ARE PERMITTED IF GRADE F1852 BOLTS ARE PRETENSIONED.
  - SPACE ROWS OF BOLTS AT 3" O.C.
  - PROVIDE 1 1/4" MINIMUM EDGE DISTANCES.
  - THE NUMBER OF ROWS ARE BASED ON THE SHALLOWER STEEL BEAM IN THE CONNECTION.
  - SUBTRACT ONE ROW OF BOLTS WHERE TOP OF STEEL DIFFERS BY 2 1/2" AND BOTTOM OF BEAM IS COPED.



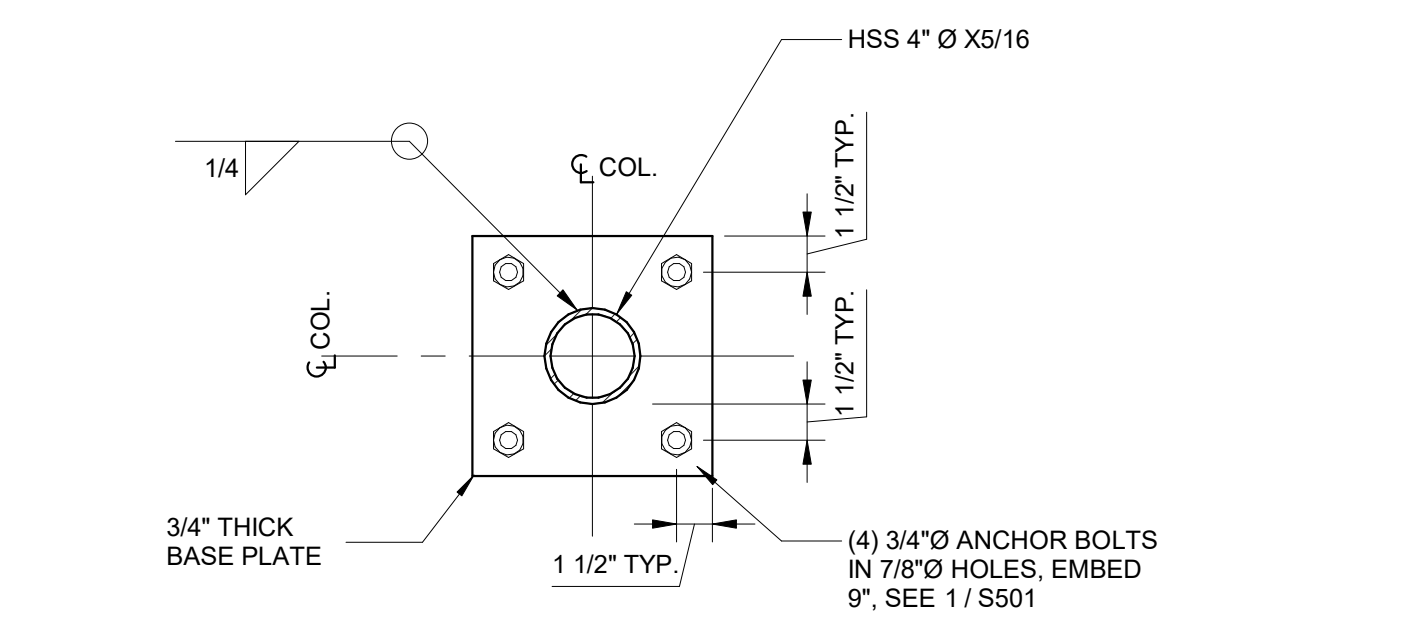
**2 HSS COLUMN TO BEAM CONNECTIONS**  
1 1/2" = 1'-0"



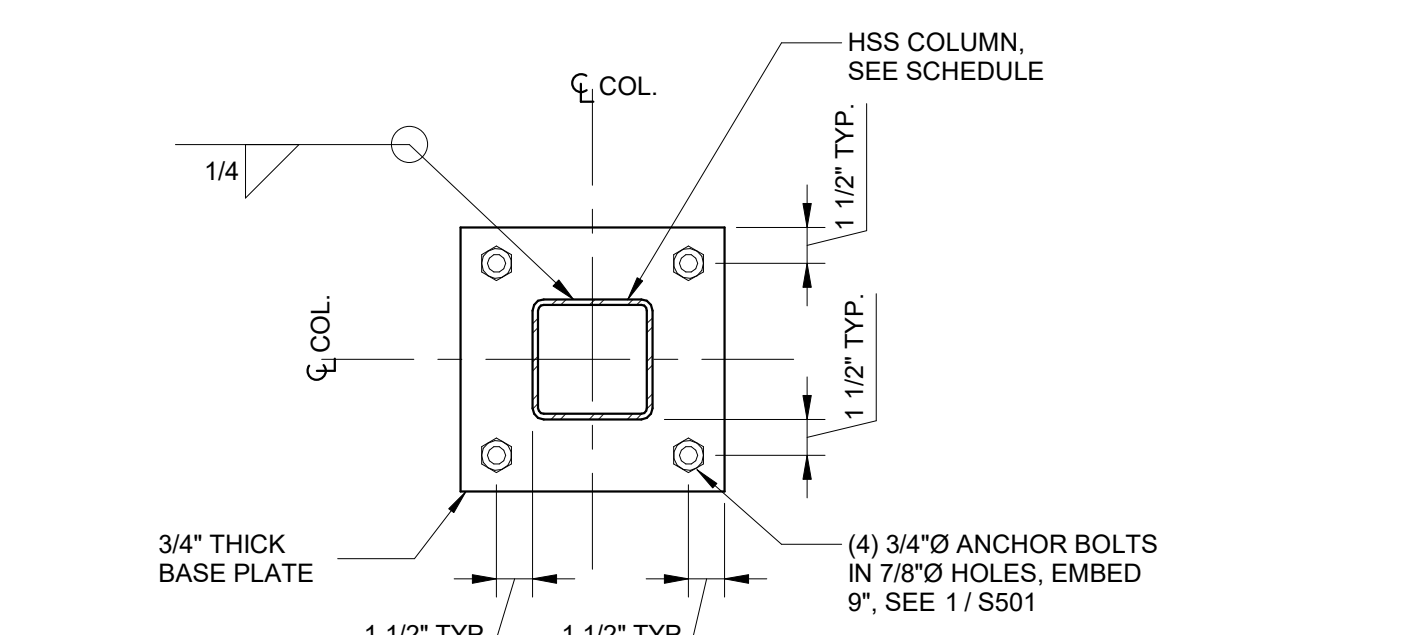
**2D HSS COLUMN TO BEAMS**



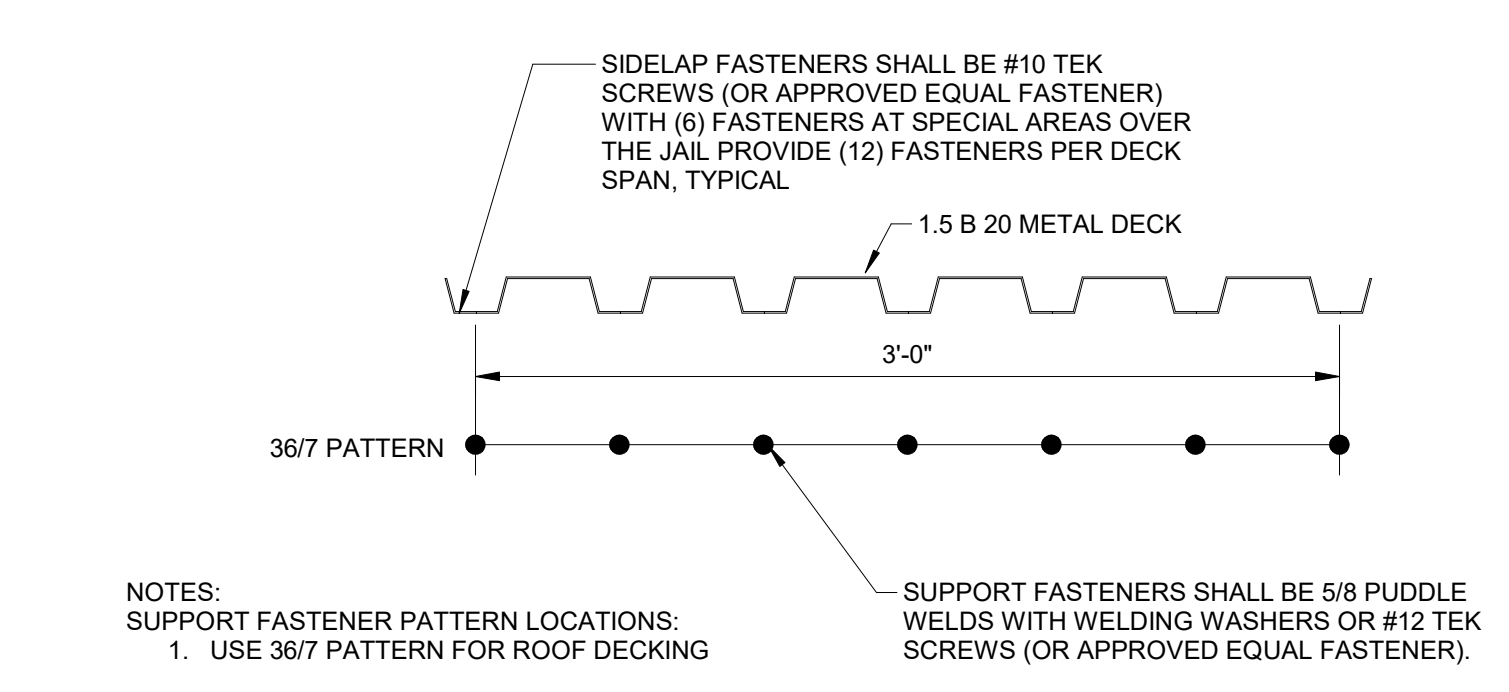
**3 BEARING PLATE DETAIL**  
1 1/2" = 1'-0"



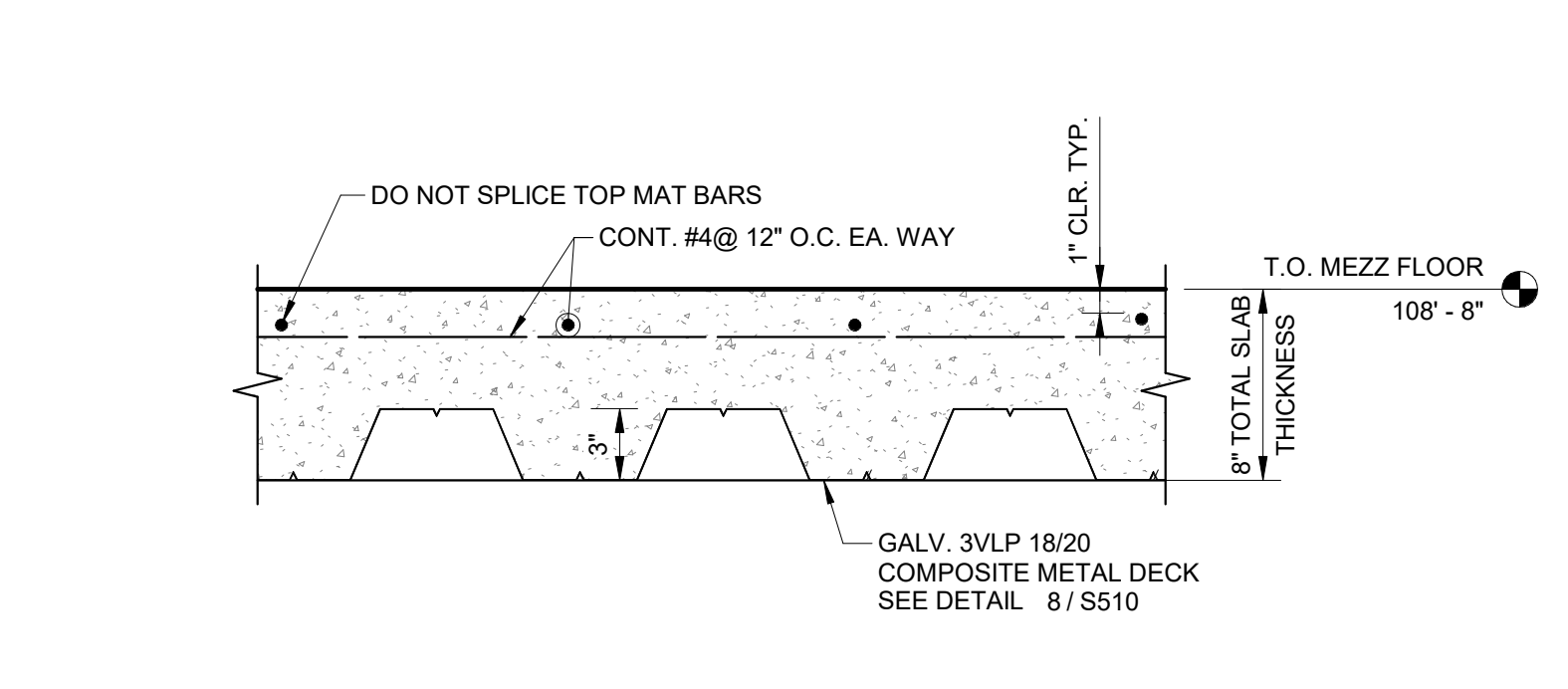
**4 HSS RD. BASE PLATE**  
1 1/2" = 1'-0"



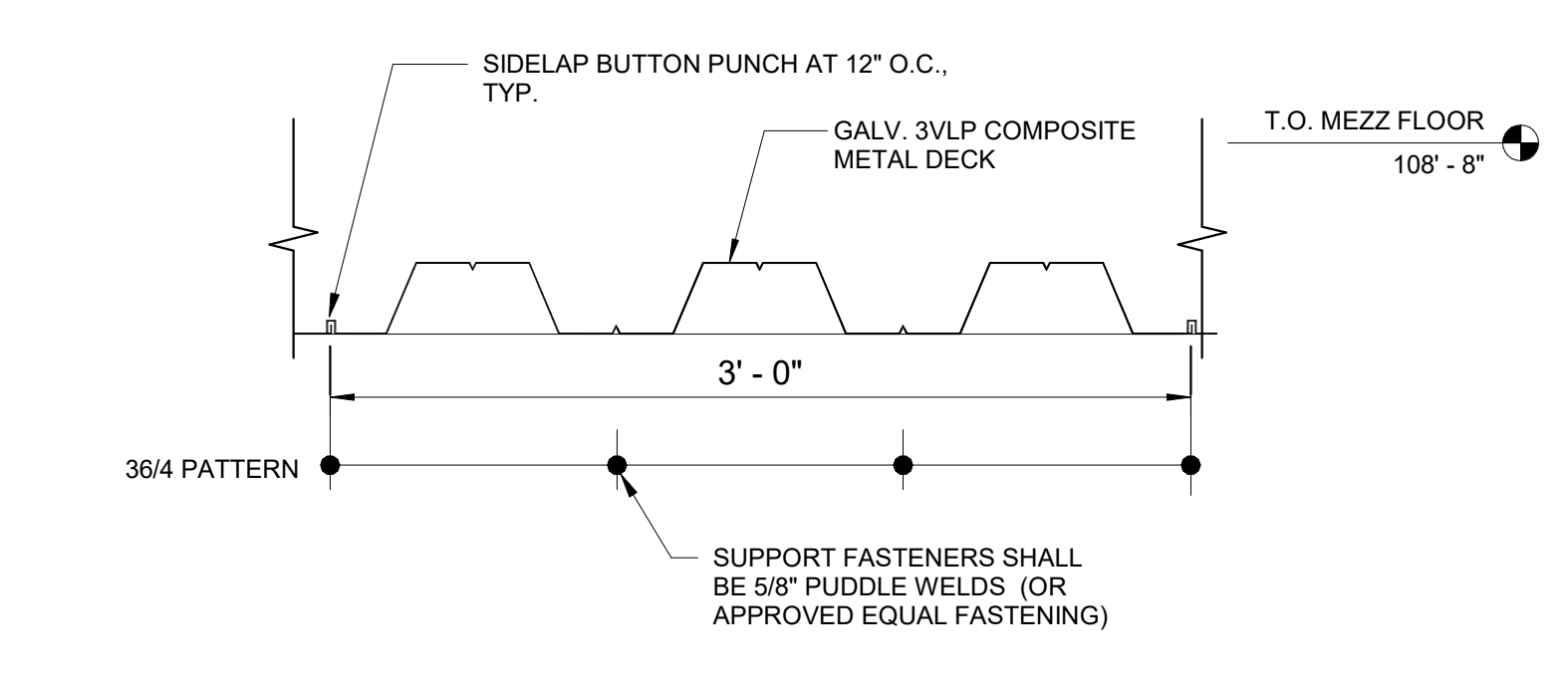
**5 HSS BASE PLATE**  
1 1/2" = 1'-0"



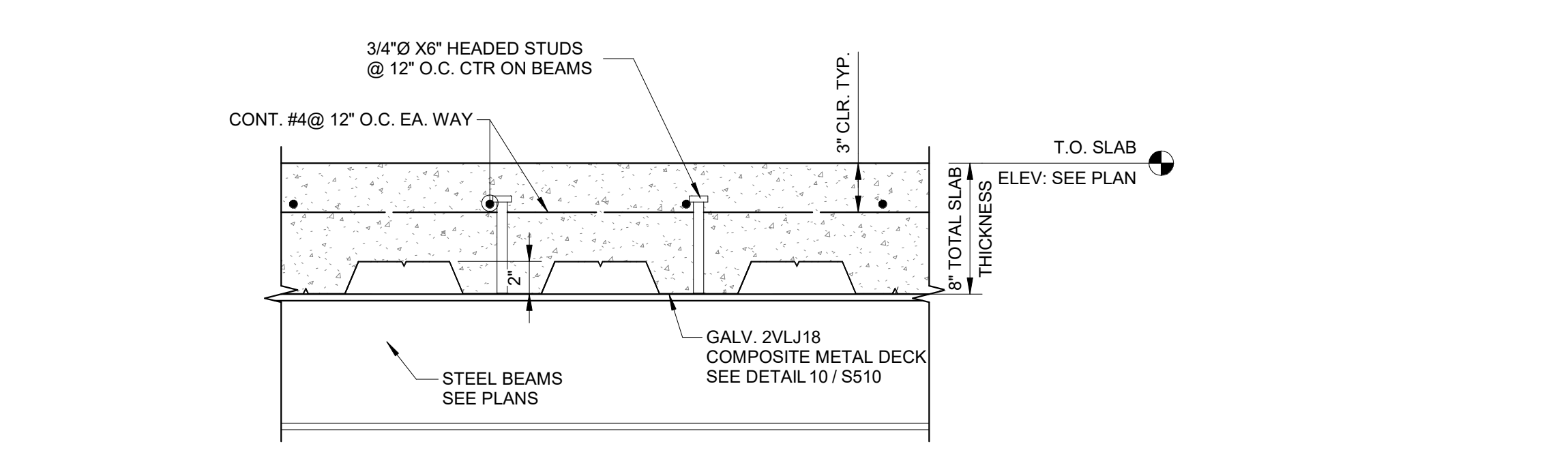
**6 1.5 B 20 METAL ROOF DECK**  
1 1/2" = 1'-0"



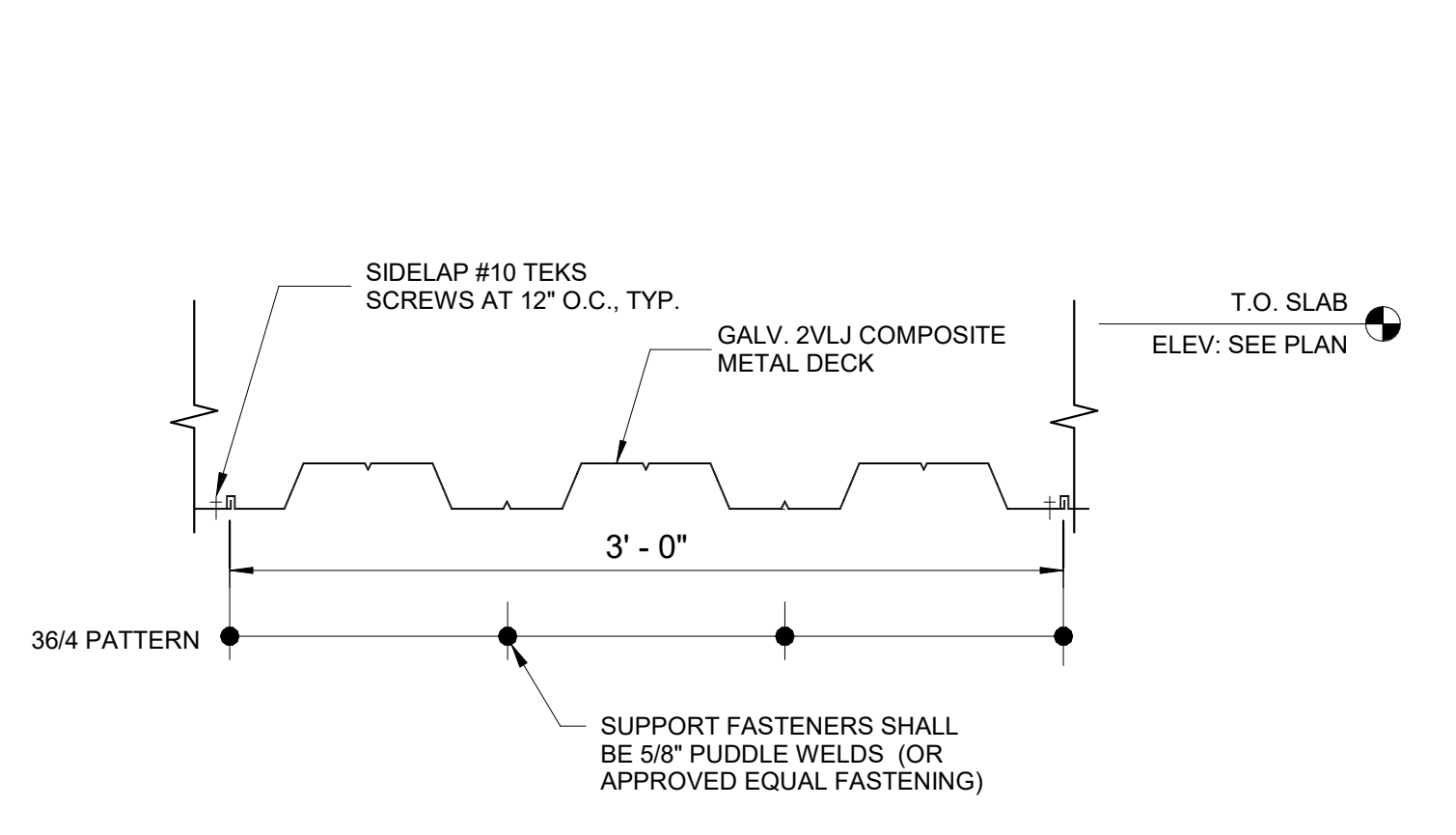
**7 COMPOSITE FLOOR SLAB DETAIL**  
1 1/2" = 1'-0"



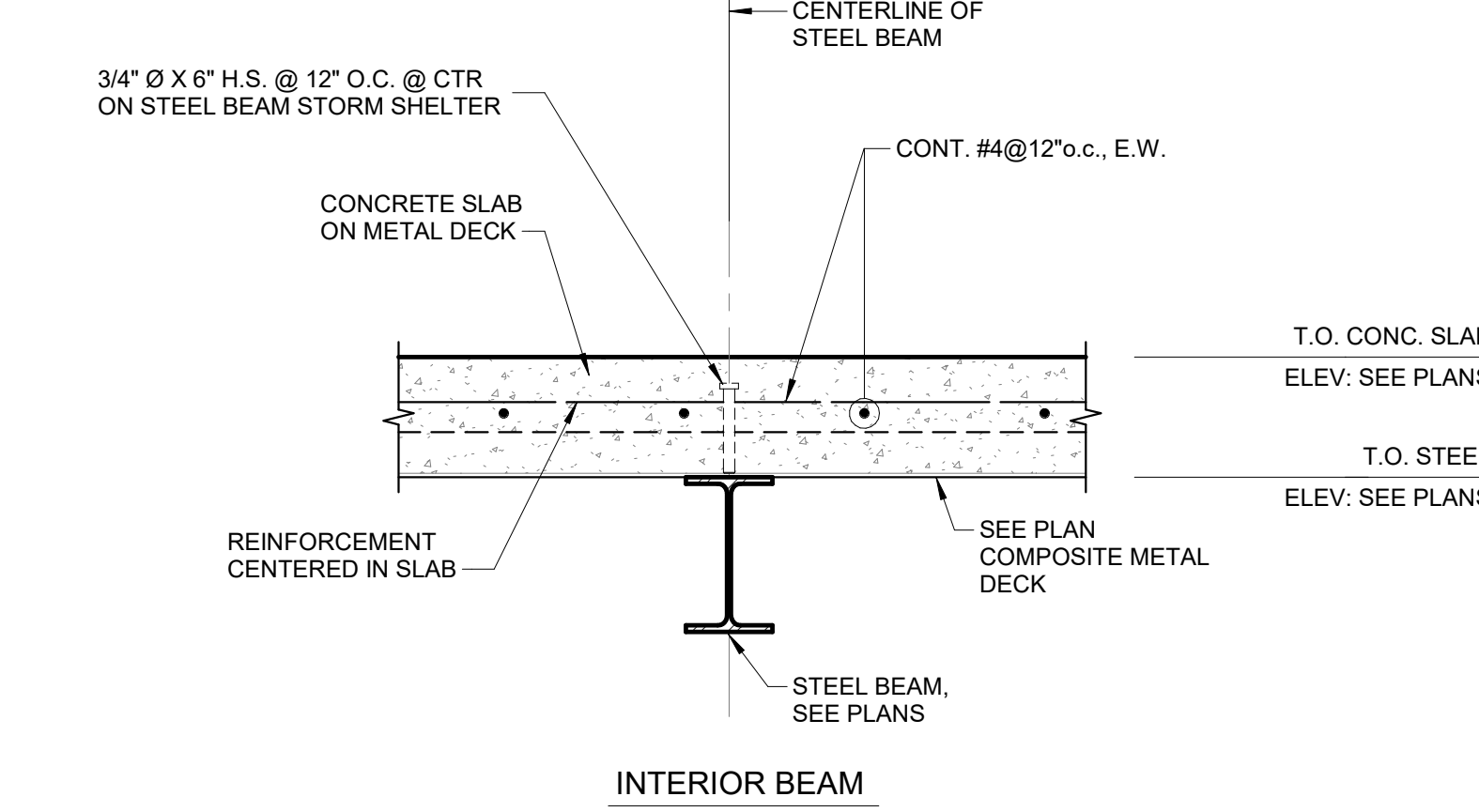
**8 TYPICAL 3VLP COMPOSITE METAL DECK**  
1 1/2" = 1'-0"



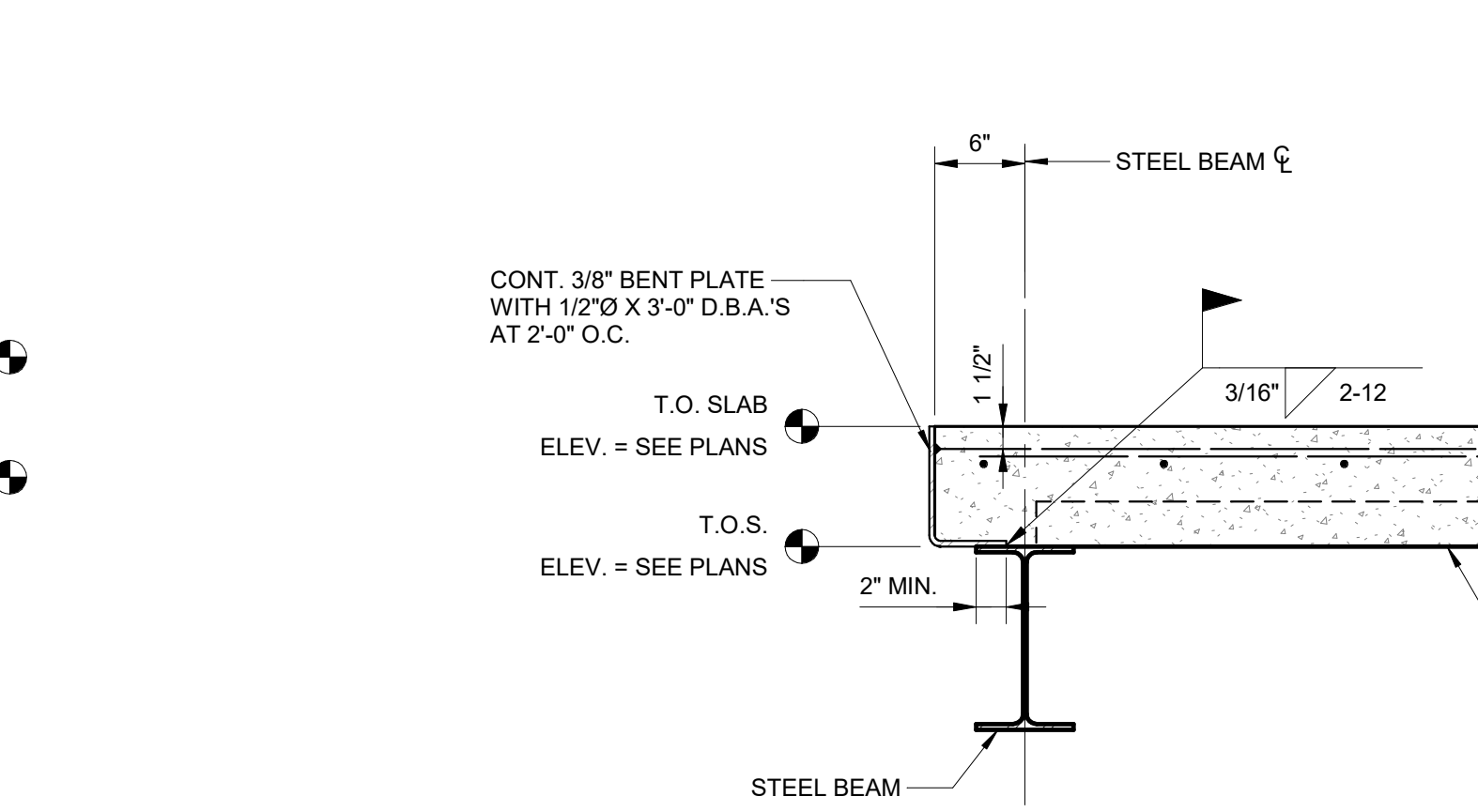
**9 2" COMPOSITE FLOOR SLAB DETAIL**  
1 1/2" = 1'-0"



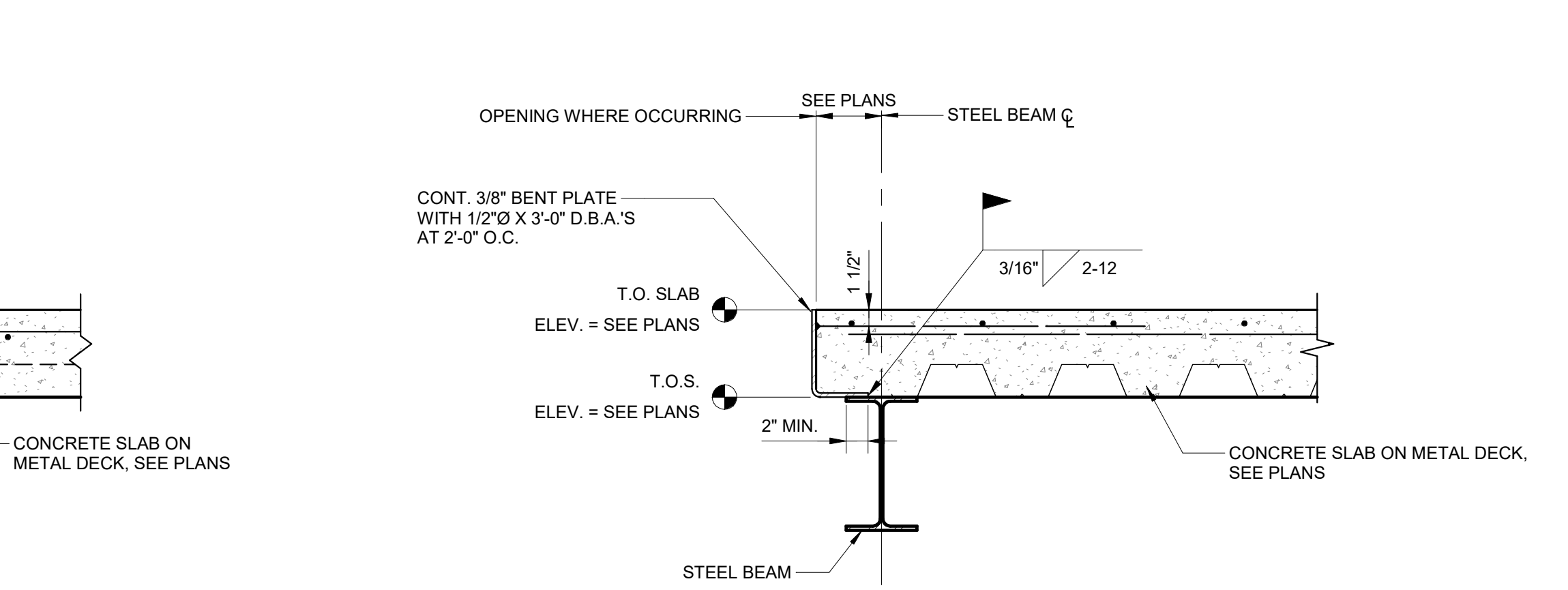
**10 TYPICAL 2VJL COMPOSITE METAL DECK**  
1 1/2" = 1'-0"



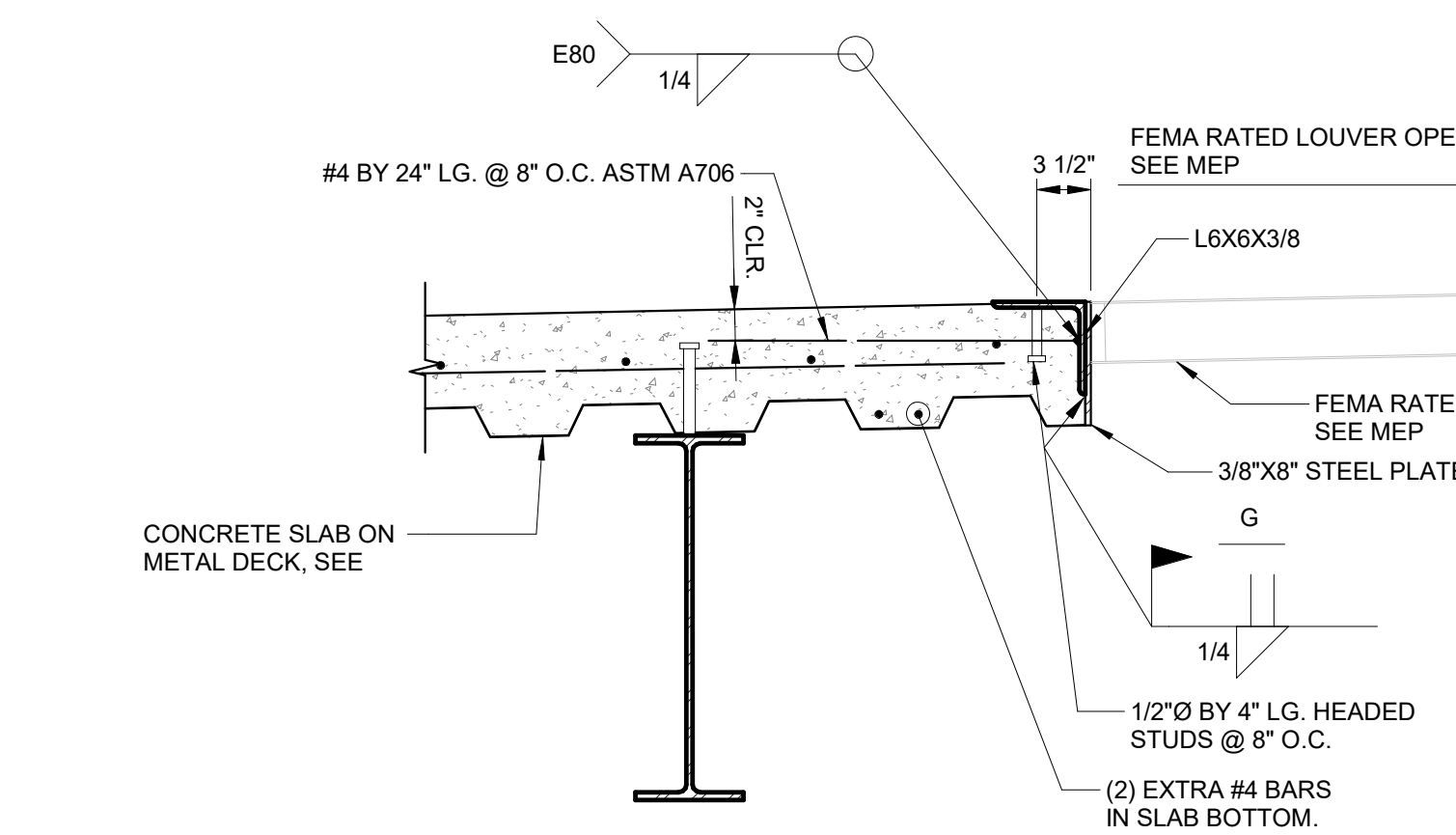
**11 CONCRETE SLAB - ON - COMPOSITE METAL DECK**  
1" = 1'-0"



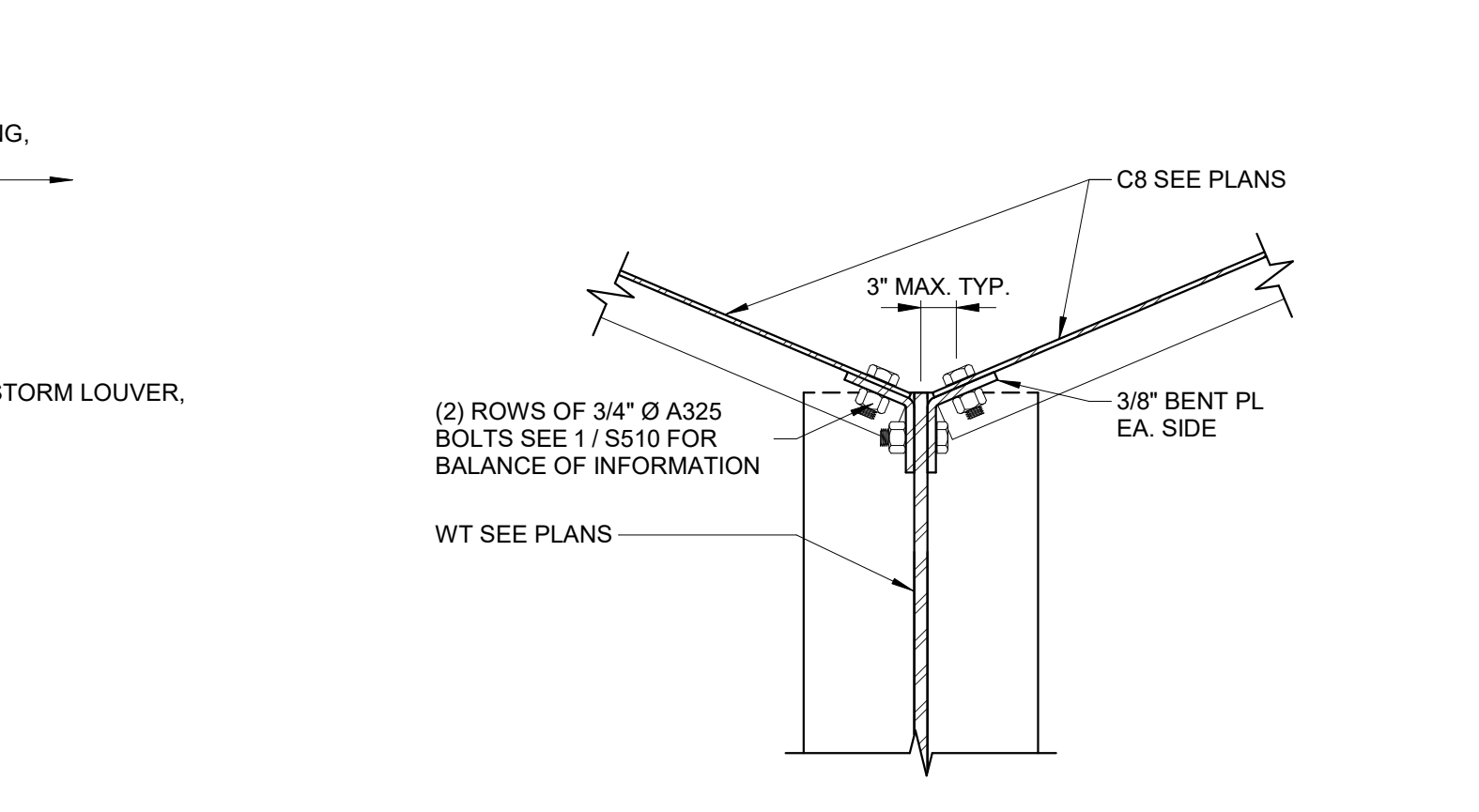
**12 SLAB EDGE BRG.**  
1" = 1'-0"



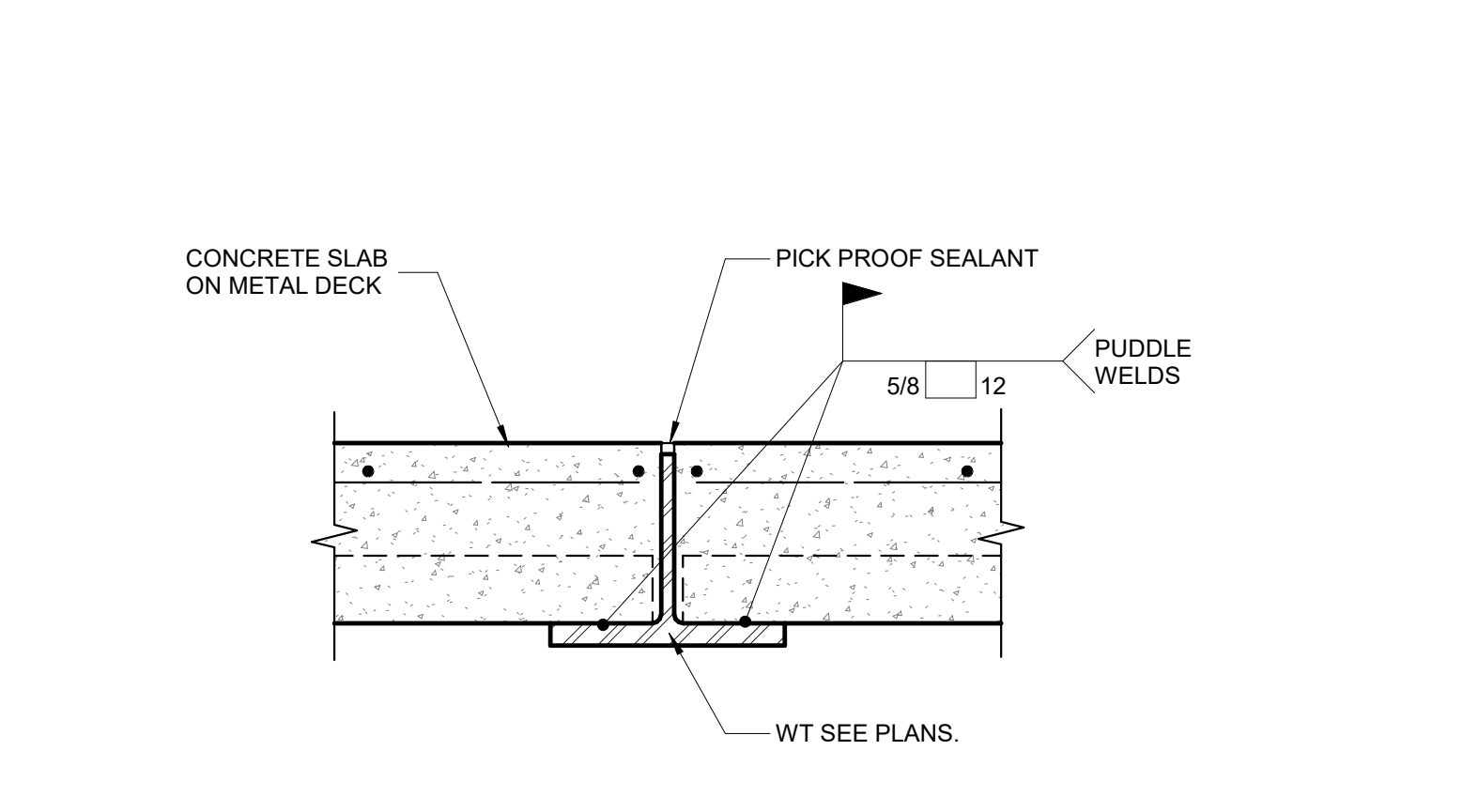
**13 SLAB EDGE NON-BRG.**  
1" = 1'-0"



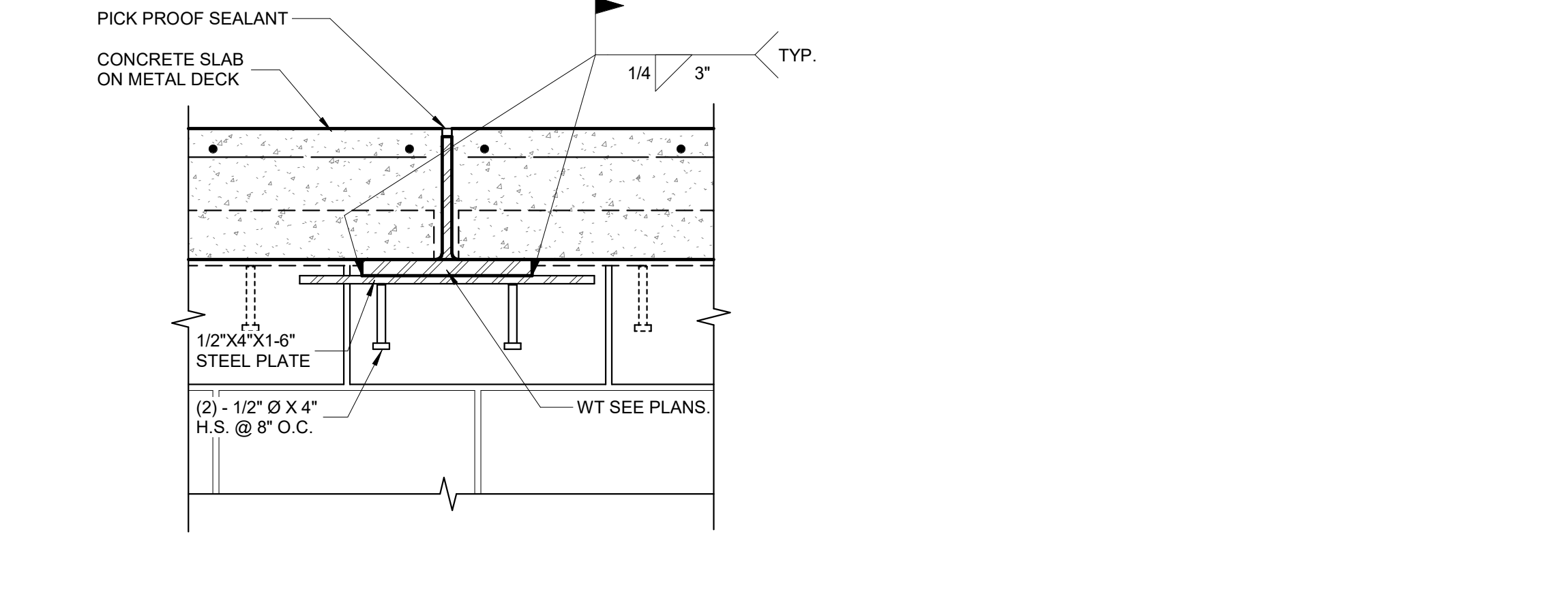
**14 STORM SHELTER LOUVER DETAIL**  
1" = 1'-0"



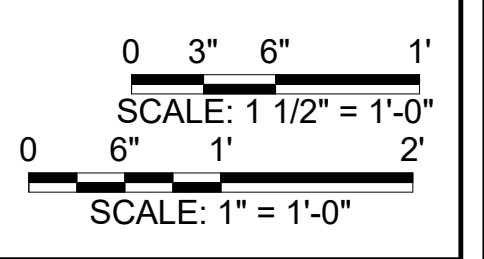
**15 C8 TO WT**  
1 1/2" = 1'-0"



**16 DECK BRG. AT WT**  
1 1/2" = 1'-0"



**17 WT BRG. ON CMU**  
1 1/2" = 1'-0"



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REVISION HISTORY	DESCRIPTION	DATE	APPR

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**EDGAR COUNTY PUBLIC SAFETY CENTER**  
EDGAR COUNTY, ILLINOIS  
12636 950TH ROAD  
PARIS, ILLINOIS 61944

Non-Reduced Sheet Size 30" x 42"  
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DESIGNED	DRAWN
JTS	NML

FIELD	FIELD BOOK
ADL	

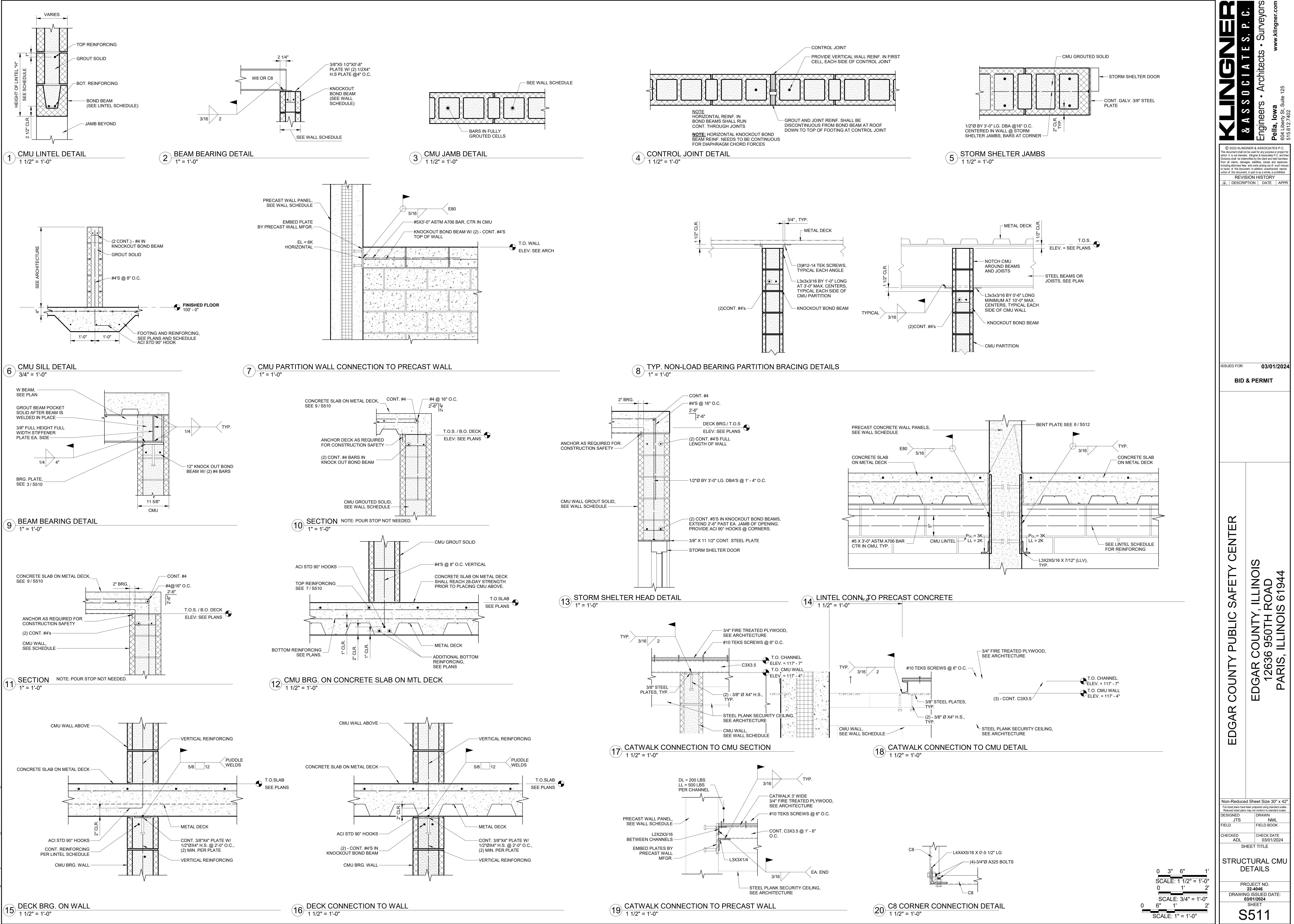
CHECKED: ADL      CHECK DATE: 03/01/2024

SHEET TITLE

**STRUCTURAL STEEL DETAILS**

PROJECT NO. 22-4048  
DRAWING ISSUED DATE: 03/01/2024  
SHEET

**S510**



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NO.	DESCRIPTION	DATE

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 12636 950TH ROAD  
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JTS	NML

FIELD	FIELD BOOK

CHECKED	CHECK DATE
ADL	03/01/2024

SHEET TITLE: **STRUCTURAL CMU DETAILS**

PROJECT NO: 22-4048  
 DRAWING ISSUED DATE: 03/01/2024  
**S511**

3/1/2024 12:42:06 PM Autodesk Docs\Edgar County Illinois 2022\S22 EDGAR COUNTY JAIL 22-0404.rvt

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 REVISION HISTORY  
 DESCRIPTION DATE APPR

DESCRIPTION	DATE	APPR

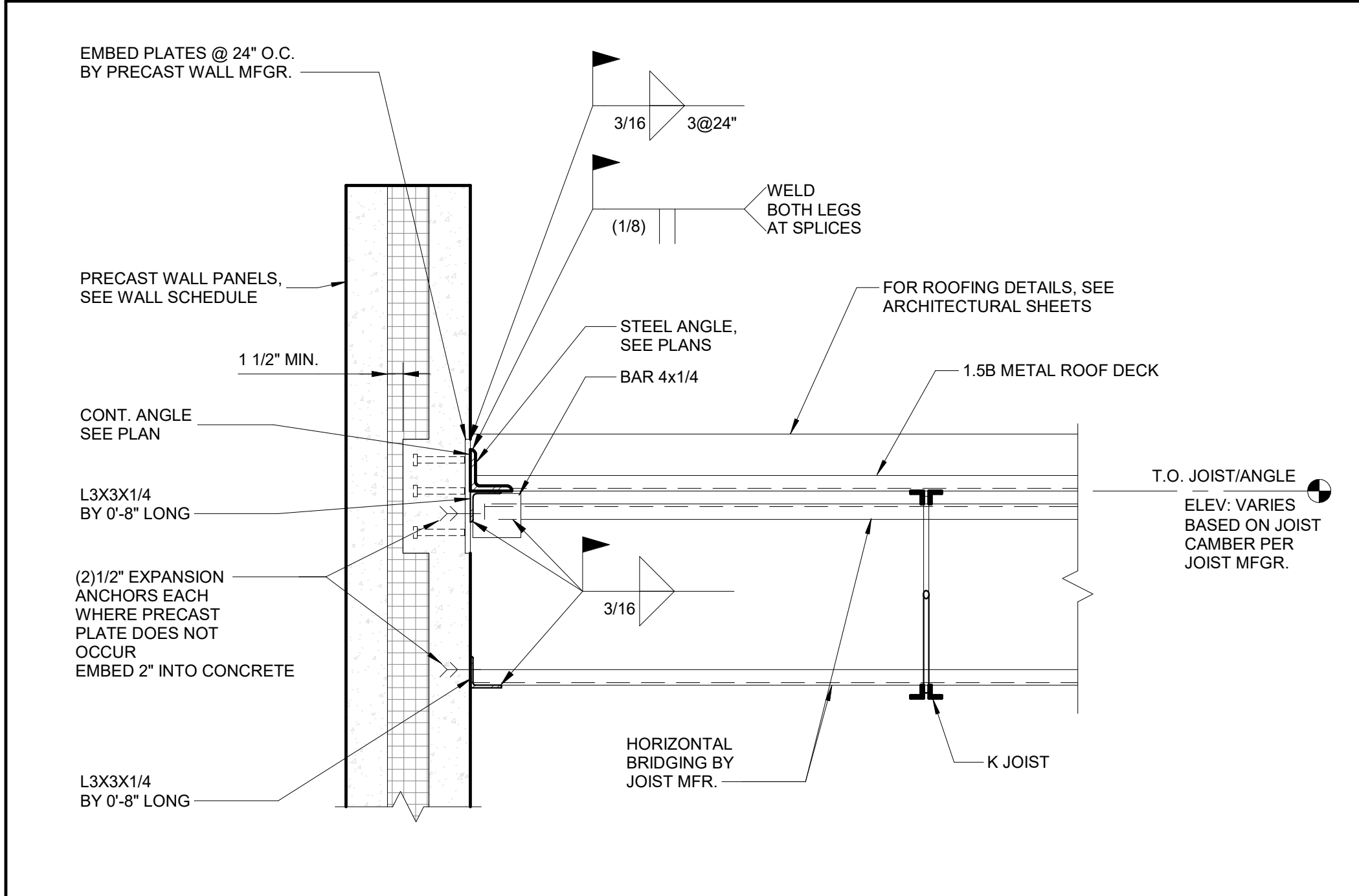
ISSUED FOR **03/01/2024**  
**BID & PERMIT**

EDGAR COUNTY PUBLIC SAFETY CENTER  
 EDGAR COUNTY, ILLINOIS  
 12636 950TH ROAD  
 PARIS, ILLINOIS 61944

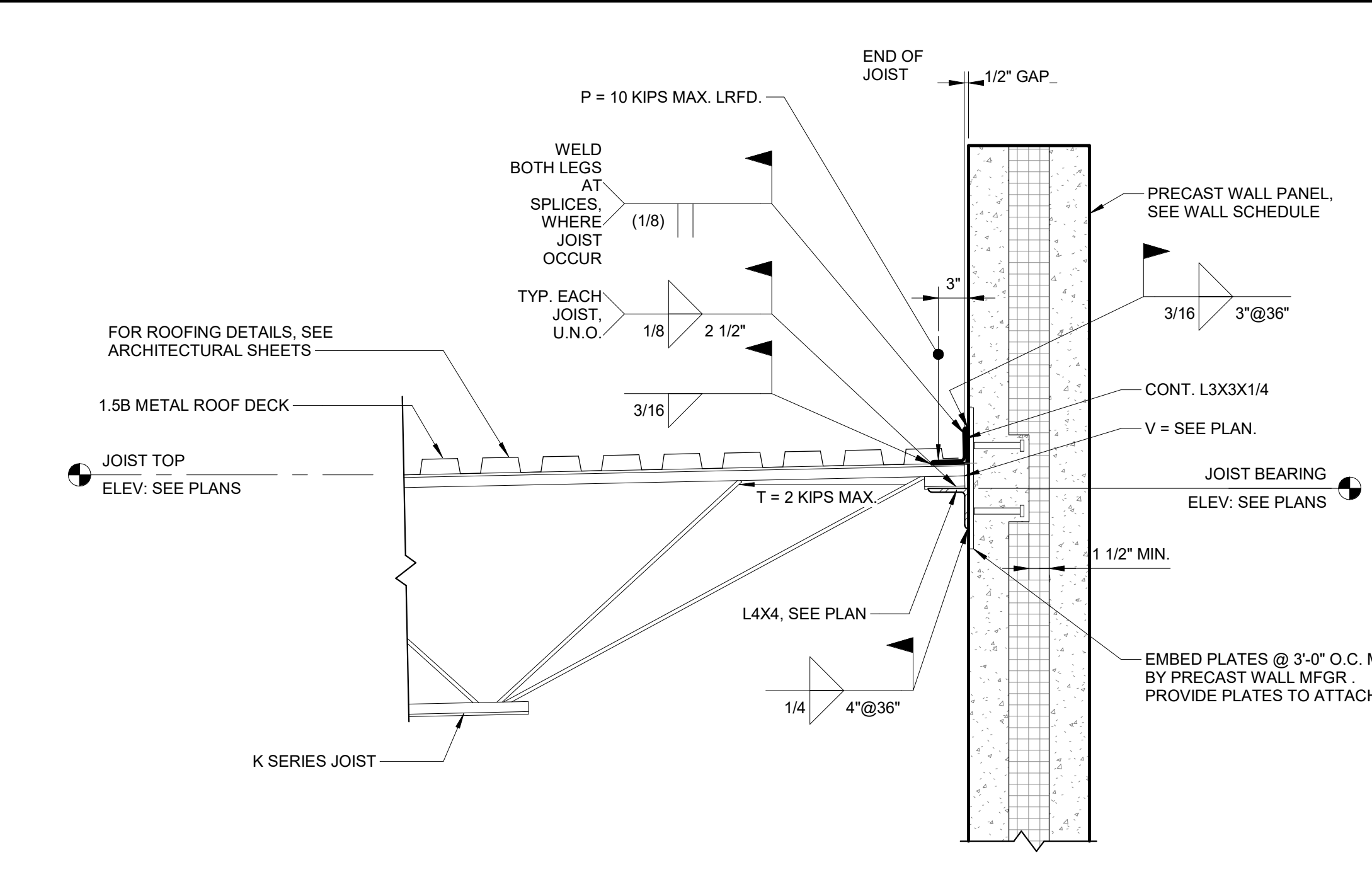
EDGAR COUNTY PUBLIC SAFETY CENTER  
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DESIGNED	JTS	NML
DRAWN	ADL	FIELD BOOK
CHECKED	ADL	CHECK DATE
		03/01/2024
SHEET TITLE		

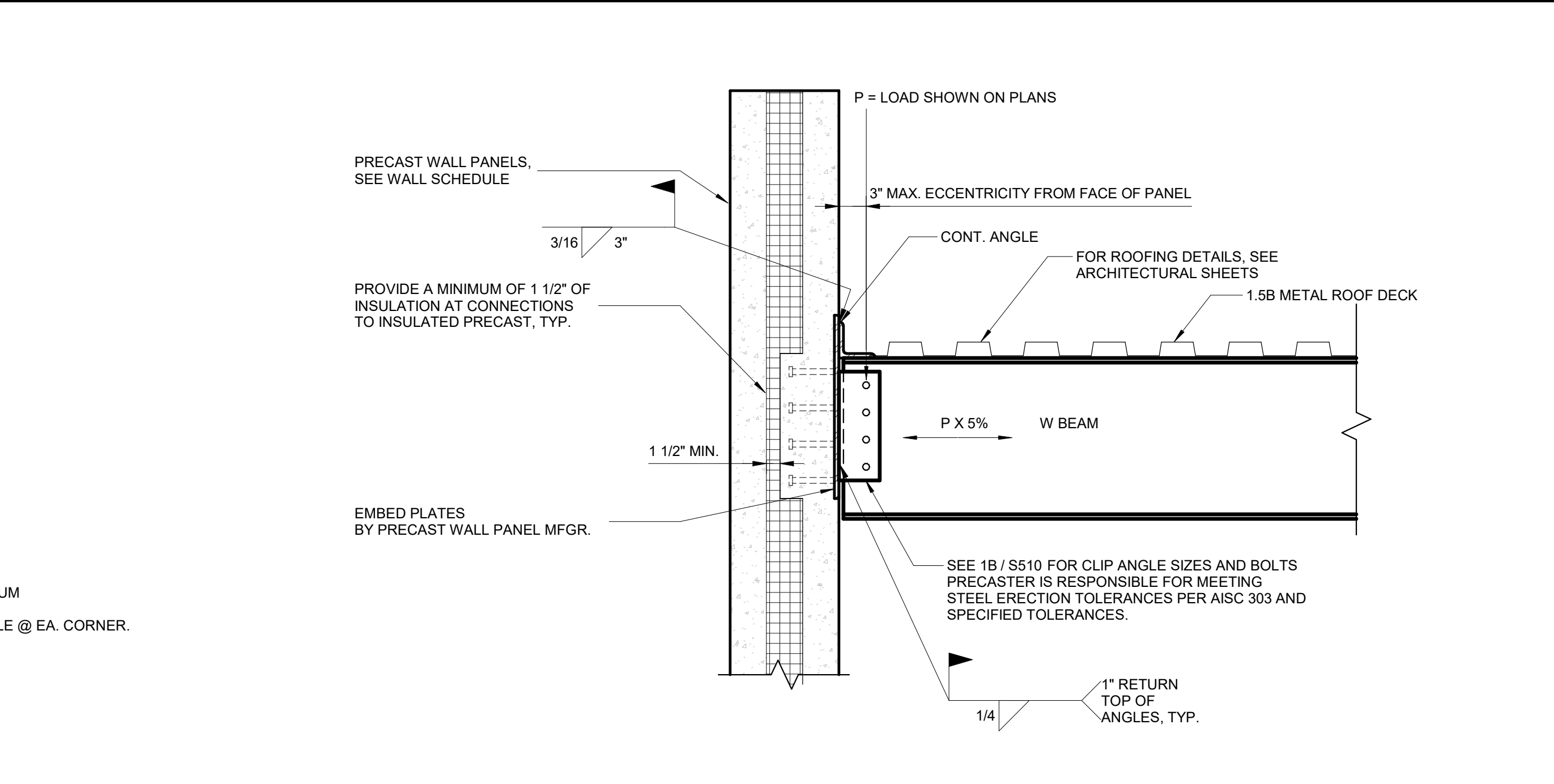
**STRUCTURAL PRECAST DETAILS**  
 PROJECT NO. 22-4048  
 DRAWING ISSUED DATE: 03/01/2024  
**S512**



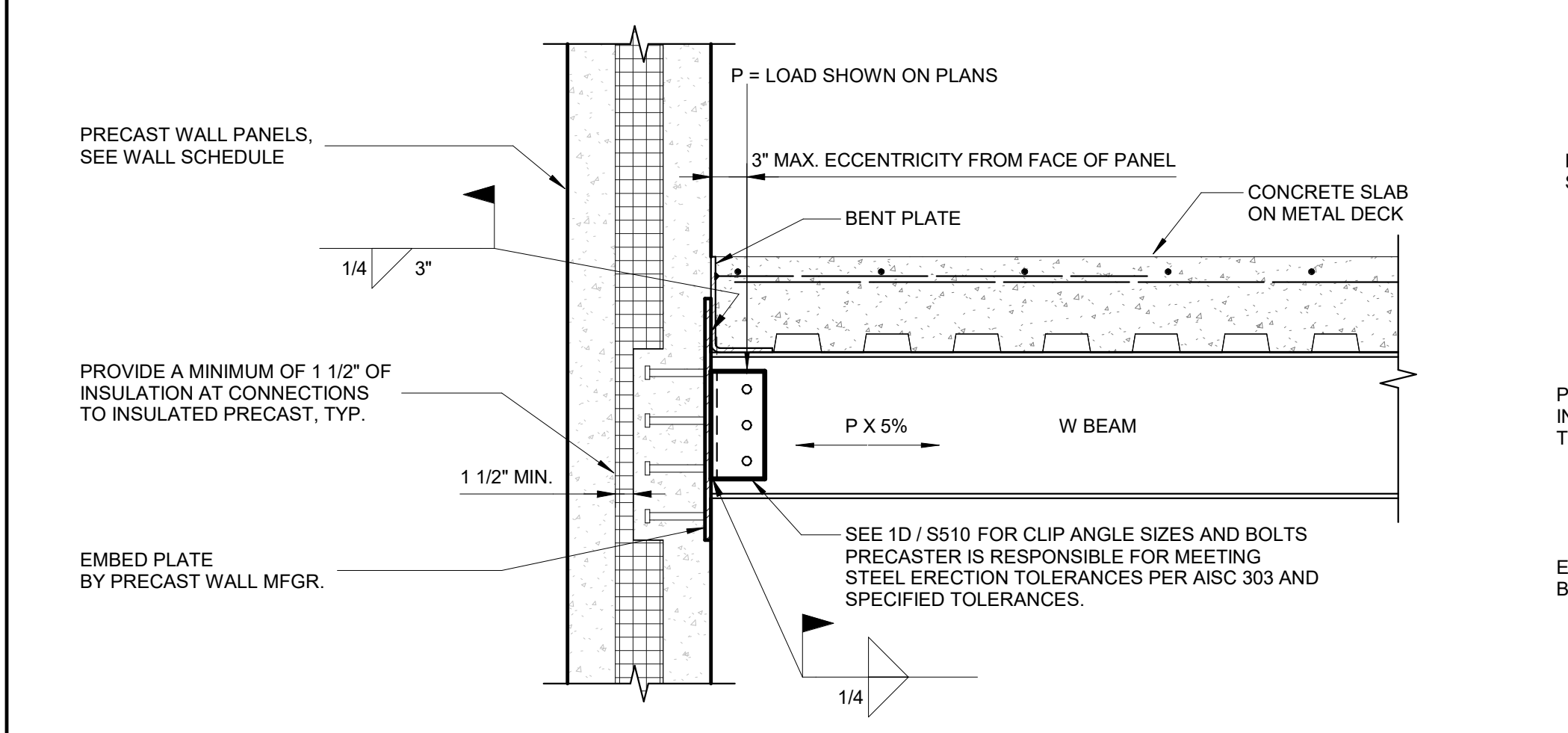
**1 STEEL JOIST BRIDGING CONNECTION**  
 1" = 1'-0"



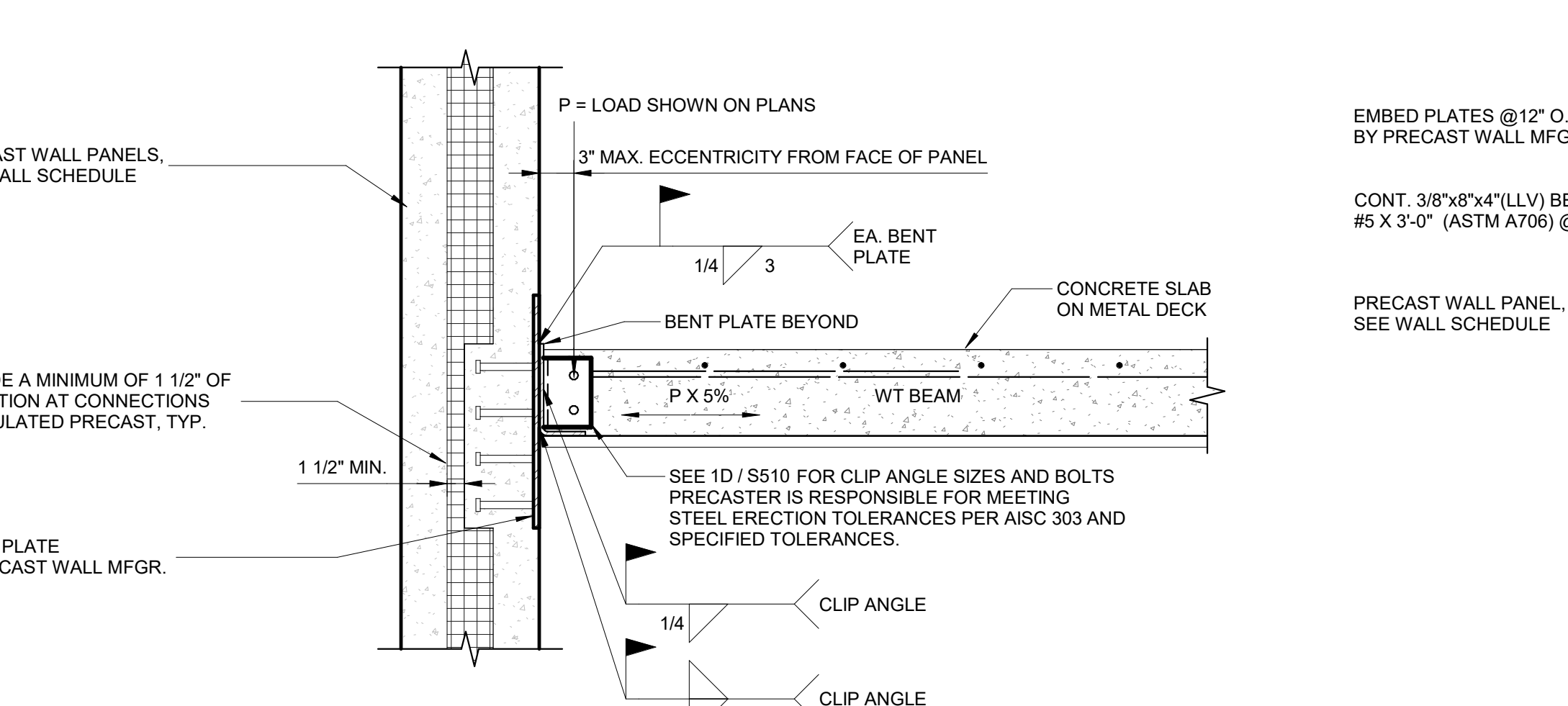
**2 STEEL JOIST BRIDGING AT PRECAST WALL**  
 1" = 1'-0"



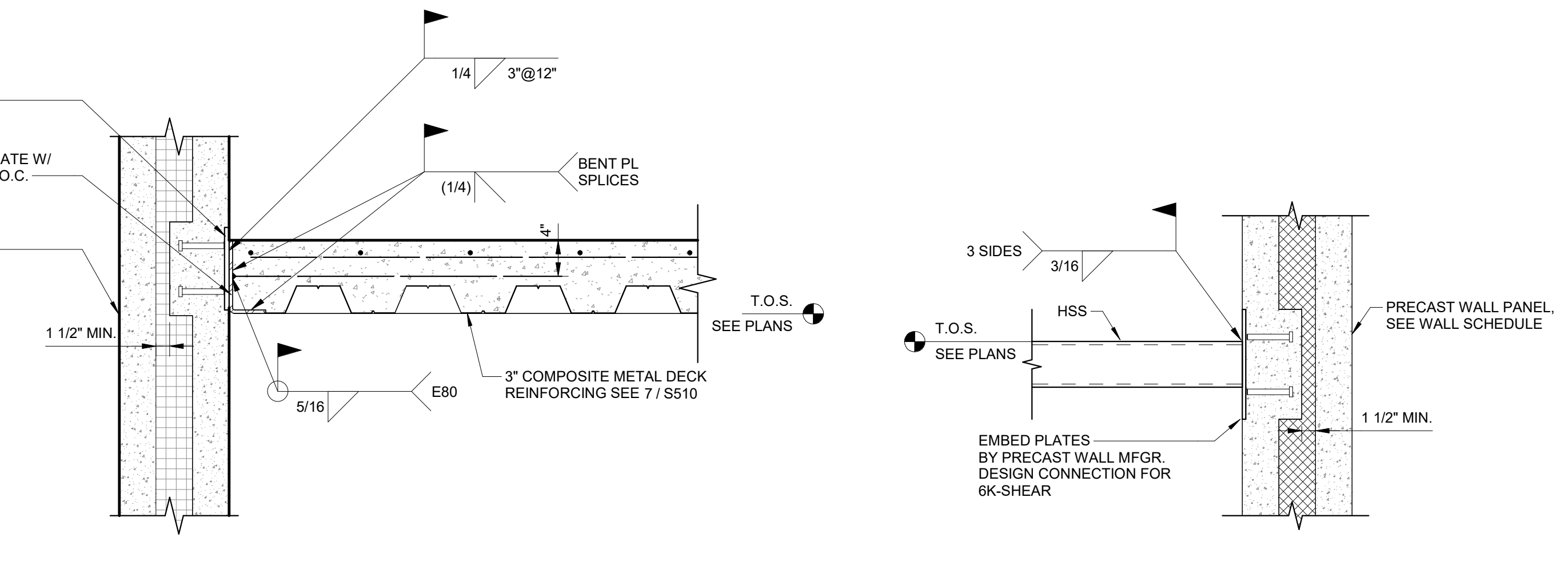
**3 BEAM CONNECTION AT PRECAST**  
 1" = 1'-0"



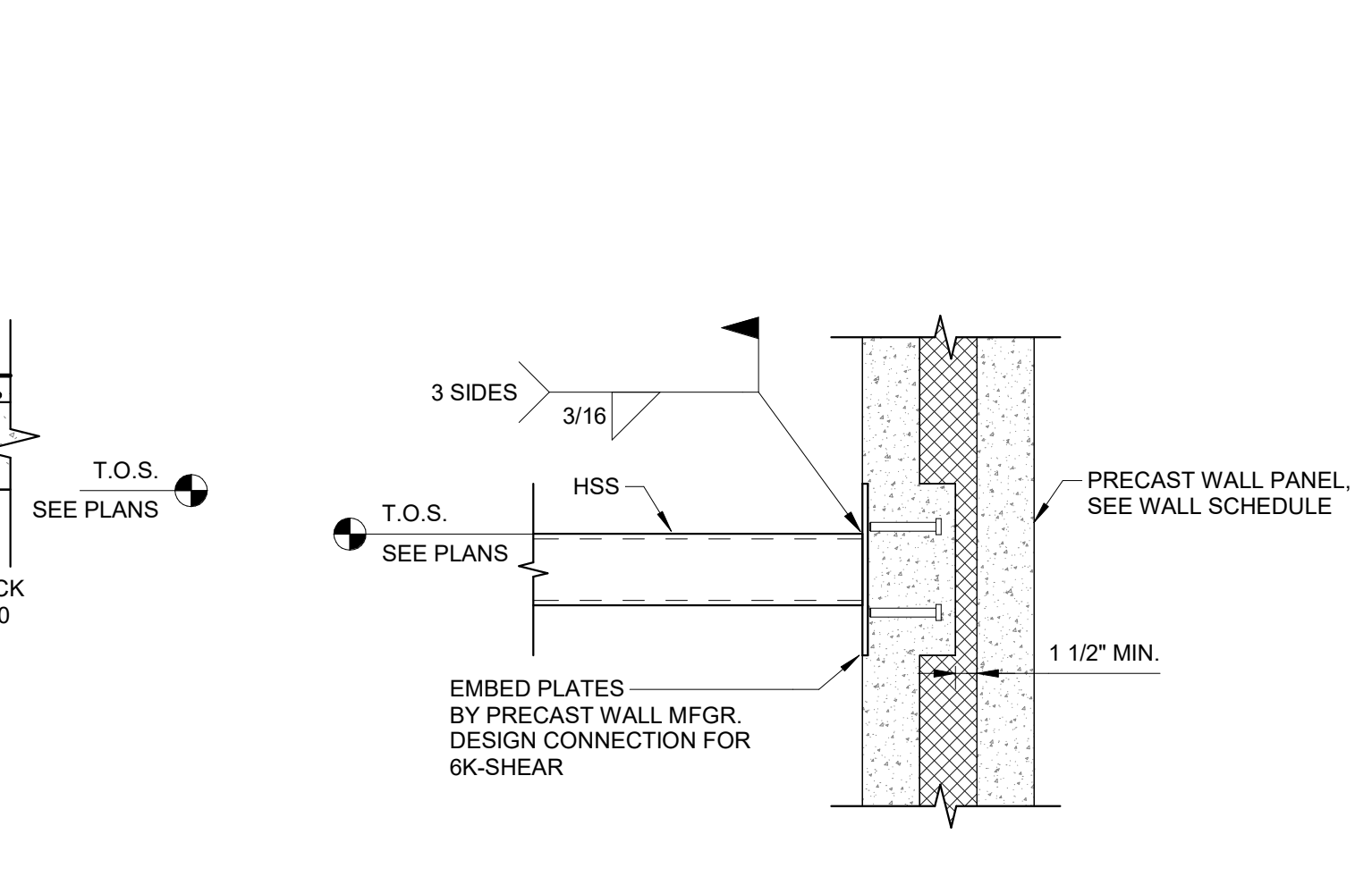
**4 BEAM CONNECTION AT PRECAST**  
 1" = 1'-0"



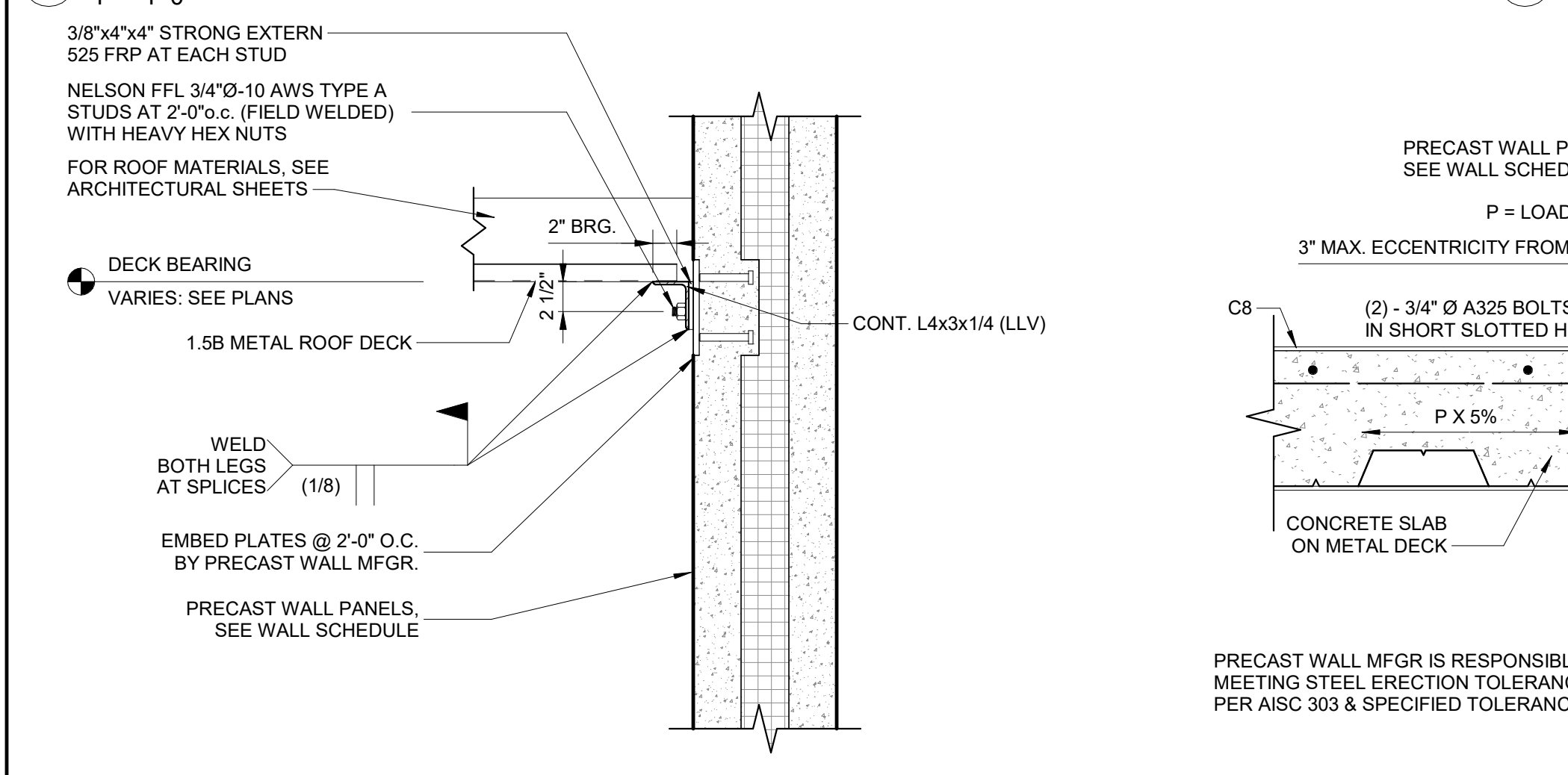
**5 BEAM CONNECTION AT PRECAST**  
 1" = 1'-0"



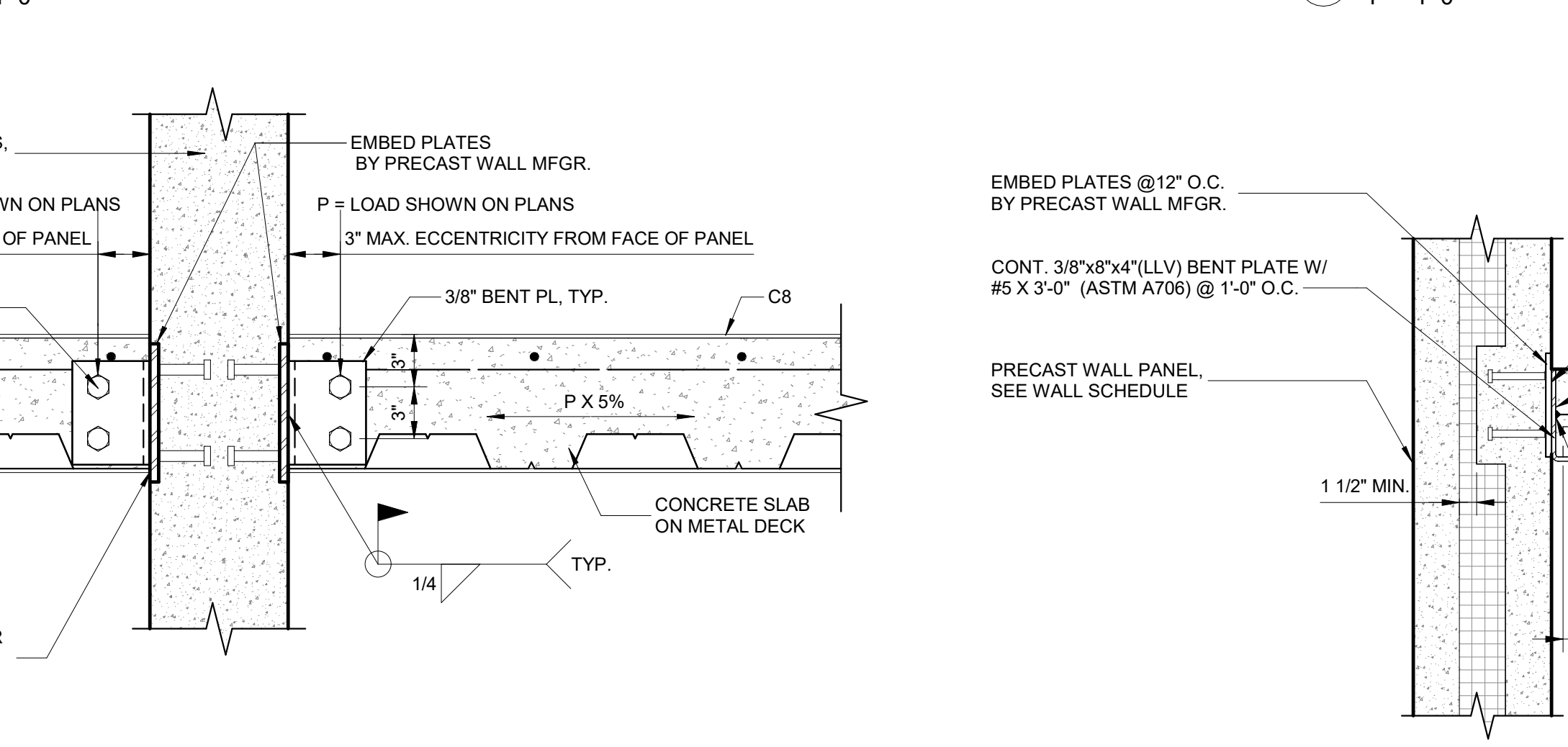
**6 MEZZANINE DECK TO PRECAST SECTION**  
 1" = 1'-0"



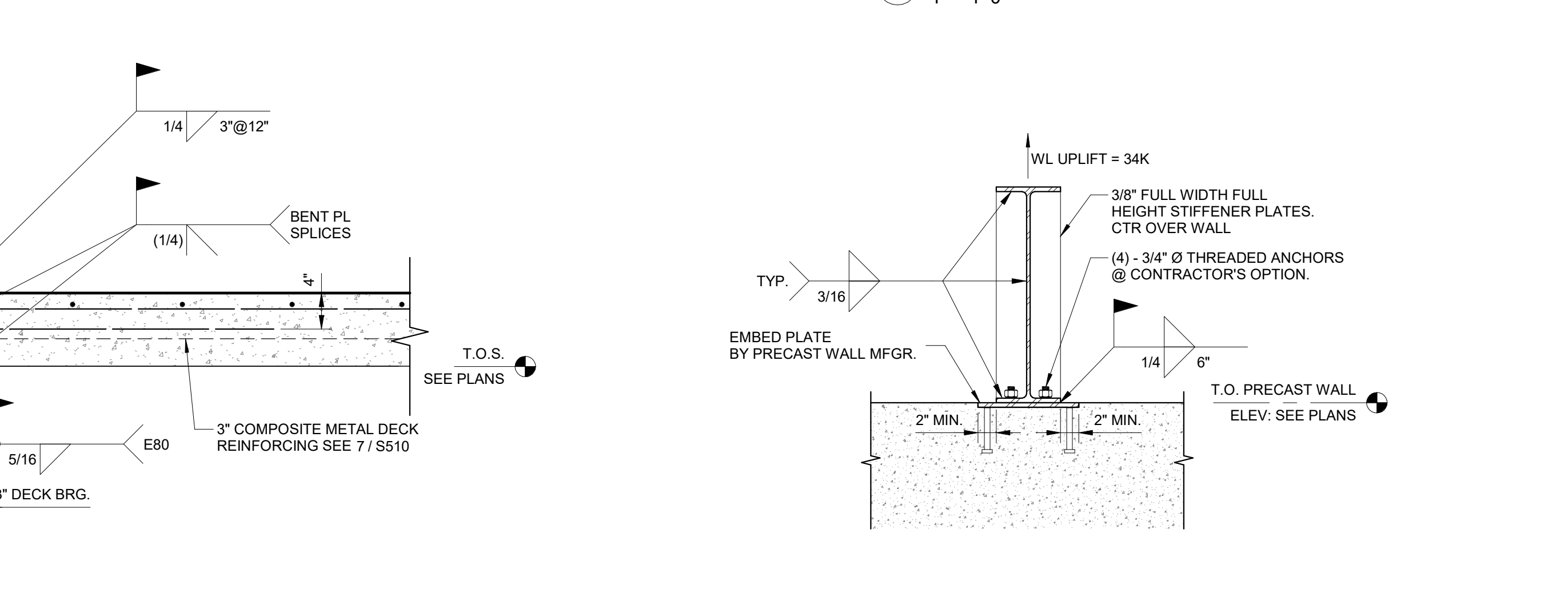
**7 HSS TO PRECAST CONNECTION**  
 1" = 1'-0"



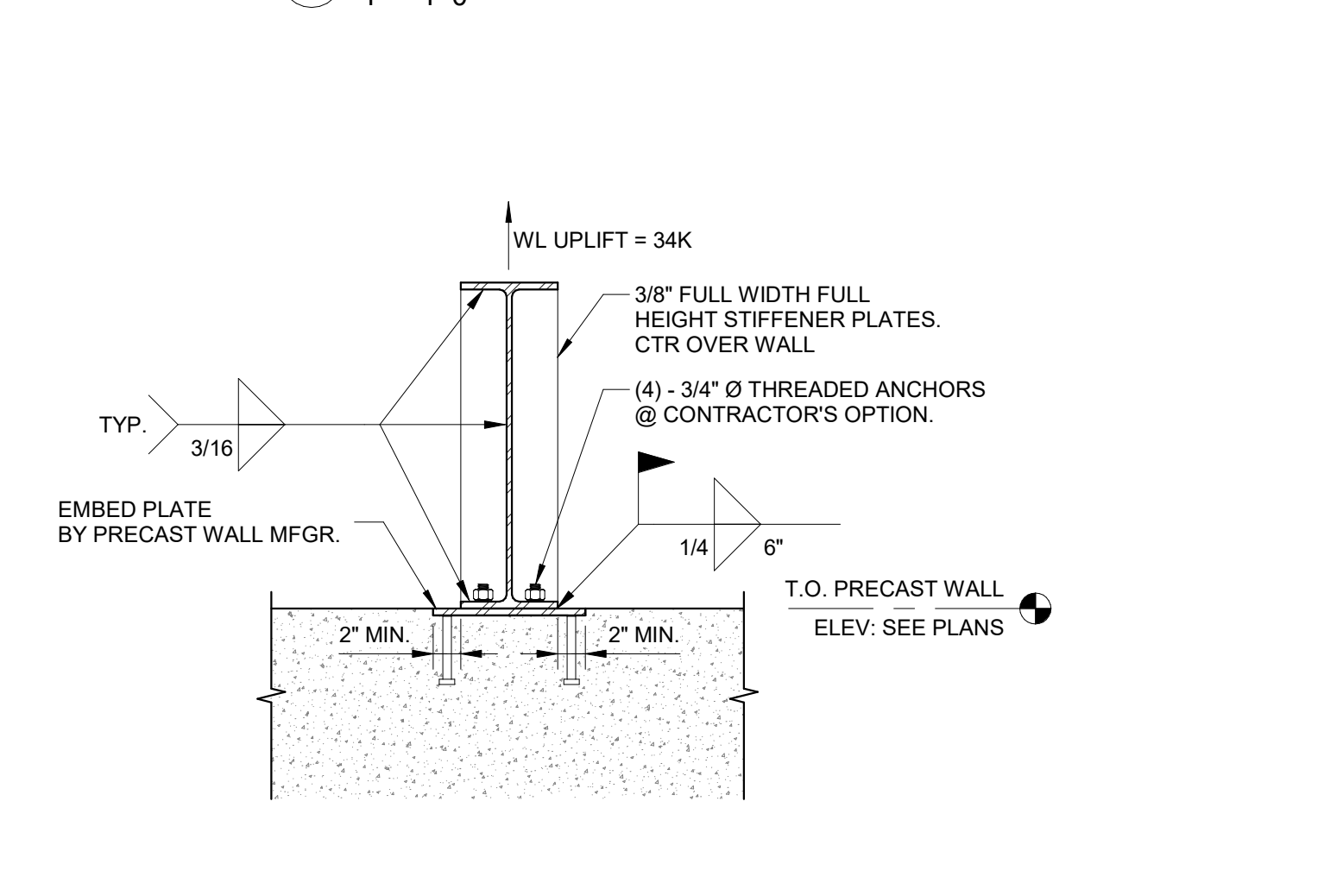
**8 DECK TO PRECAST SECTION**  
 1" = 1'-0"



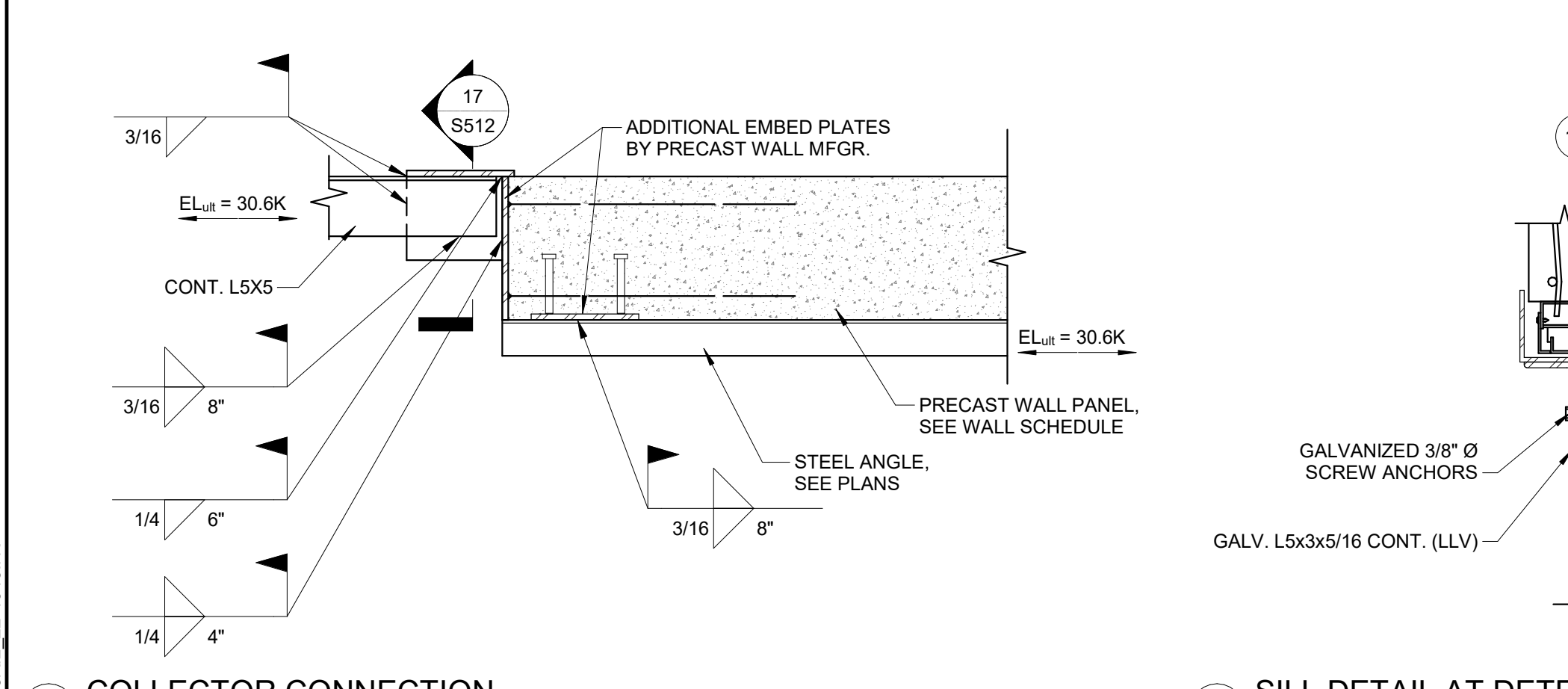
**9 C8 CONNECTION TO PRECAST**  
 1 1/2" = 1'-0"



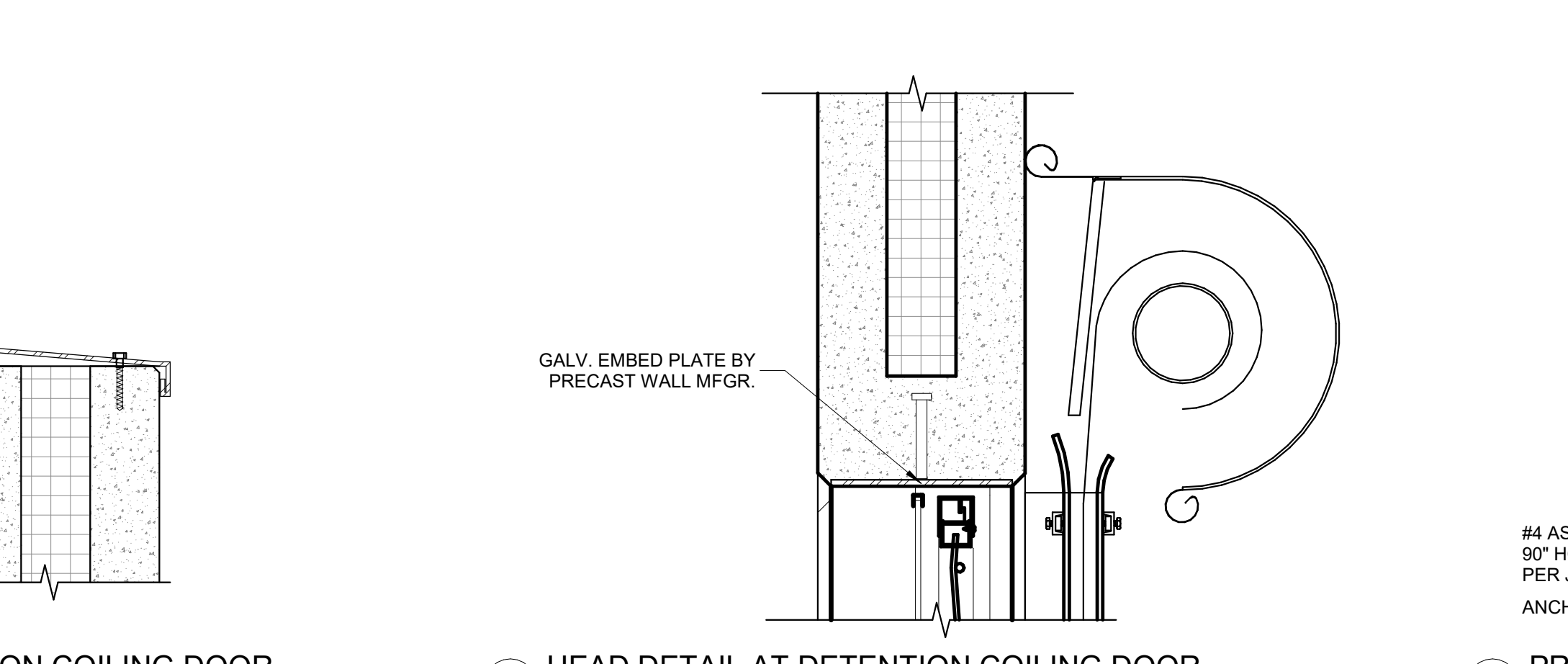
**10 MEZZANINE DECK TO PRECAST SECTION**  
 1" = 1'-0"



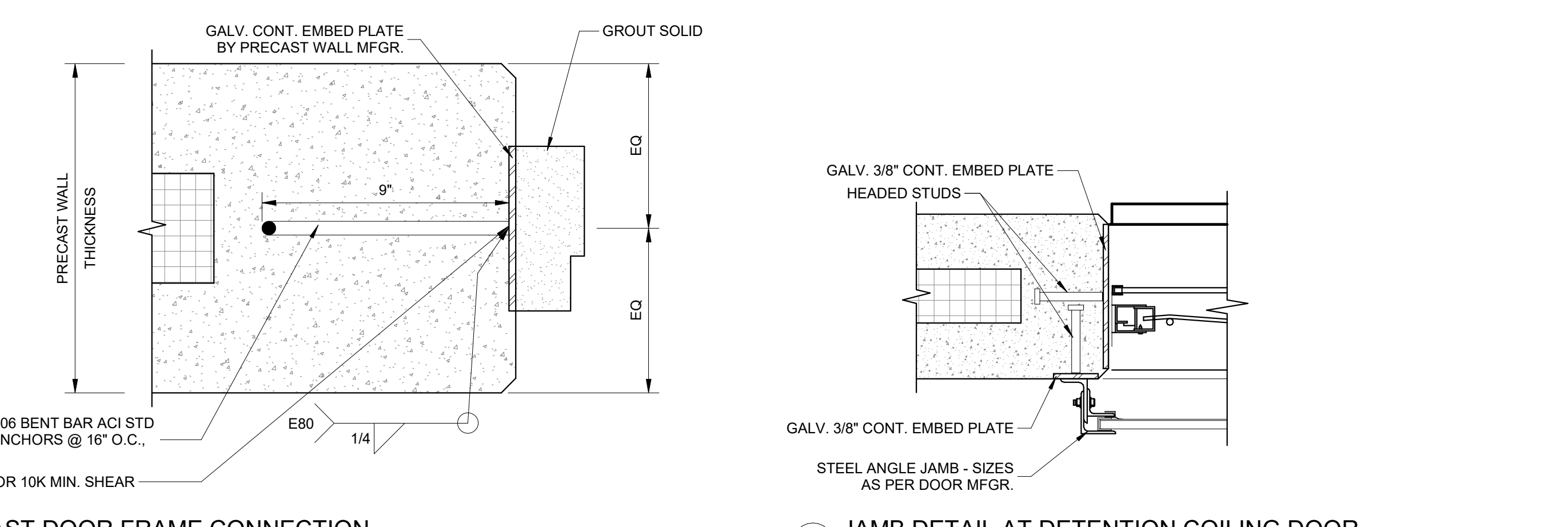
**11 STEEL GIRDER BRG. ON PRECAST WALL**  
 1" = 1'-0"



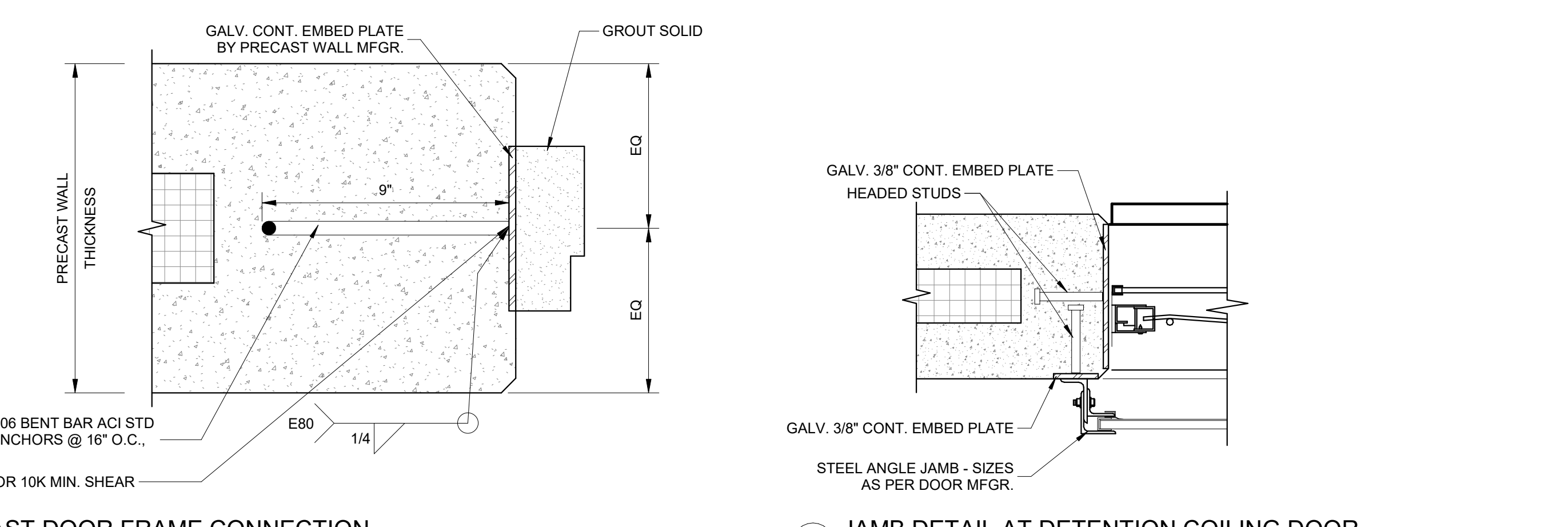
**12 COLLECTOR CONNECTION**  
 1" = 1'-0"



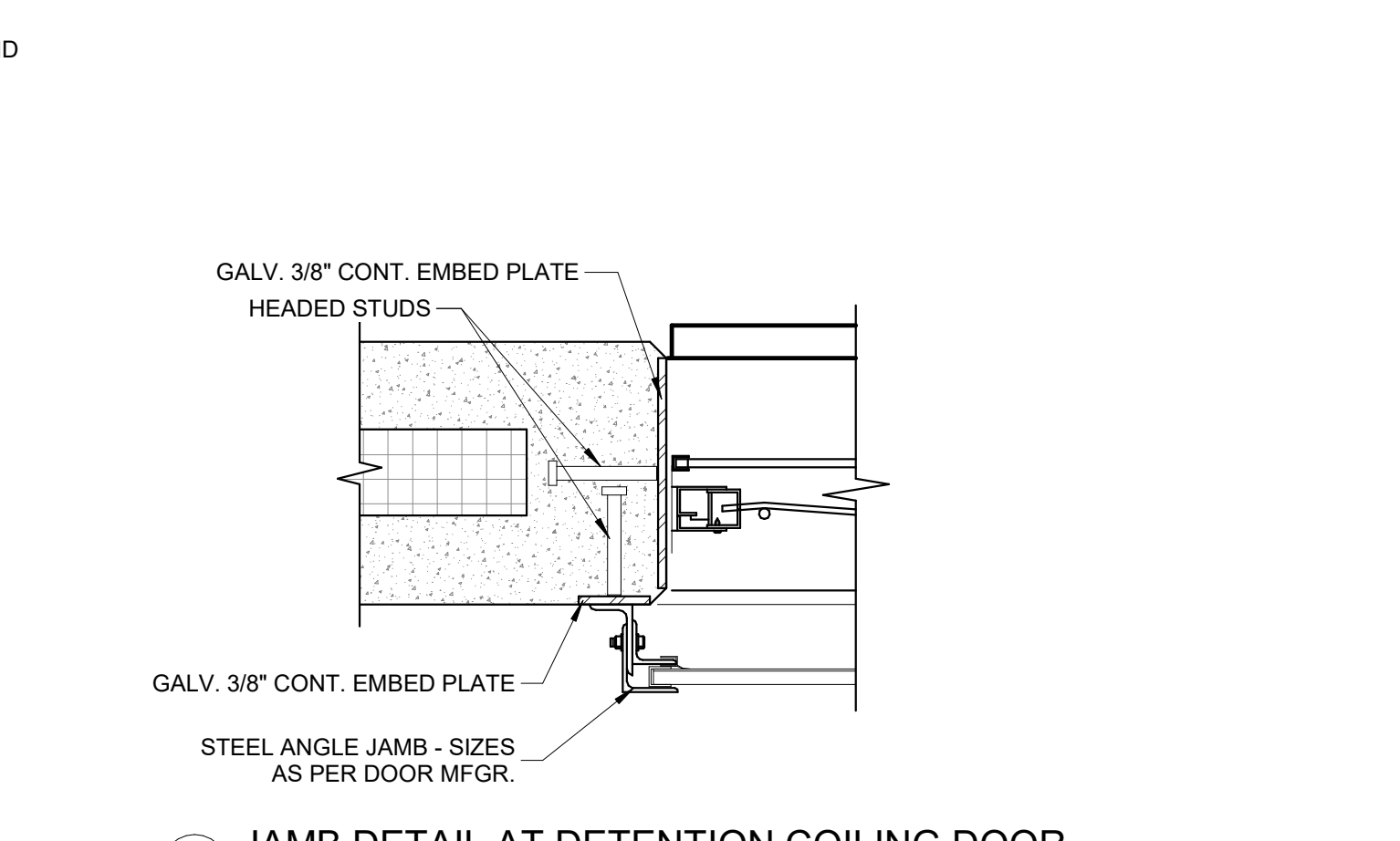
**13 SILL DETAIL AT DETENTION COILING DOOR**  
 1 1/2" = 1'-0"



**14 HEAD DETAIL AT DETENTION COILING DOOR**  
 1 1/2" = 1'-0"



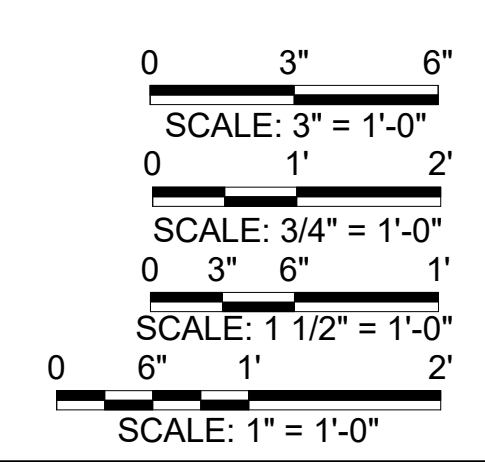
**15 PRECAST DOOR FRAME CONNECTION**  
 3" = 1'-0"



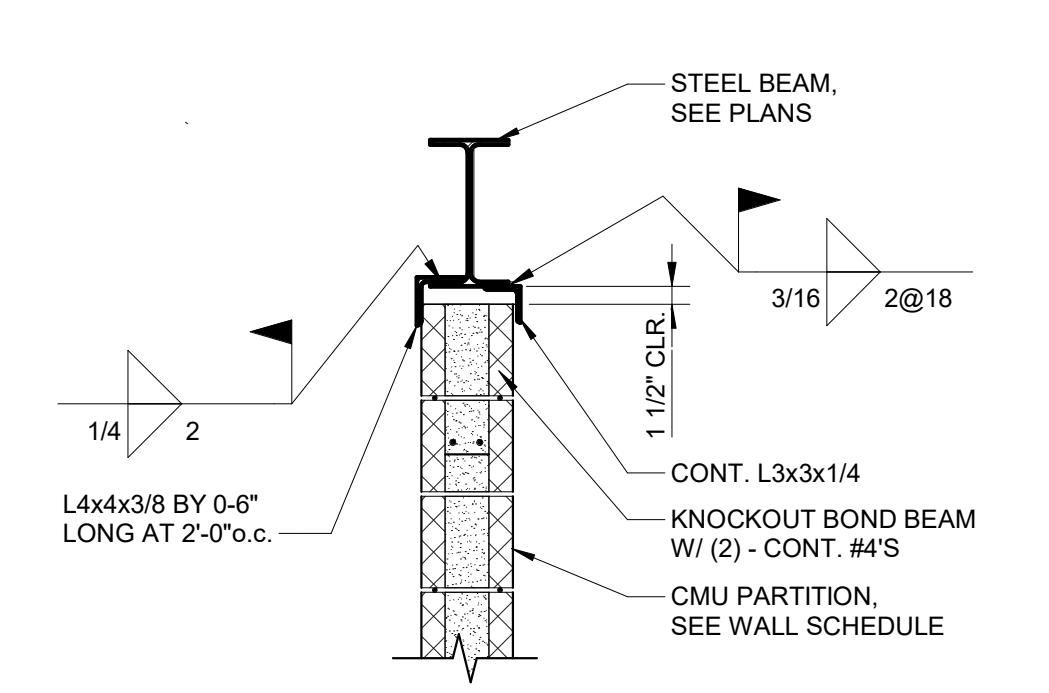
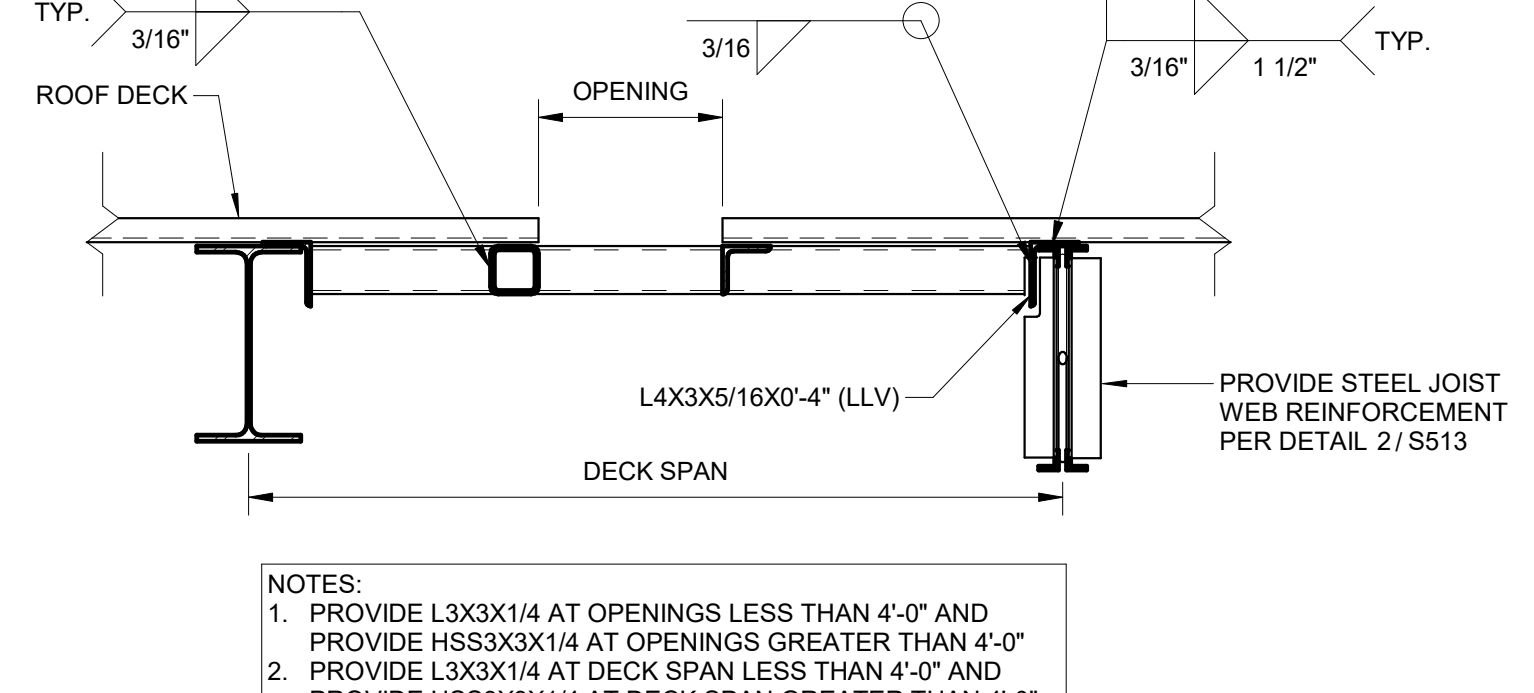
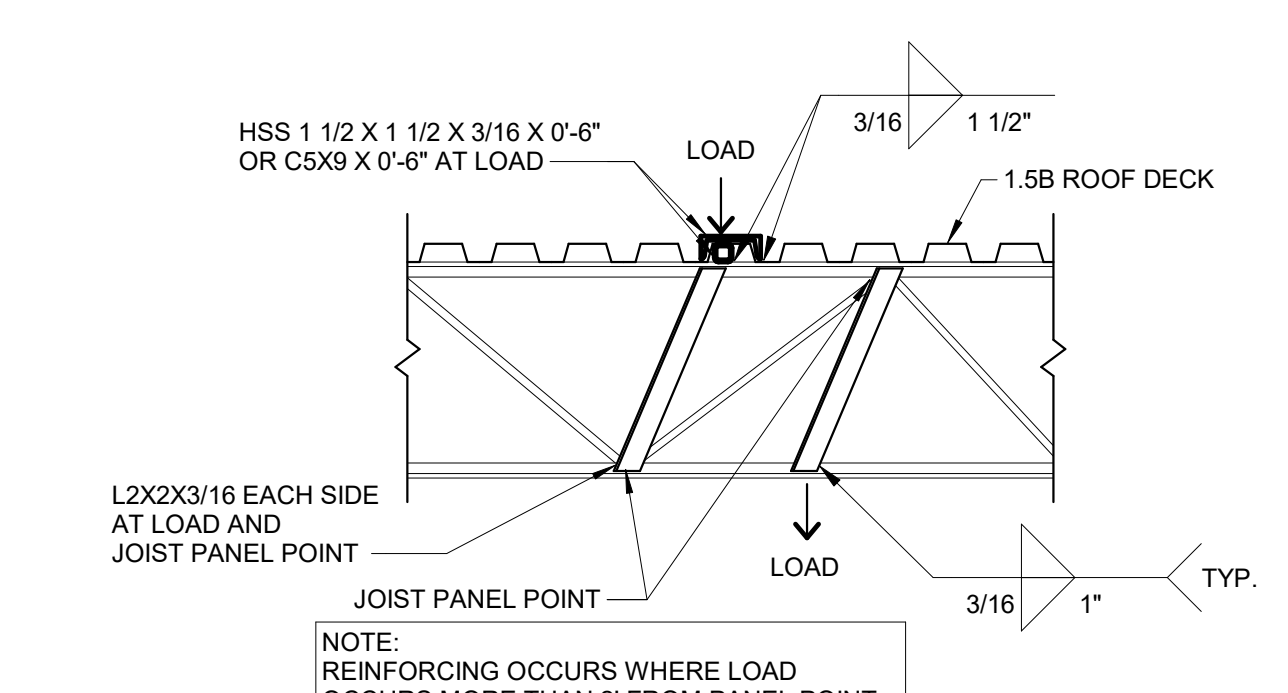
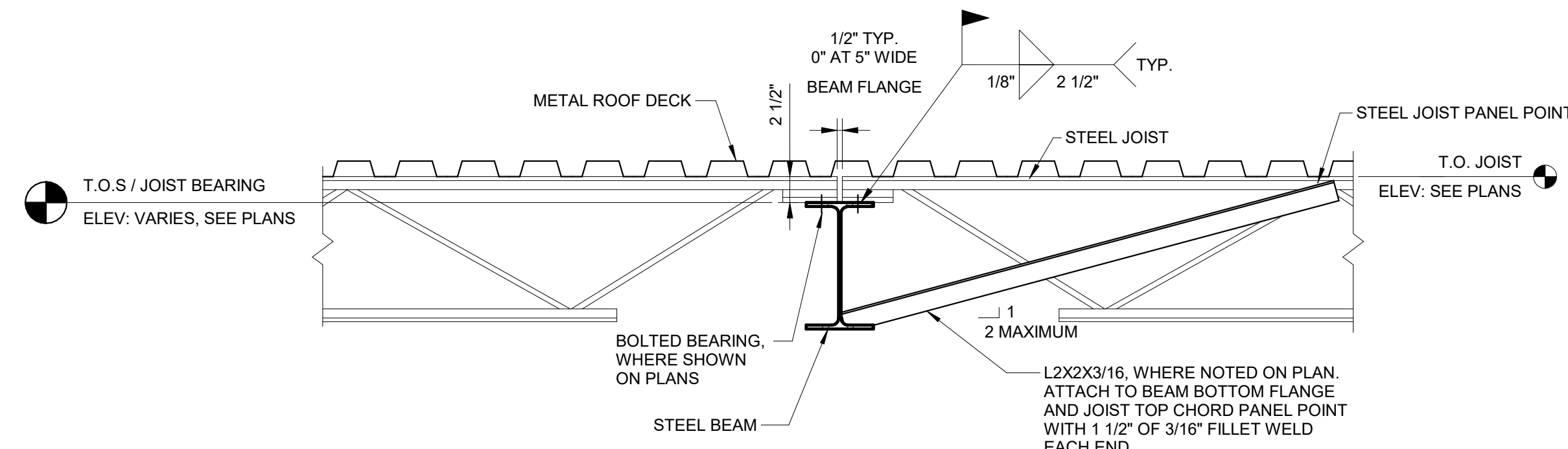
**16 JAMB DETAIL AT DETENTION COILING DOOR**  
 1 1/2" = 1'-0"



**17 SECTION**  
 1" = 1'-0"



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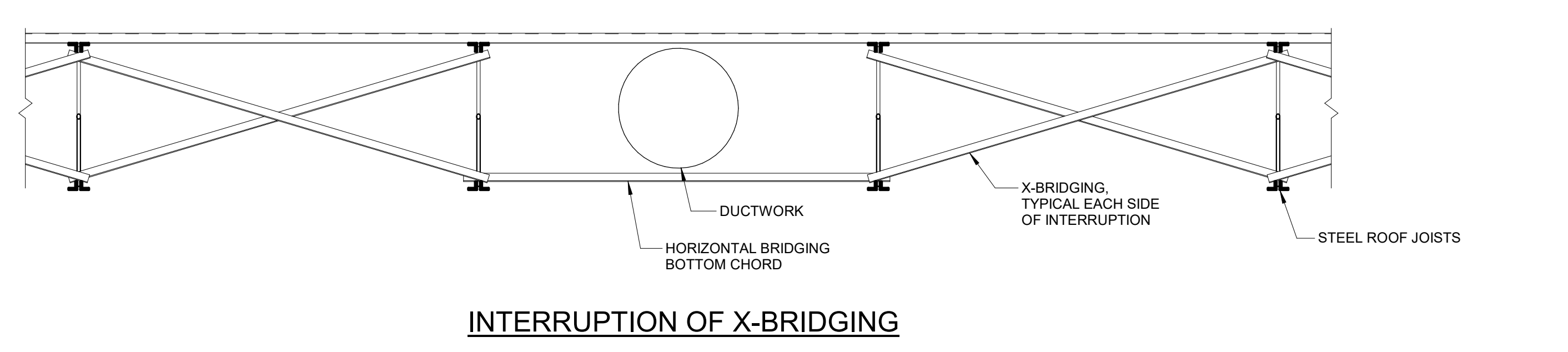
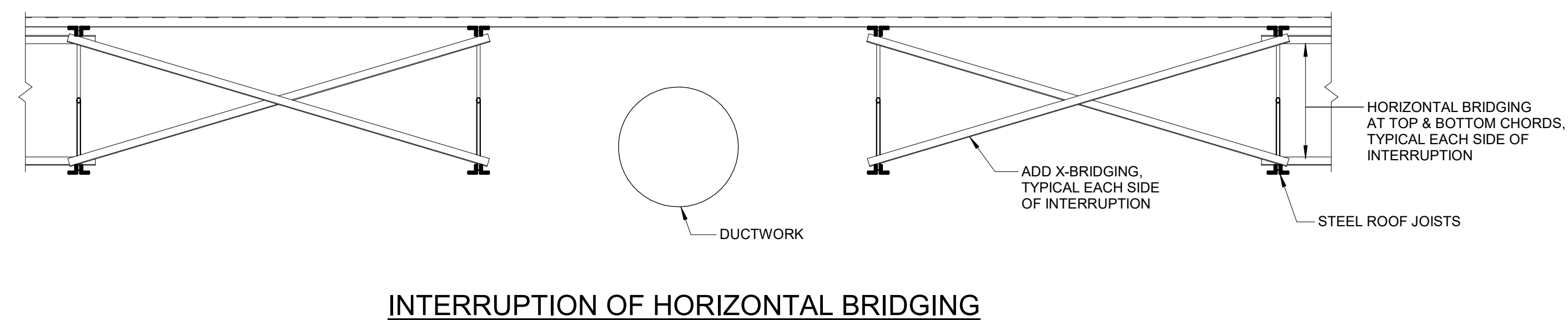


1 JOIST TO BEAM  
1" = 1'-0"

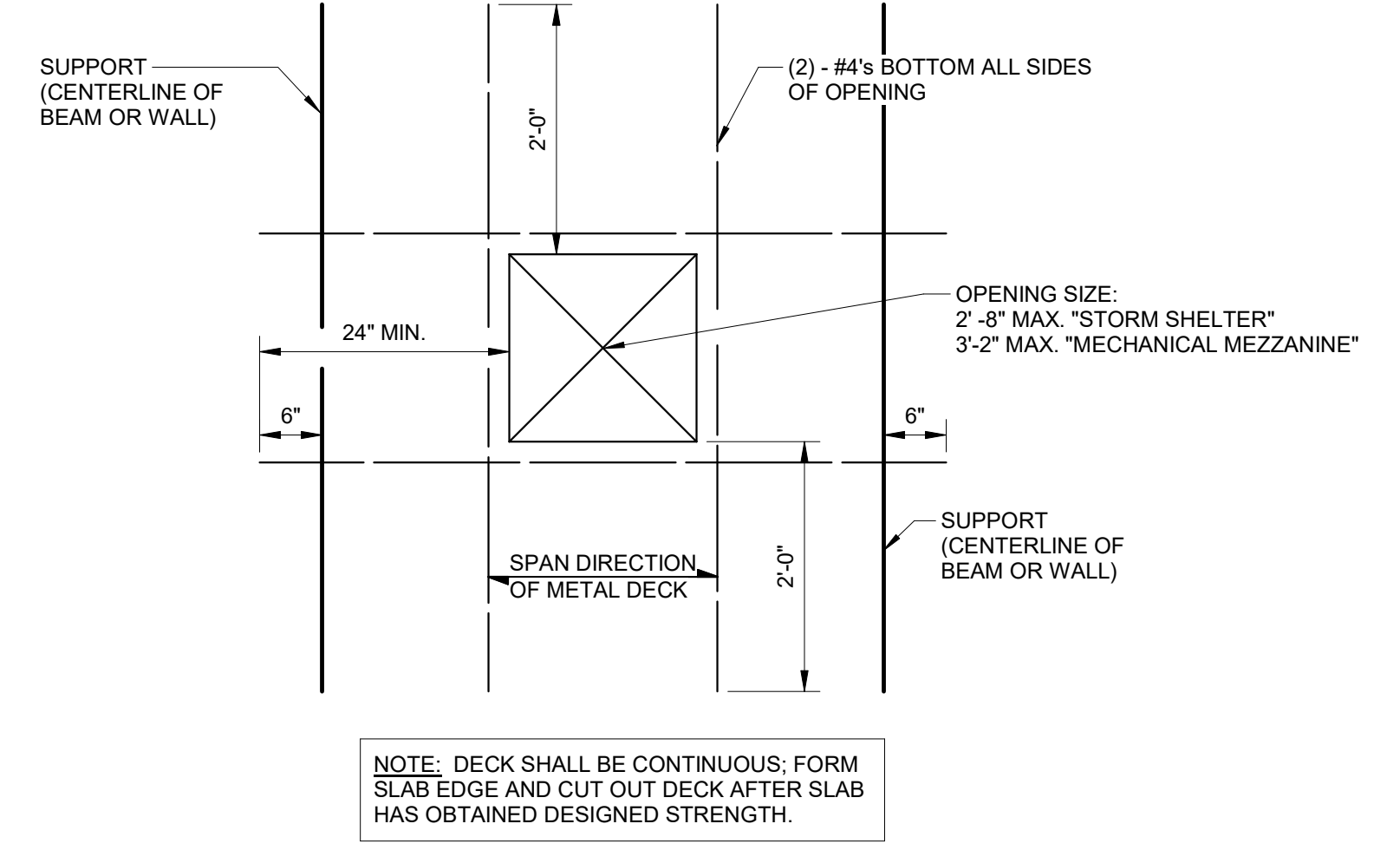
2 JOIST REINFORCING  
3/4" = 1'-0"

3 OPENINGS IN STEEL ROOF DECK  
1" = 1'-0"

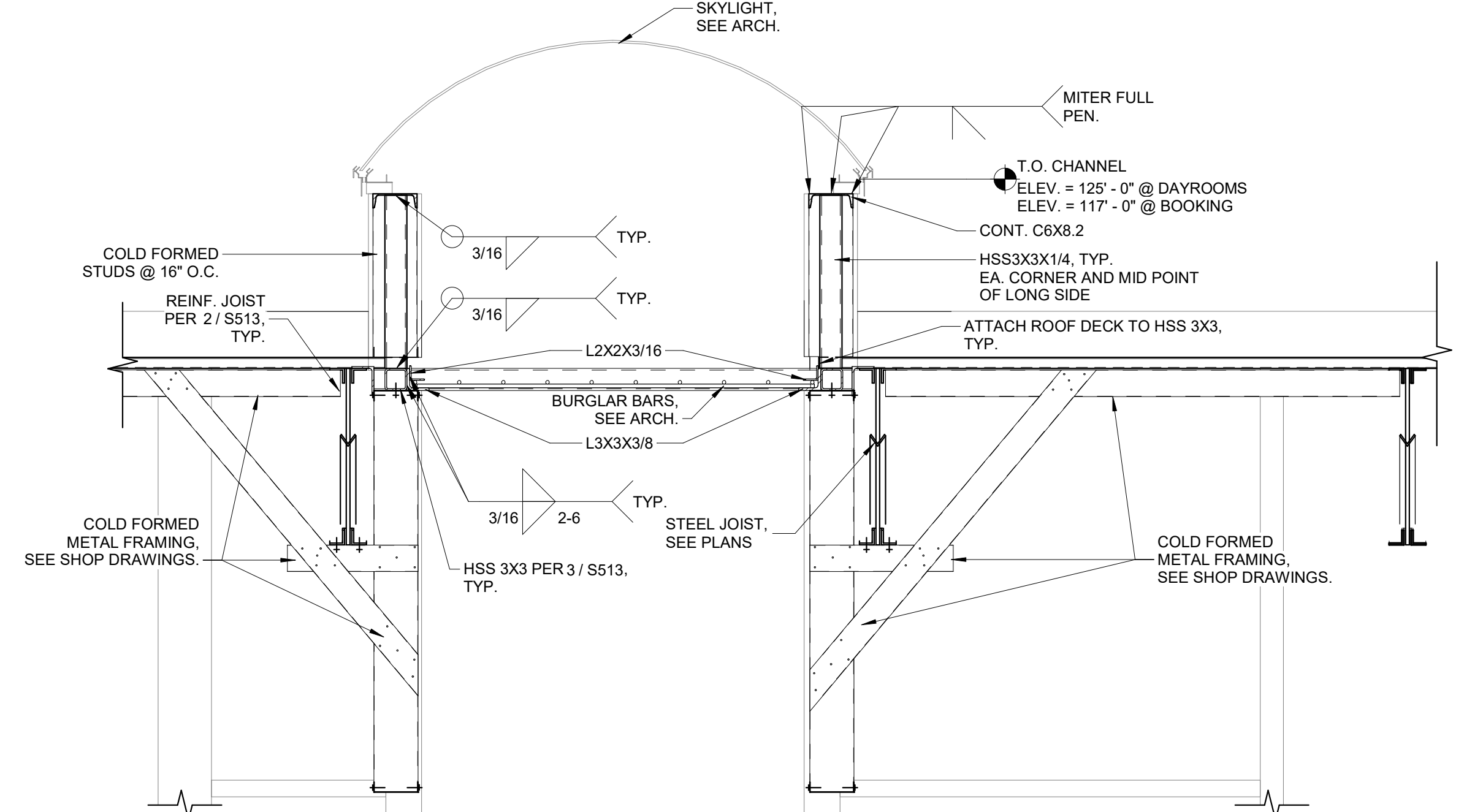
4 CMU PARTITION BRACING AT STEEL BEAM  
3/4" = 1'-0"



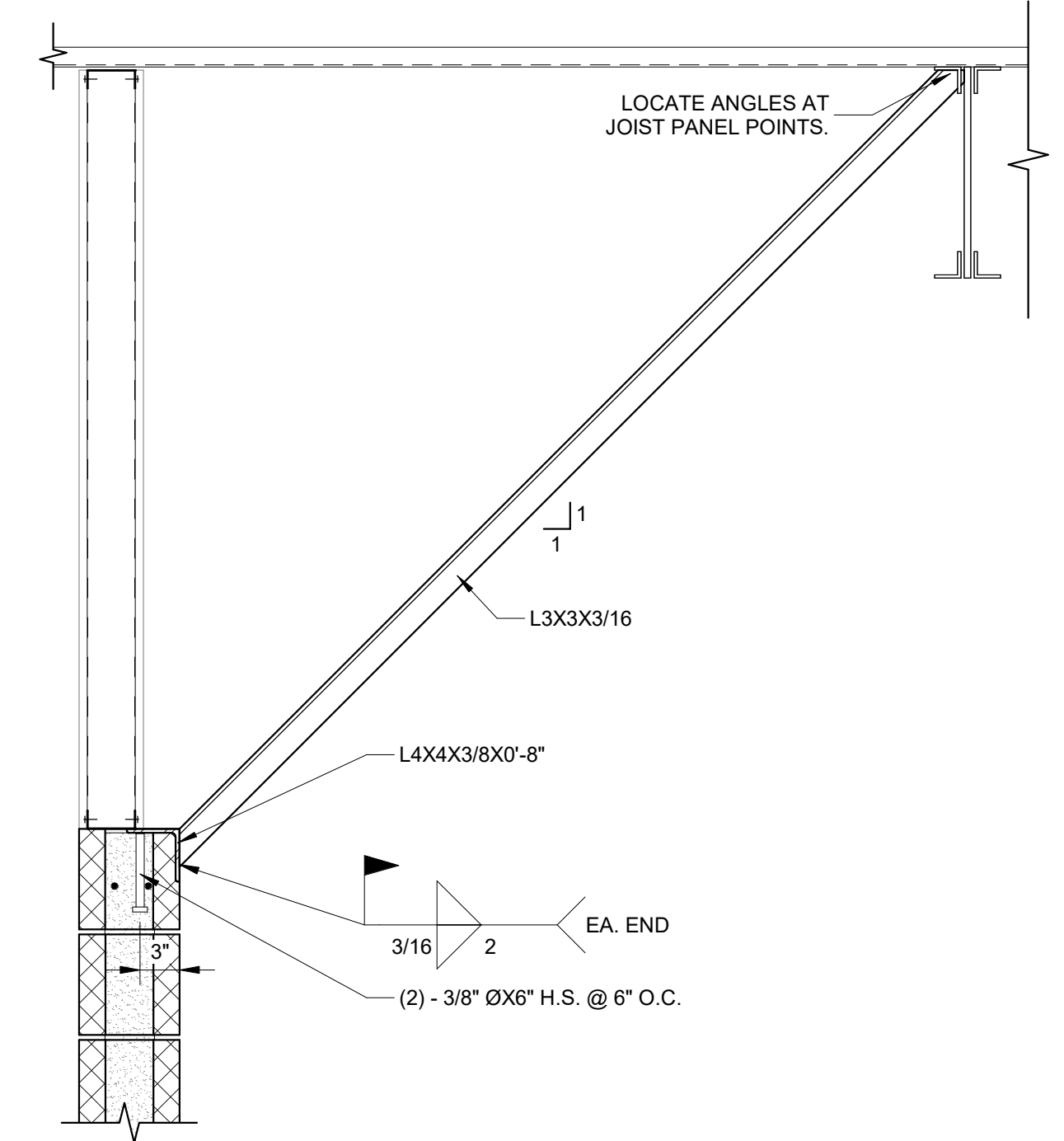
5 BRIDGING INTERRUPTION DETAILS  
3/4" = 1'-0"



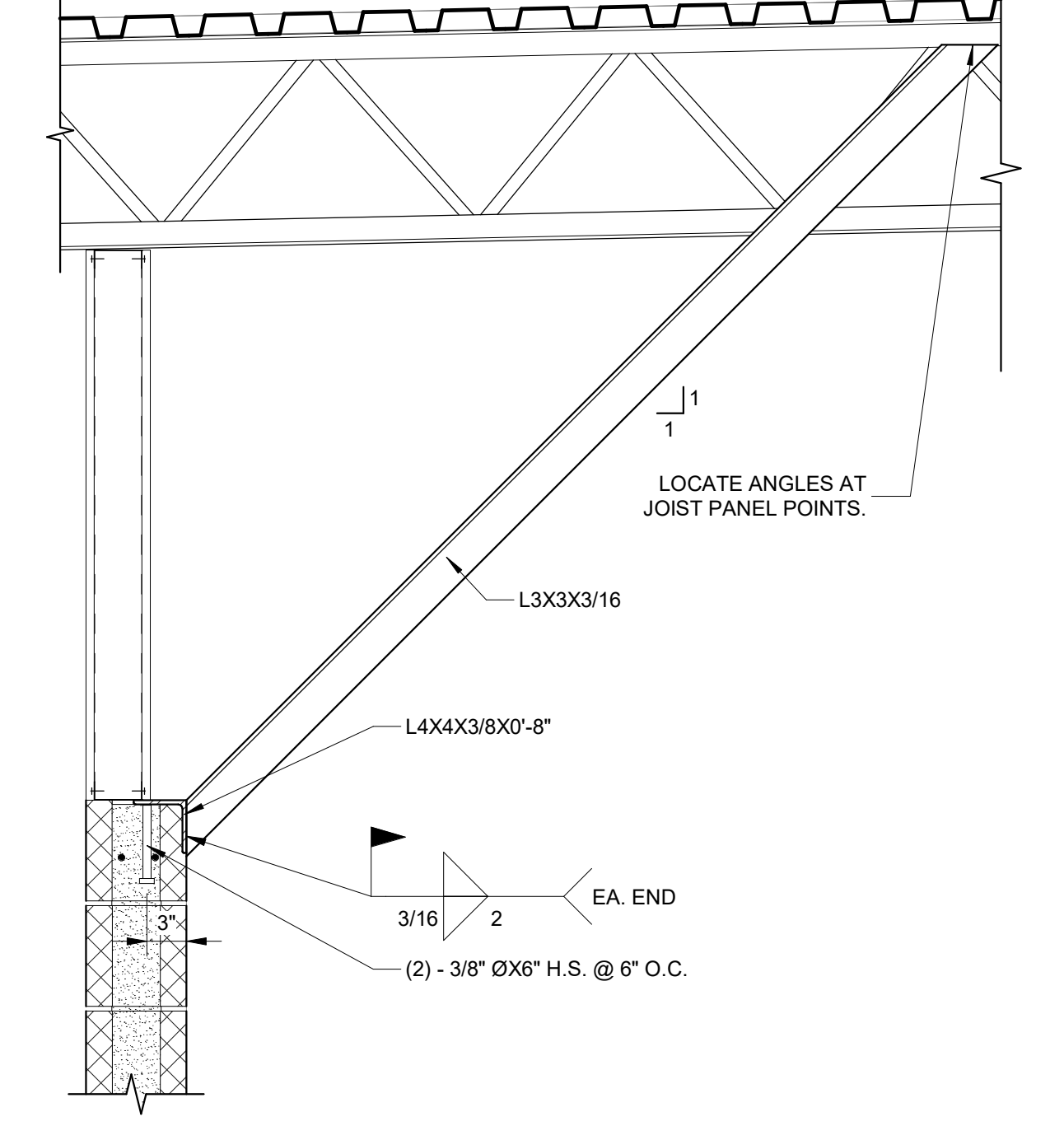
6 SLAB OPENING REINFORCING  
3/4" = 1'-0"



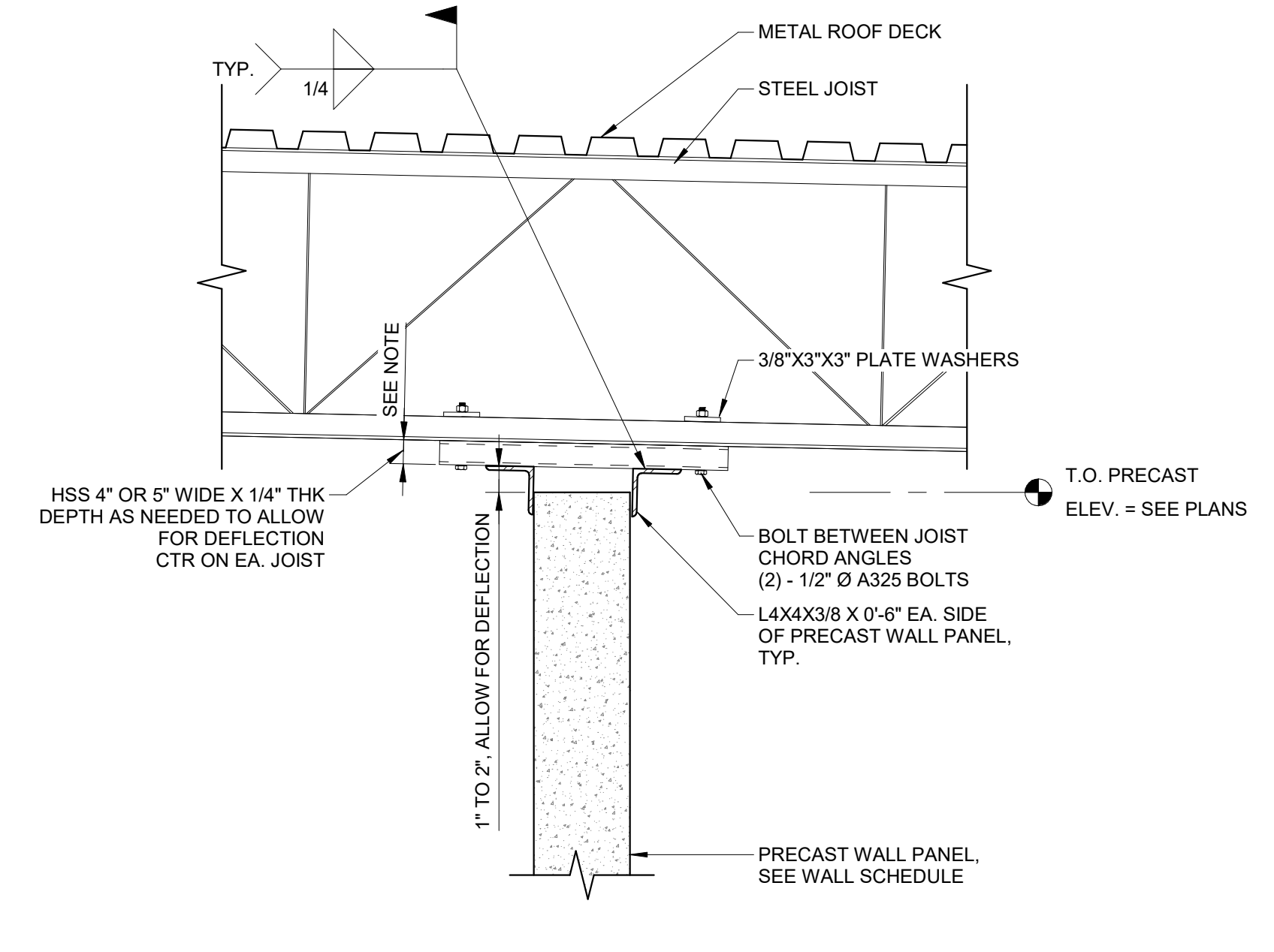
7 SKYLIGHT FRAMING  
3/4" = 1'-0"



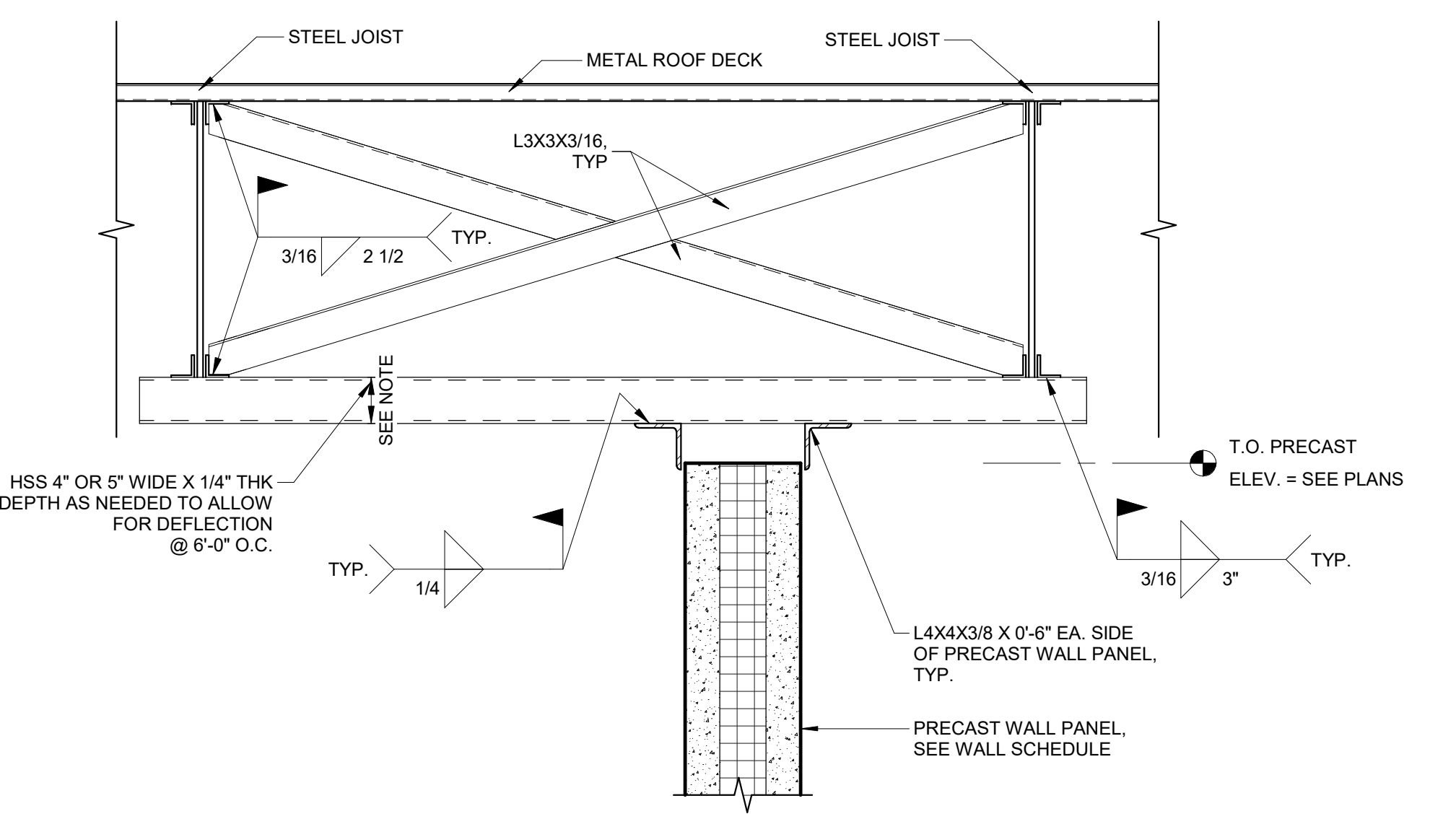
8 BRACING @ CMU WALLS  
1" = 1'-0"



9 BRACING @ CMU WALLS BRG.  
1" = 1'-0"



10 PRECAST BRACING DETAIL  
1" = 1'-0"



11 PRECAST BRACING DETAIL  
1" = 1'-0"

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1		06/26/2018	

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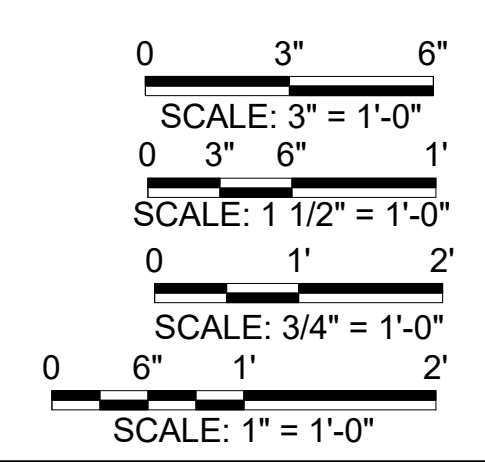
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STRUCTURAL FRAMING DETAILS

PROJECT NO. 22-4048  
DRAWING ISSUED DATE: 03/01/2024  
SHEET

S513





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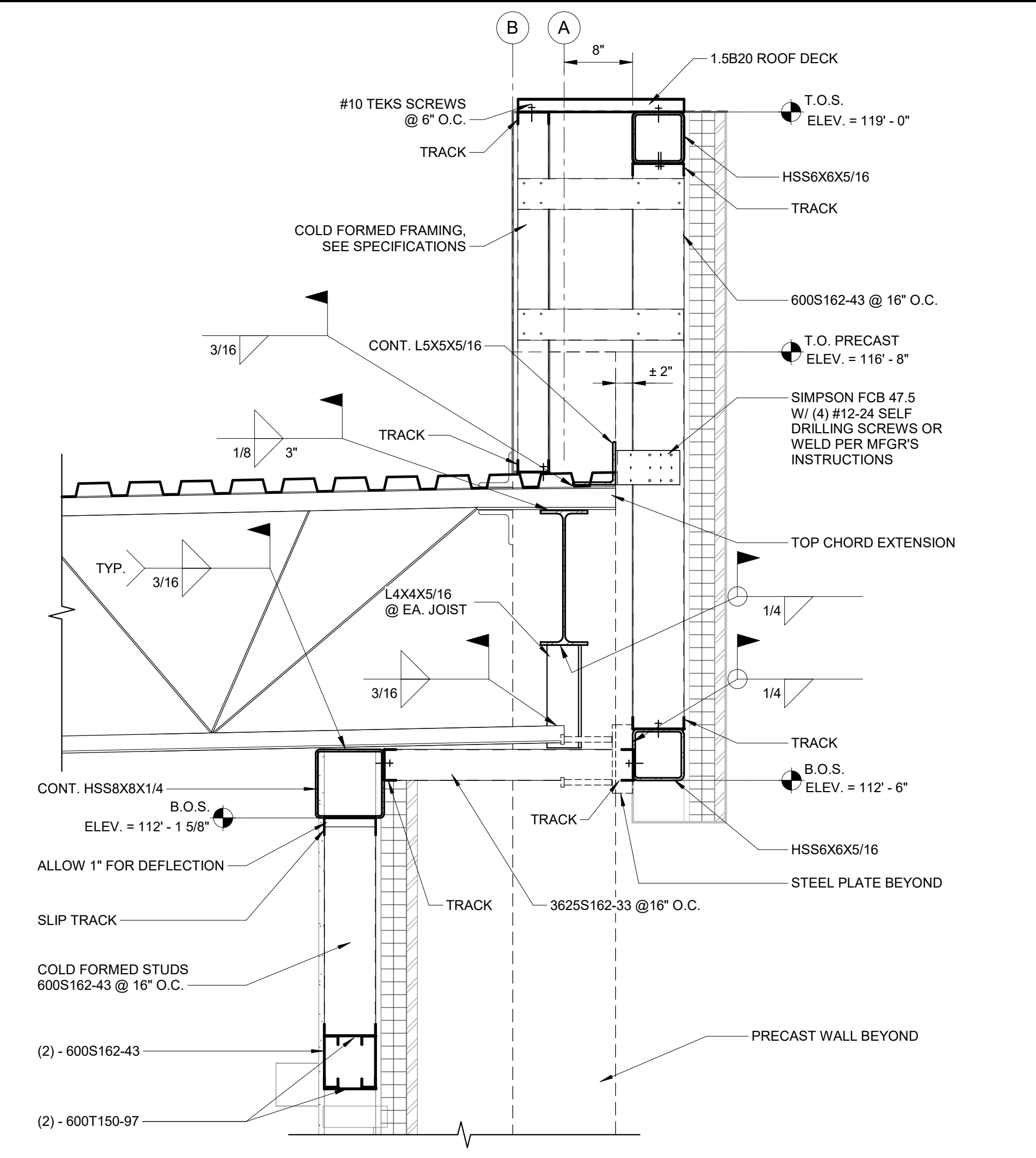
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CHECKED	ADL	CHECK DATE	03/01/2024
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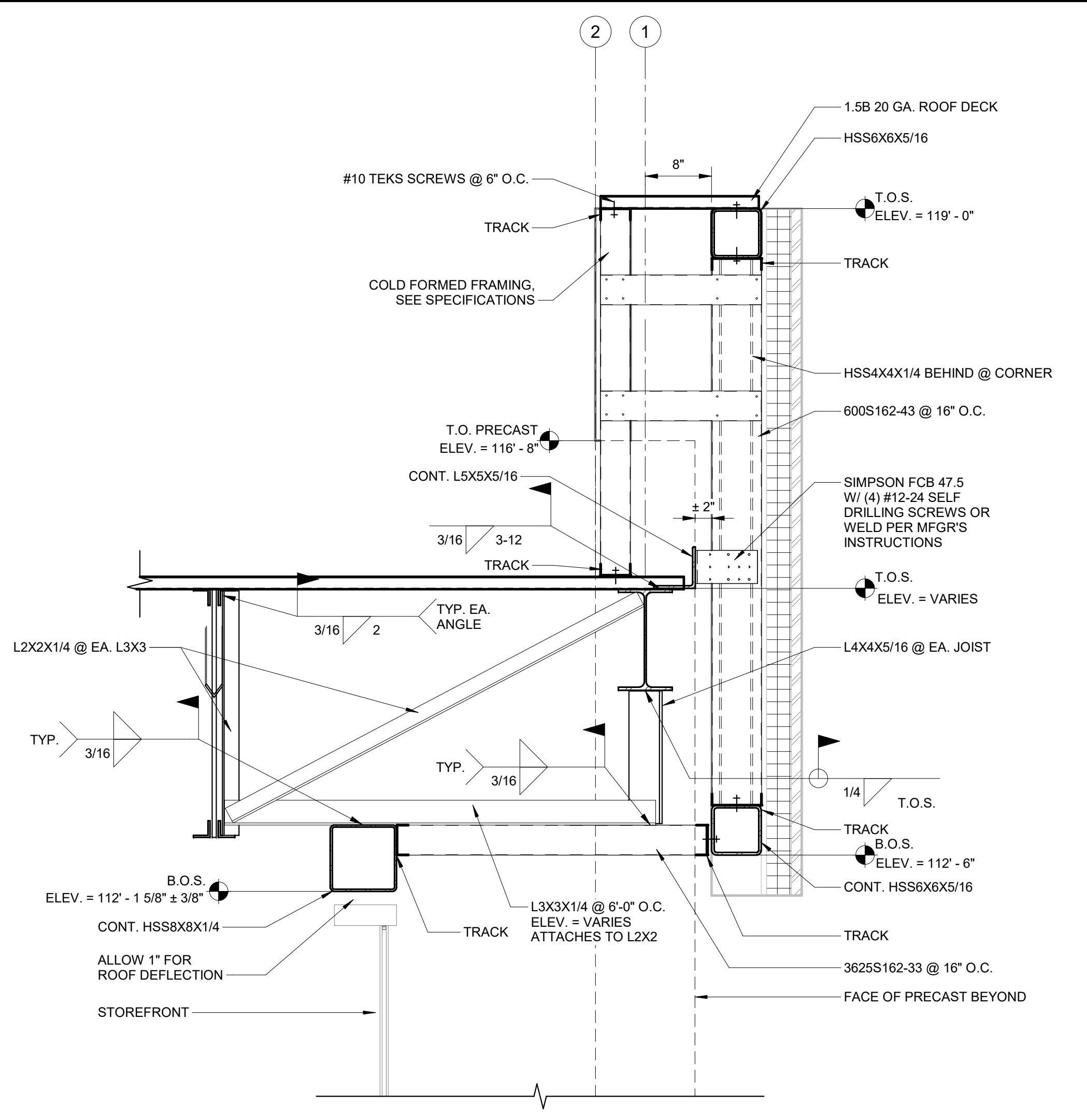
**STRUCTURAL FRAMING DETAILS**

PROJECT NO. 22-4048  
 DRAWING ISSUED DATE: 03/01/2024

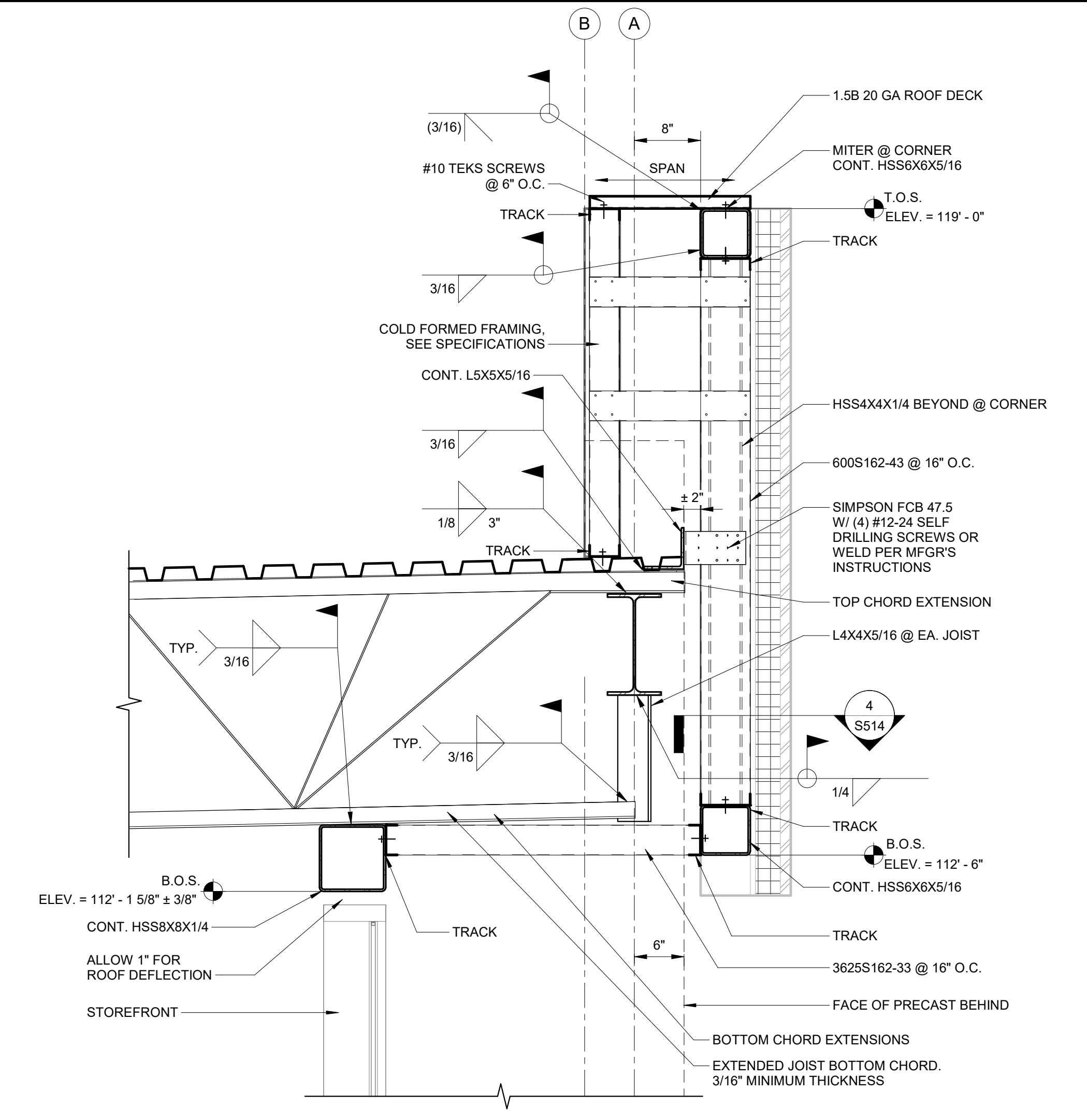
SHEET **S514**



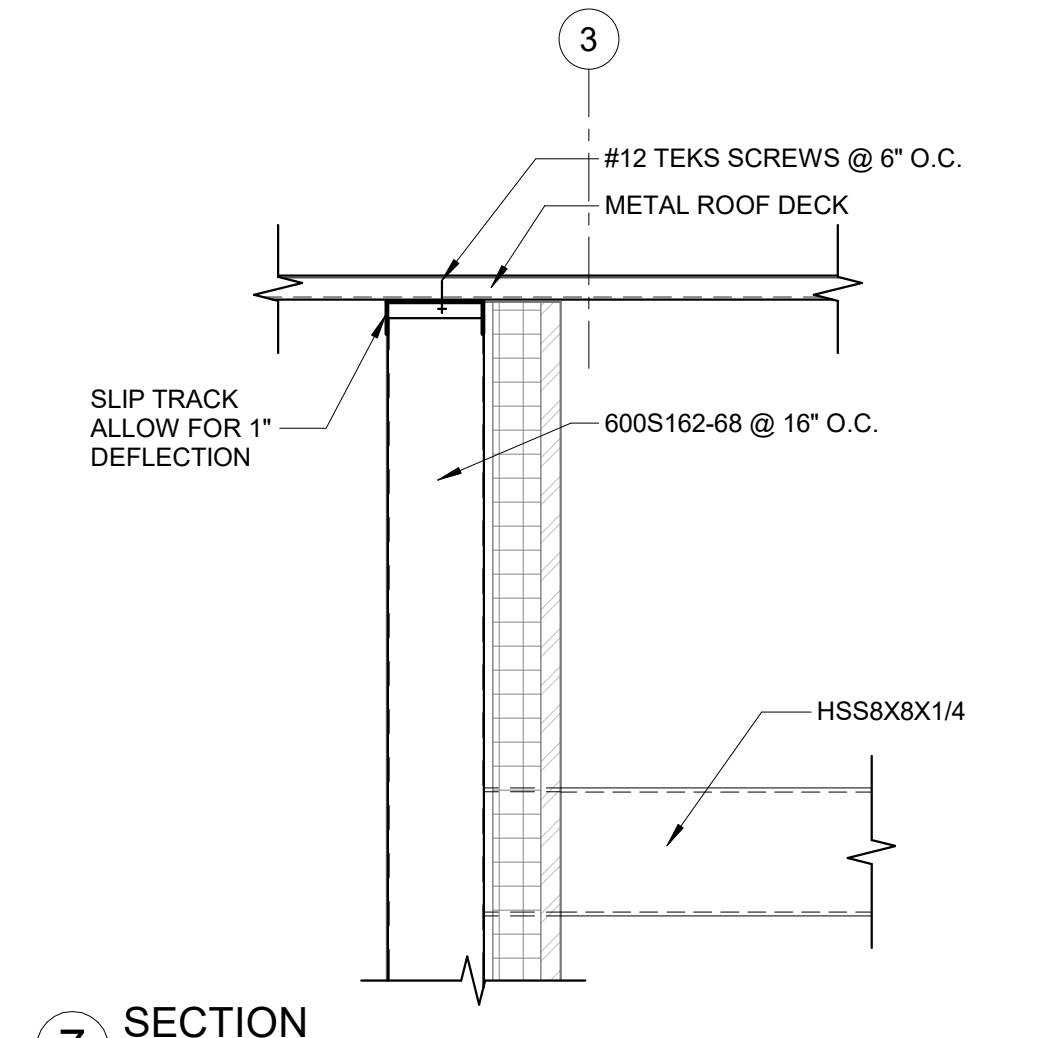
**3 SECTION**  
1" = 1'-0"



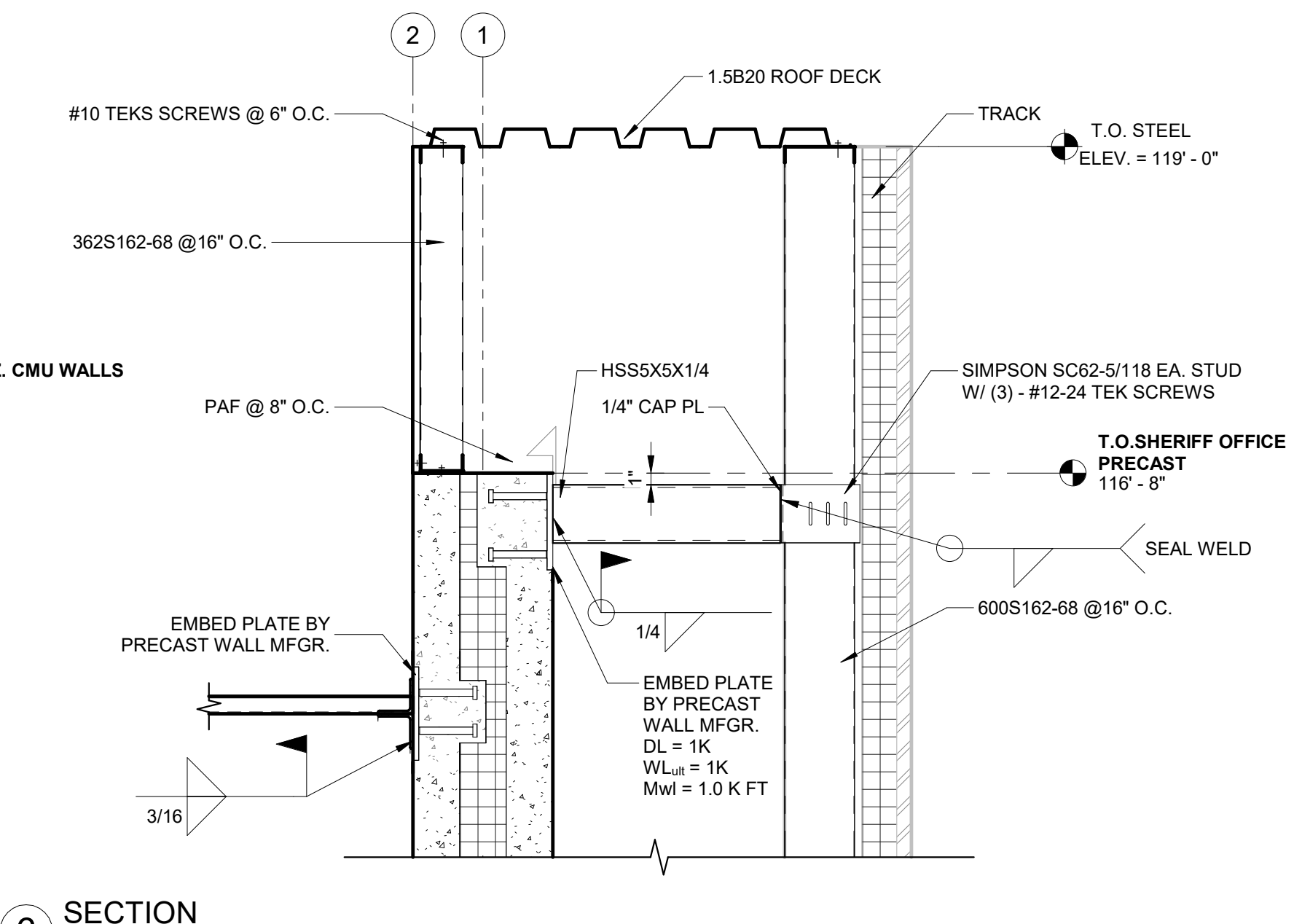
**2 SECTION**  
1" = 1'-0"



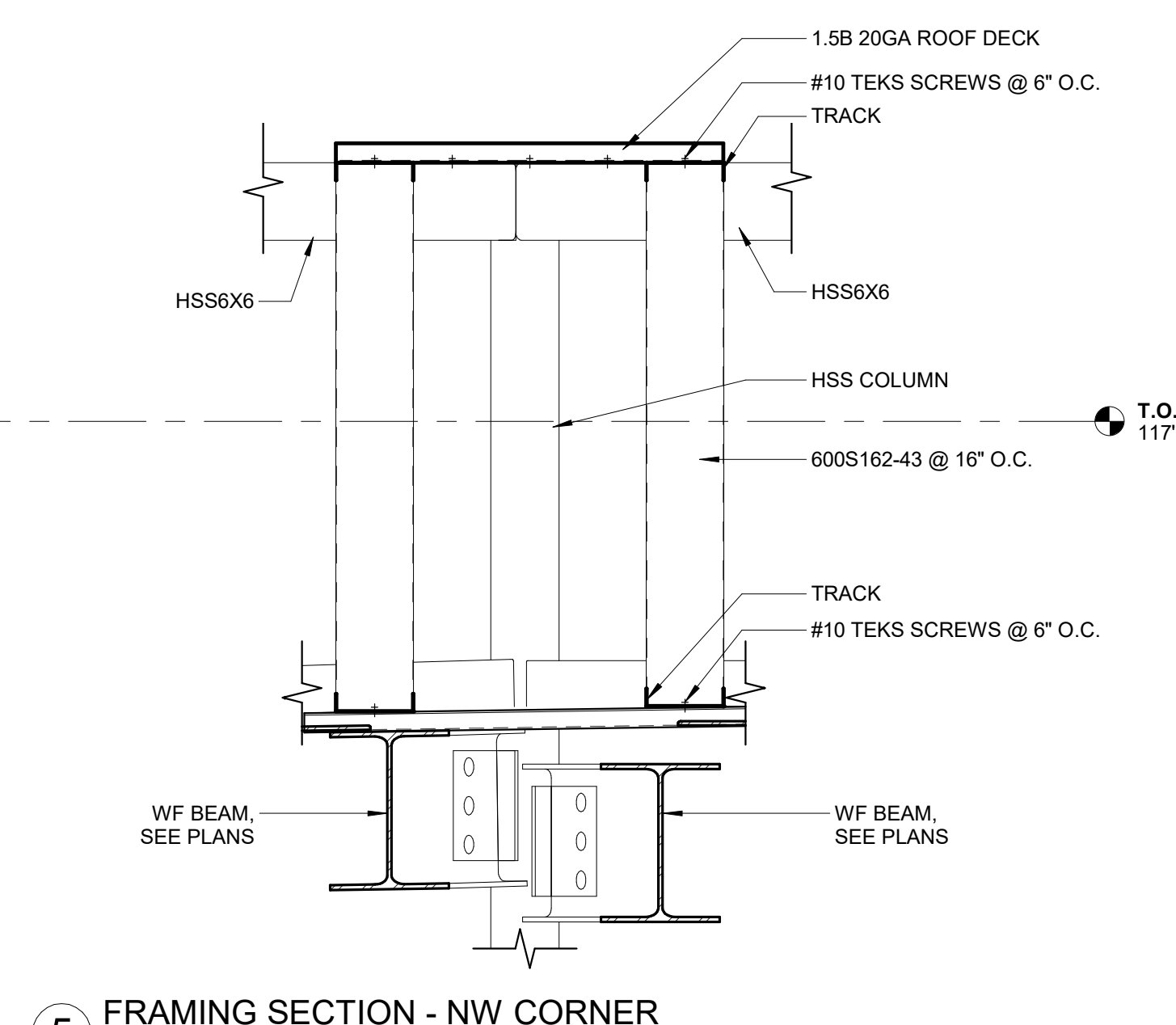
**1 SECTION**  
1" = 1'-0"



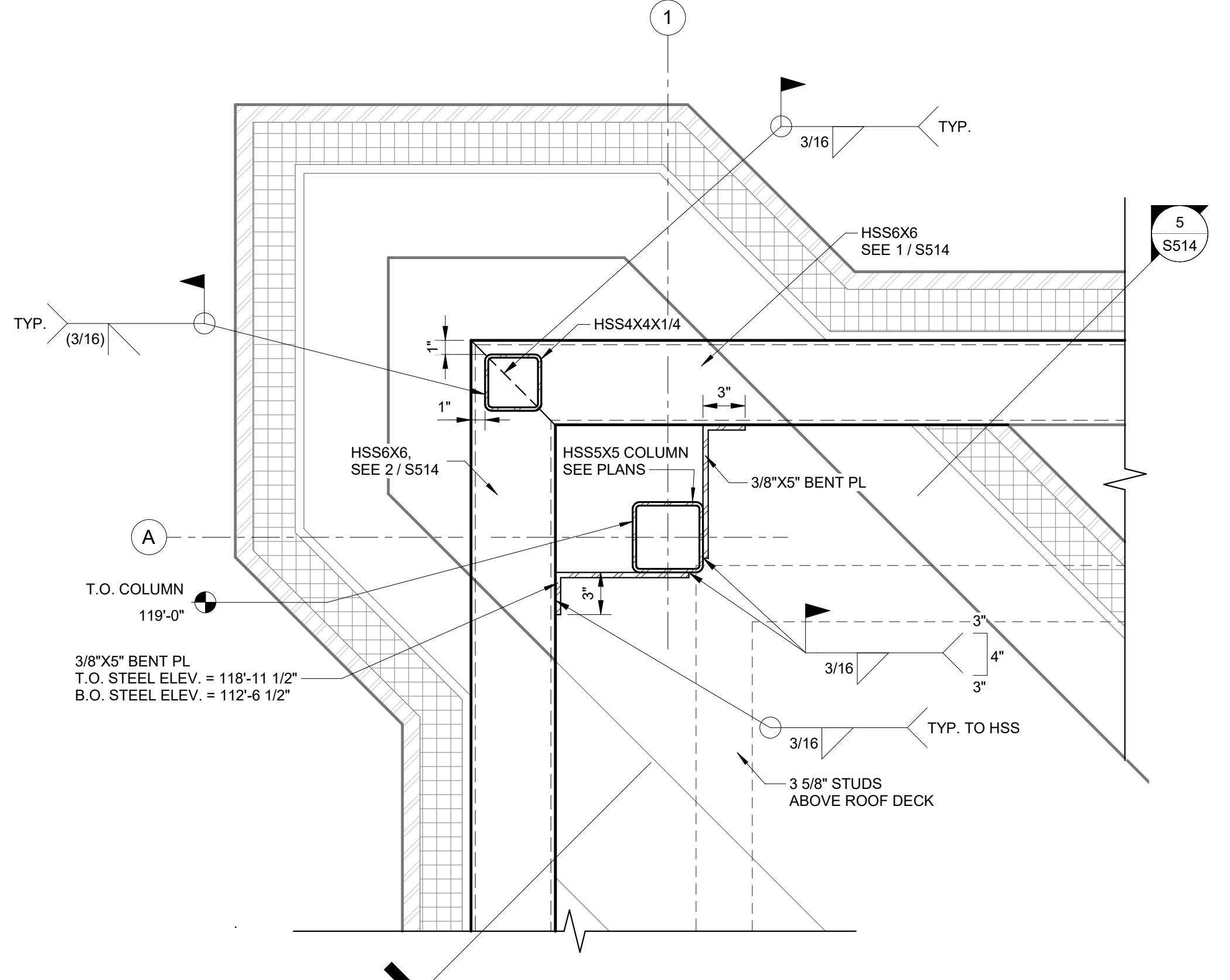
**7 SECTION**  
1" = 1'-0"



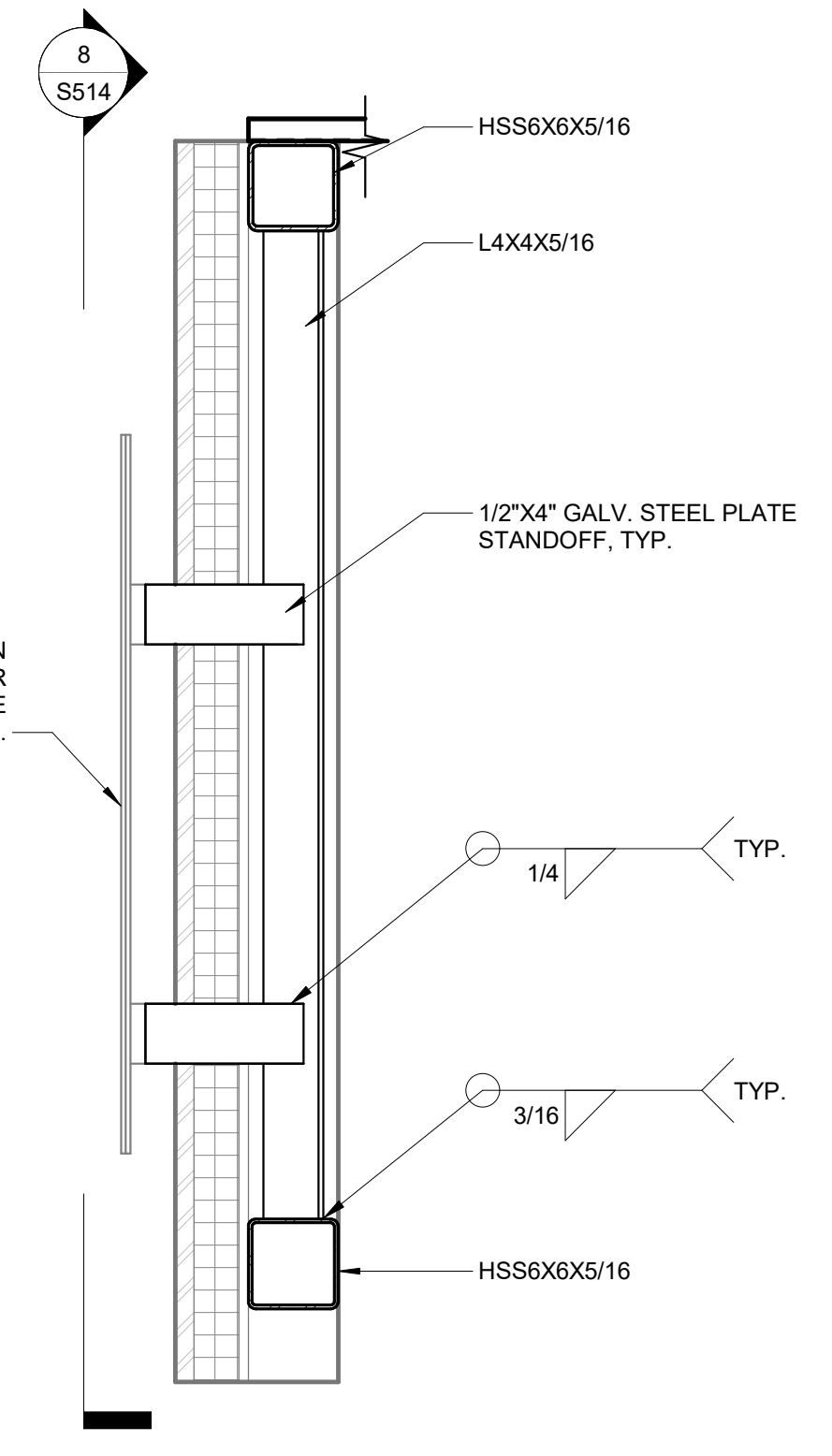
**6 SECTION**  
1" = 1'-0"



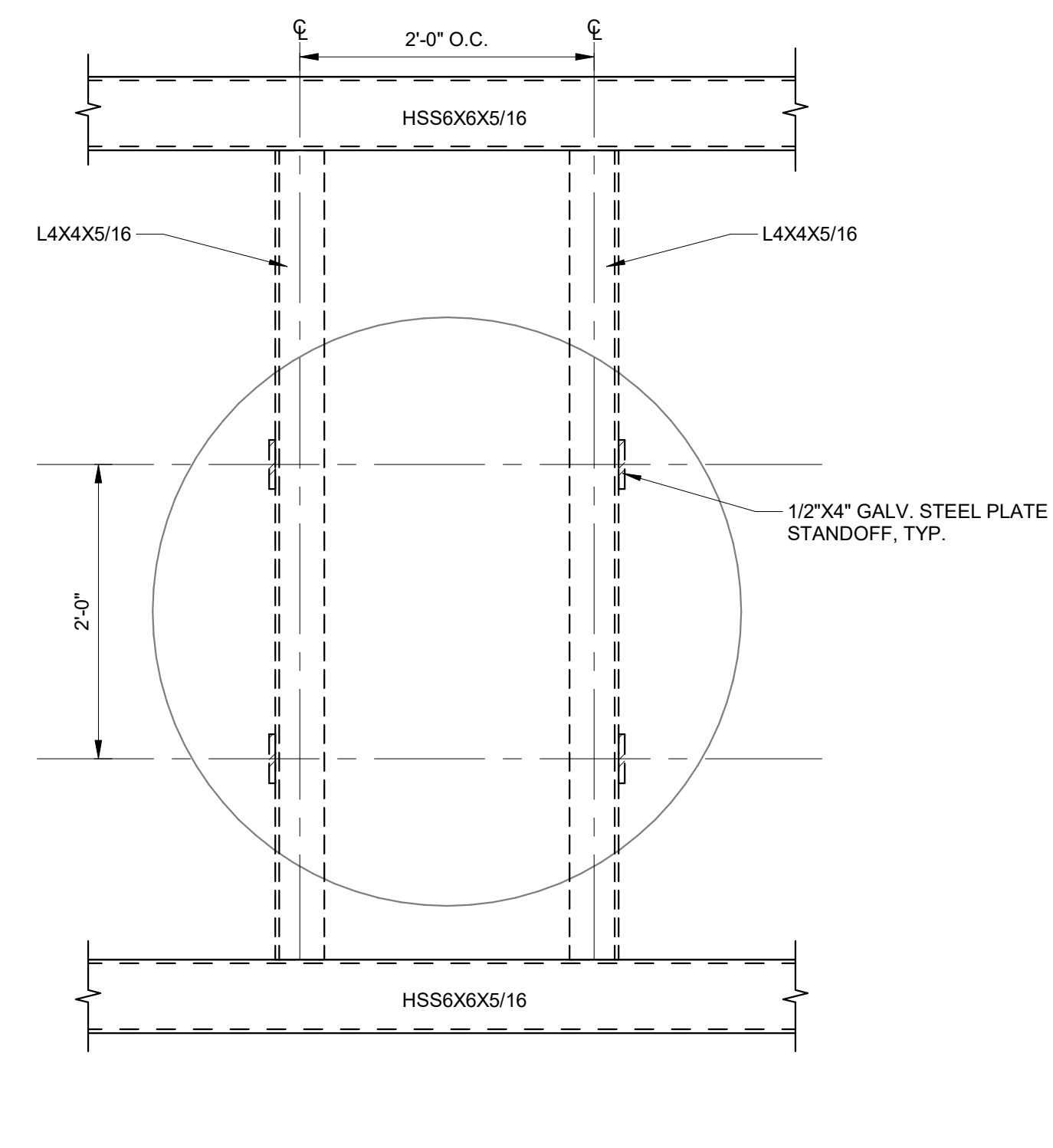
**5 FRAMING SECTION - NW CORNER**  
1" = 1'-0"



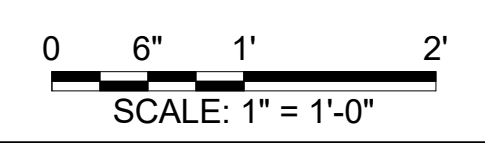
**4 HSS CONNECTION PLAN DETAIL**  
1" = 1'-0"

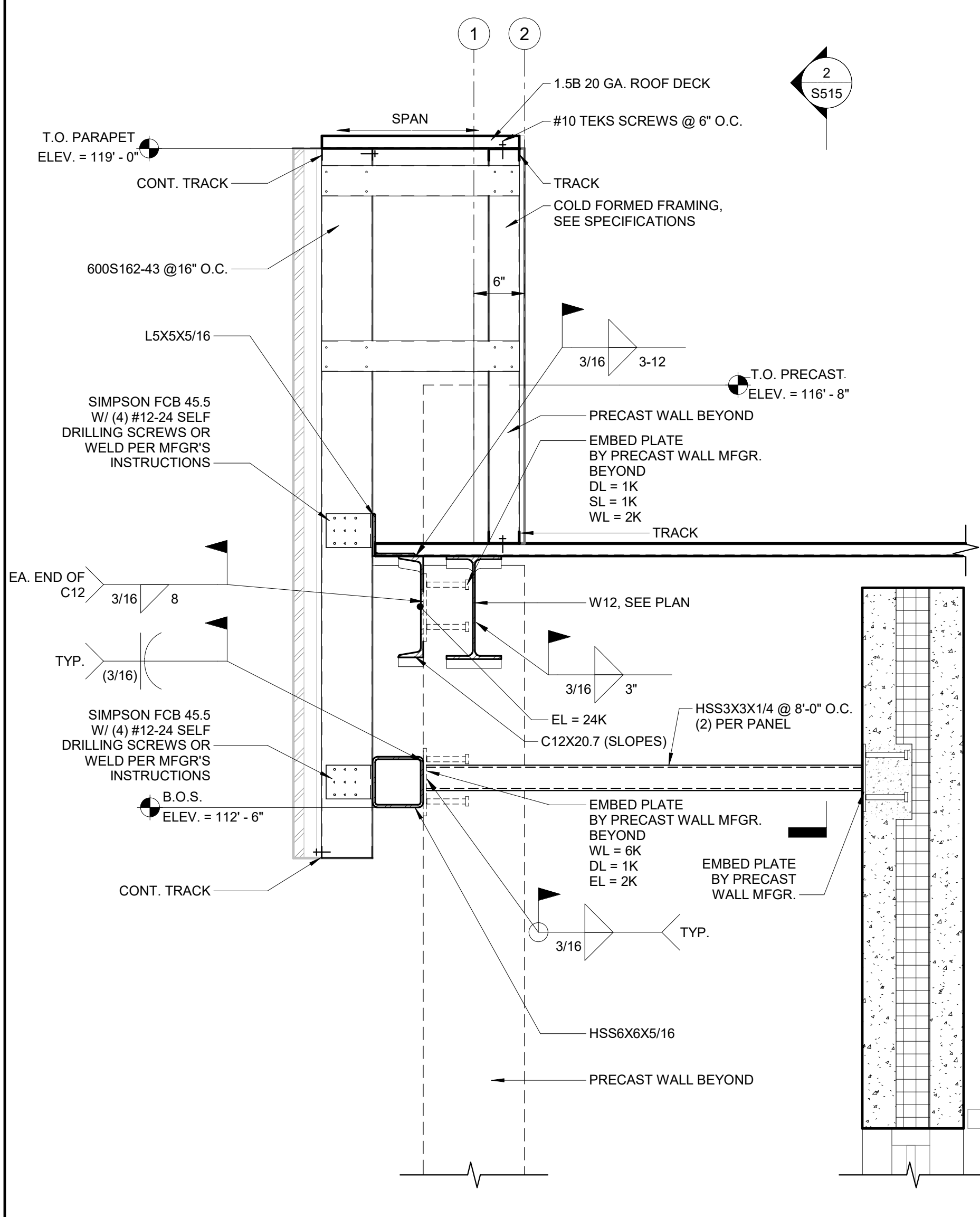


**9 MEDALLION SECTION**  
1" = 1'-0"

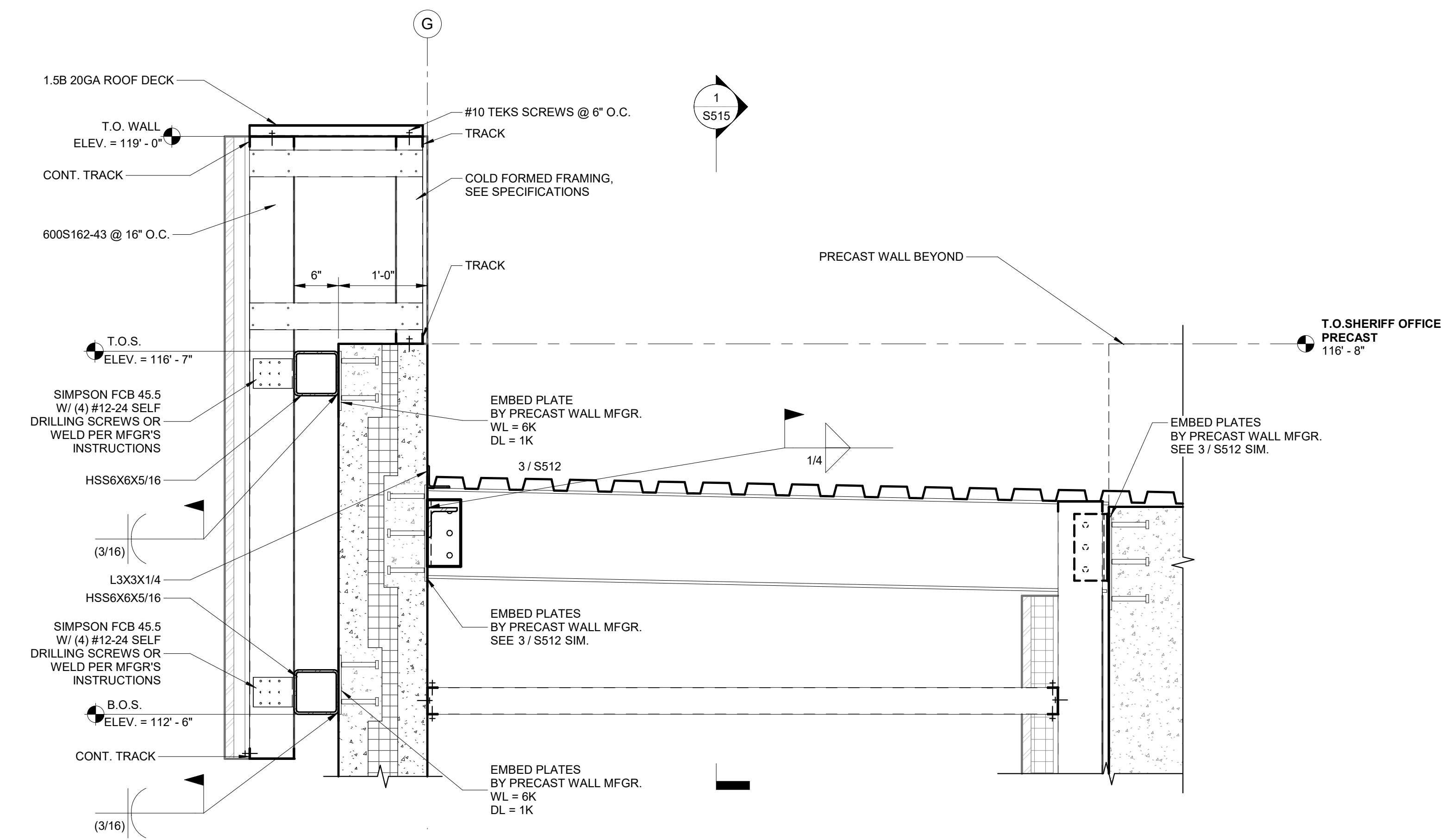


**8 MEDALLION ELEVATION**  
1" = 1'-0"

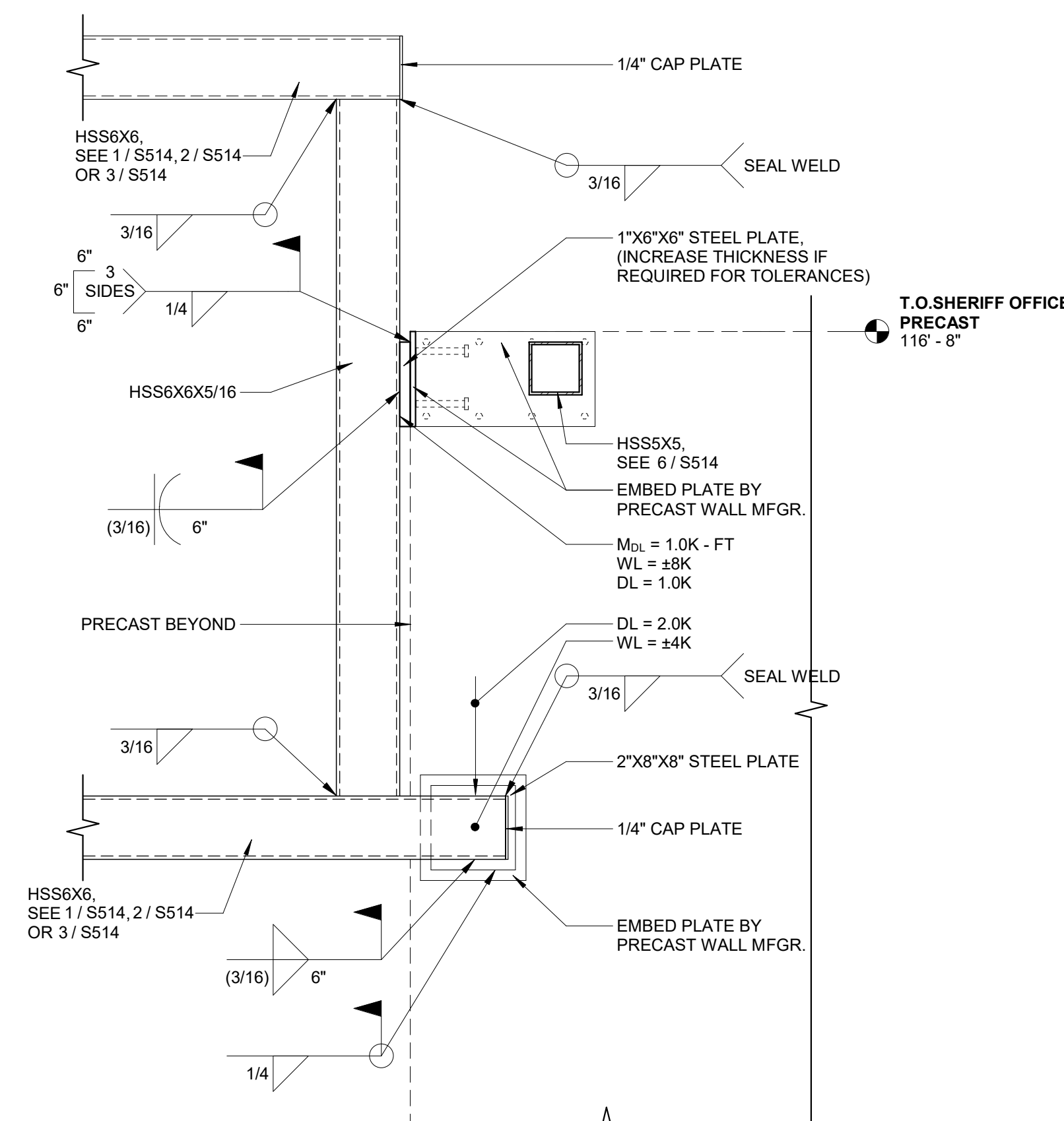




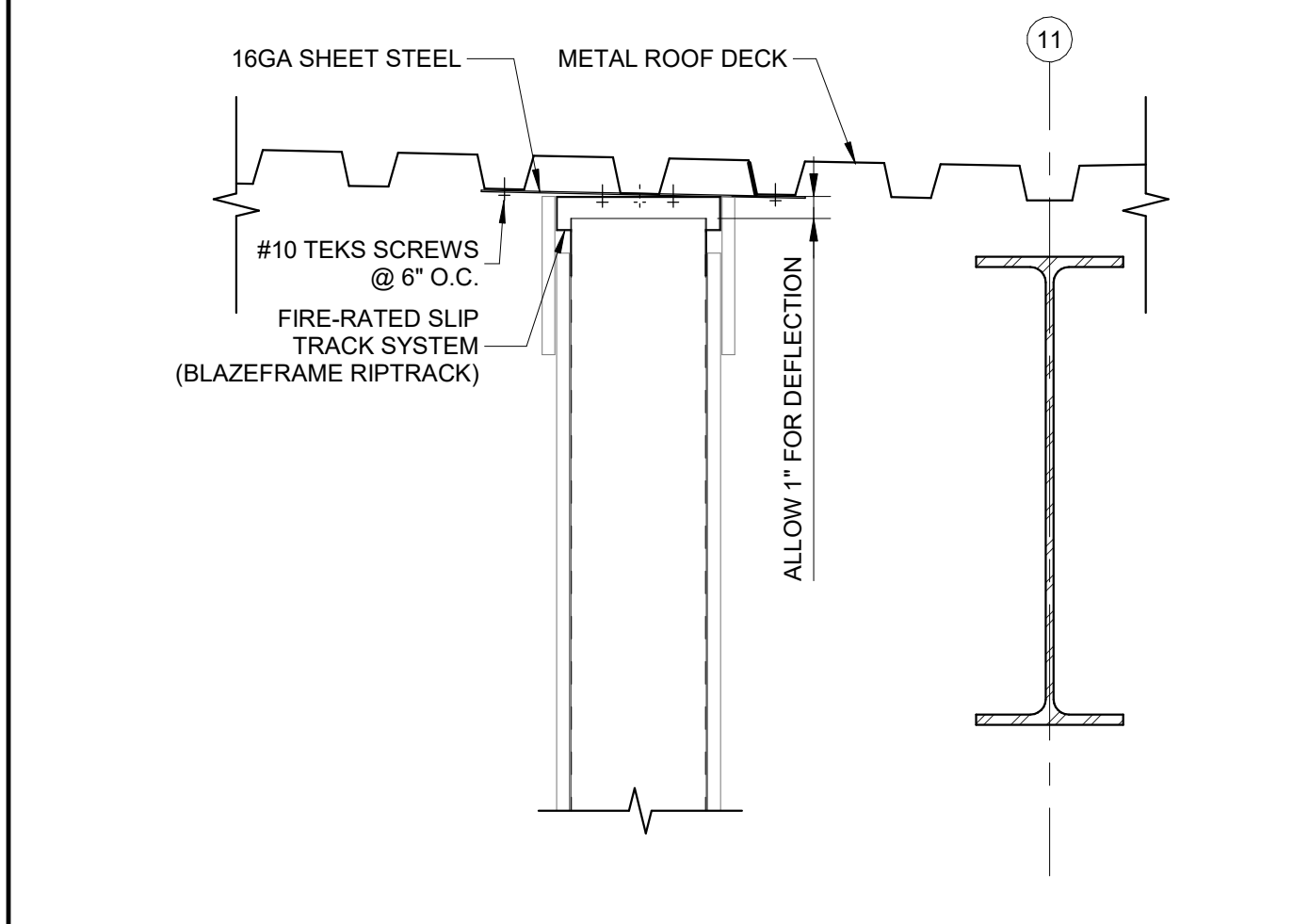
**1 SECTION**  
1" = 1'-0"



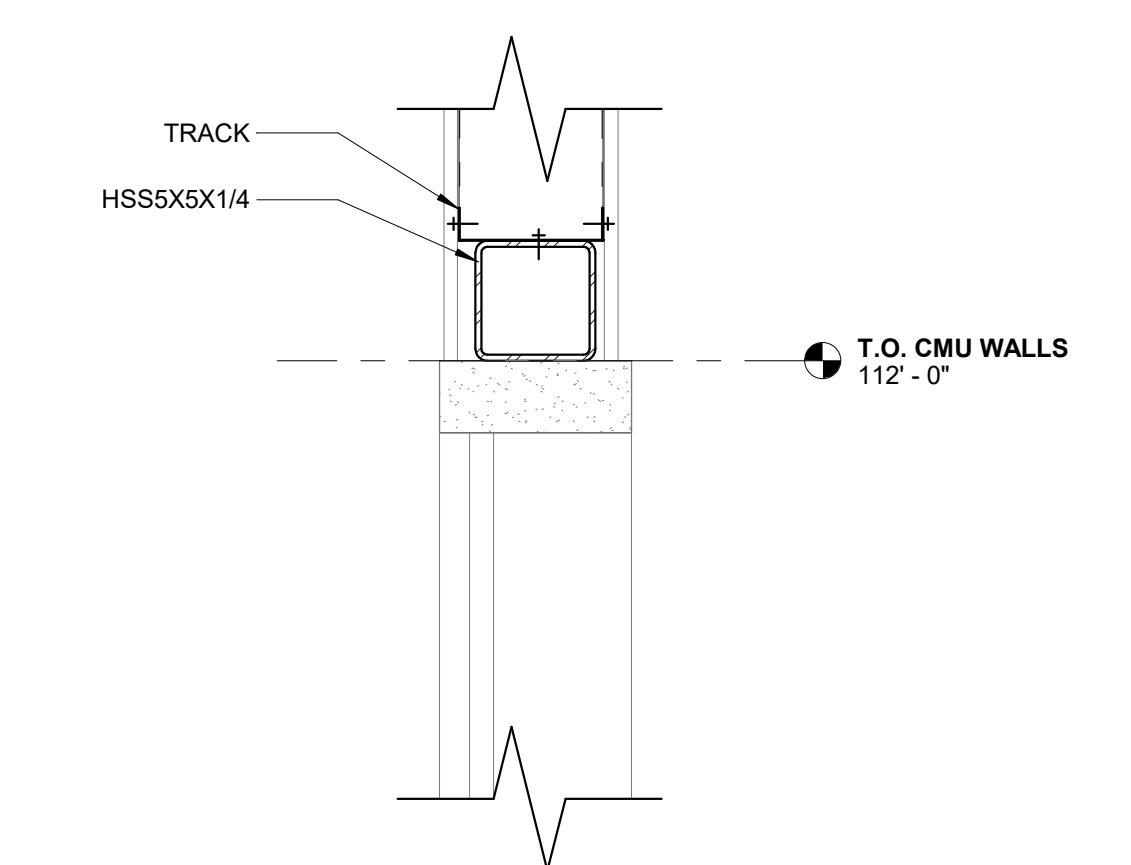
**2 SECTION**  
1" = 1'-0"



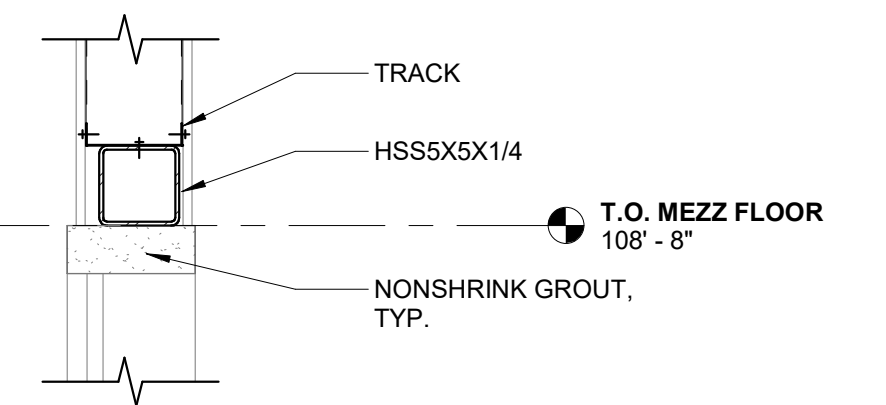
**3 WALL FRAMING CONNECTION TO PRECAST**  
1" = 1'-0"



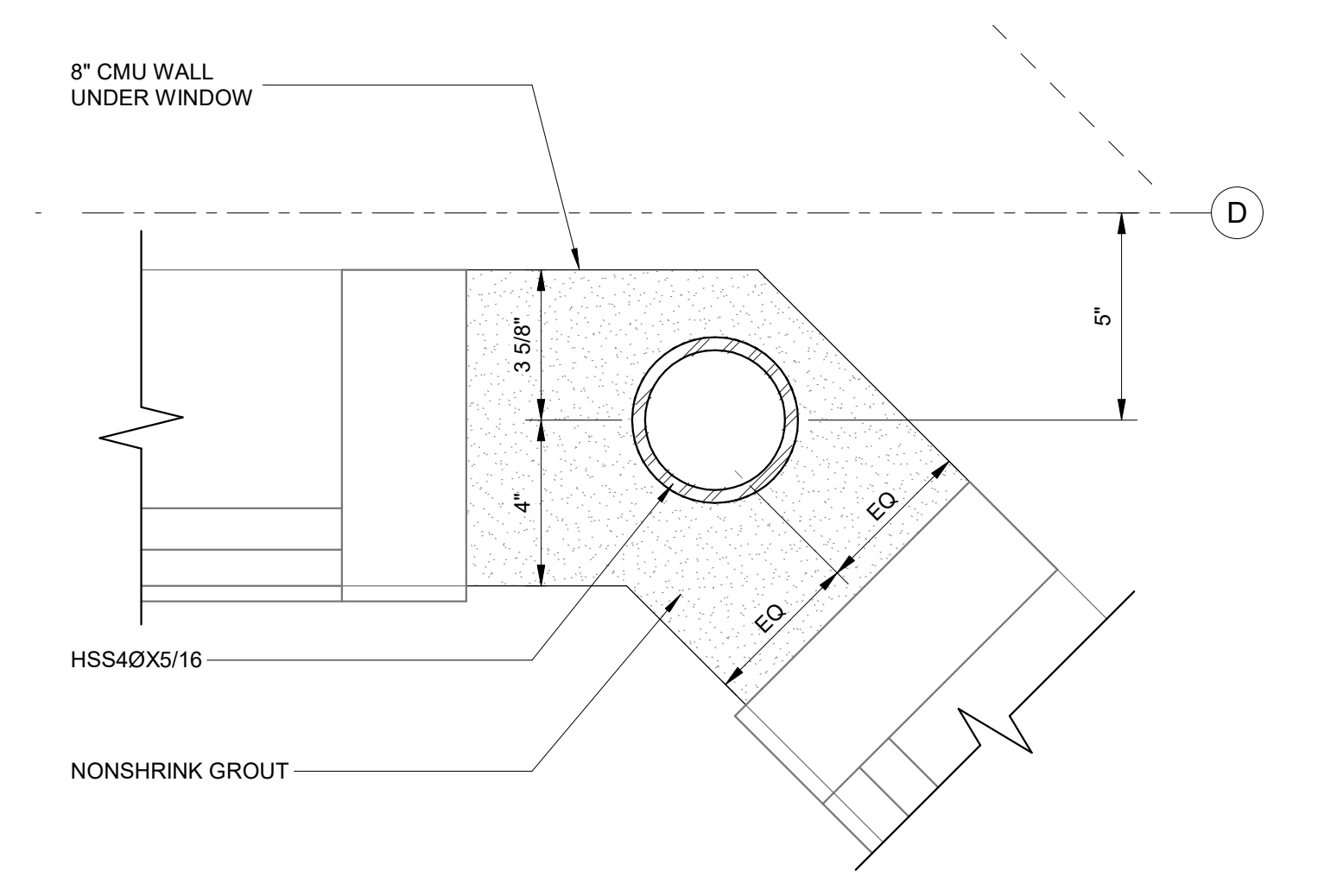
**4 DAYROOM SECURITY - DECK CONNECTION**  
1 1/2" = 1'-0"



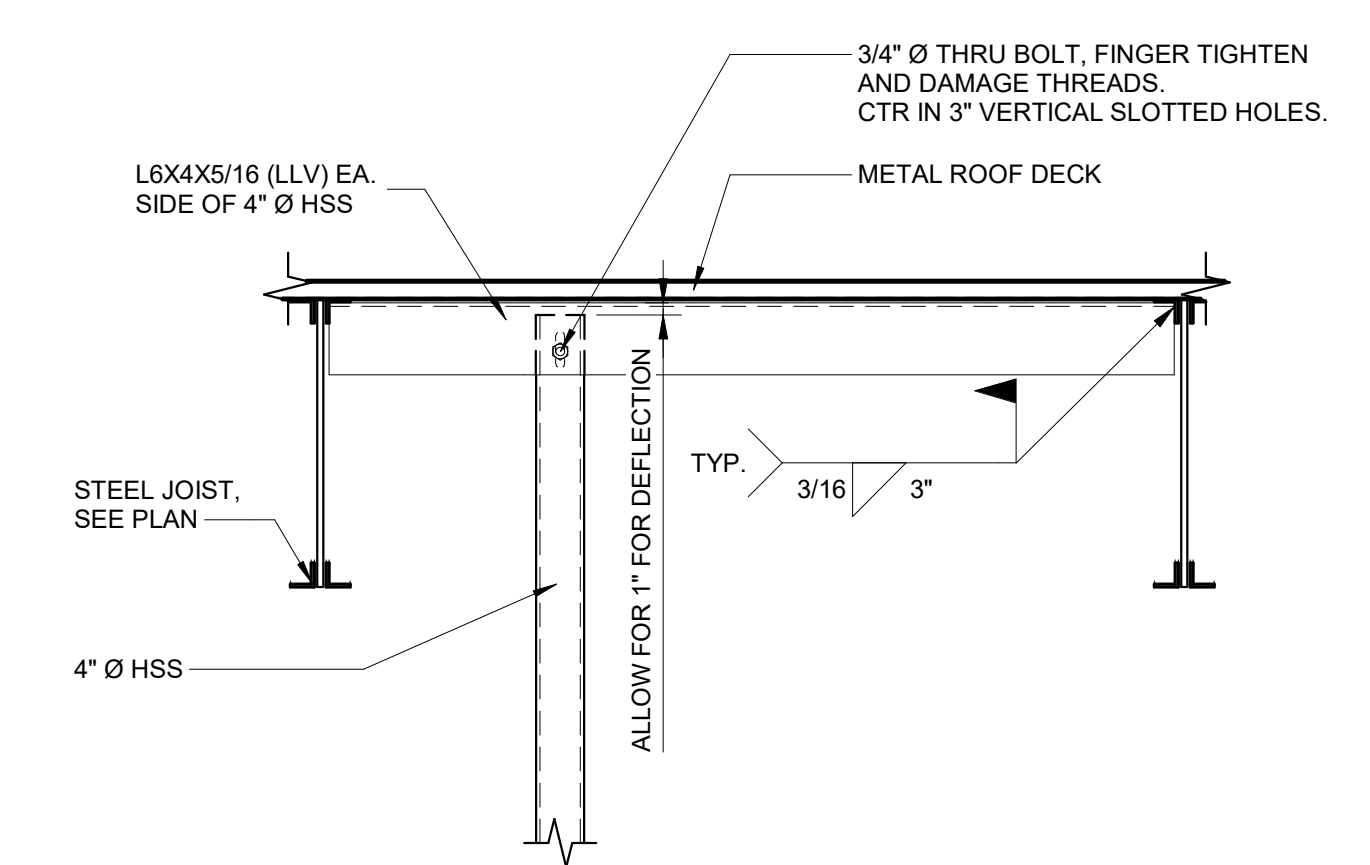
**5 DAYROOM SECURITY - SECTION**  
1 1/2" = 1'-0"



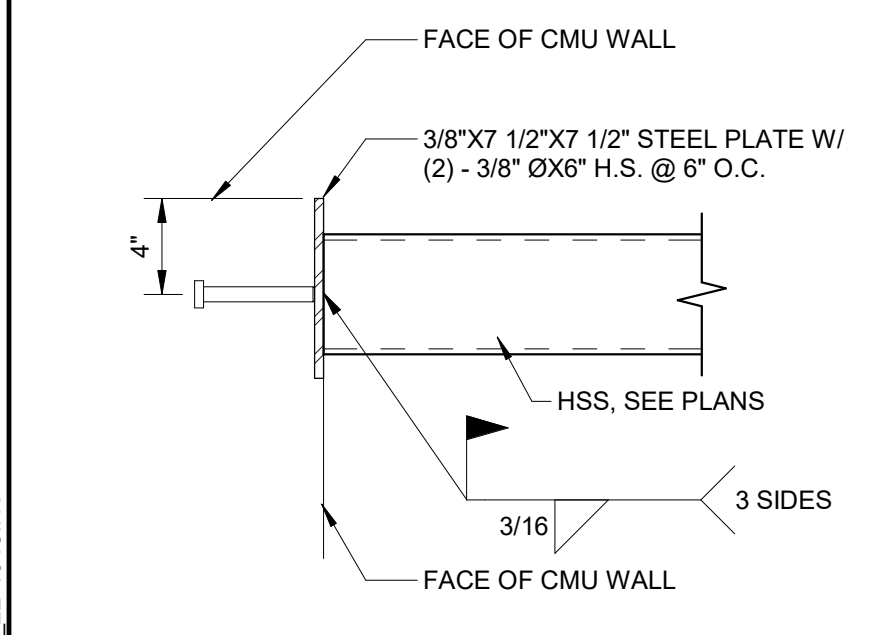
**6 CONTROL ROOM MEZZ SECTION**  
1" = 1'-0"



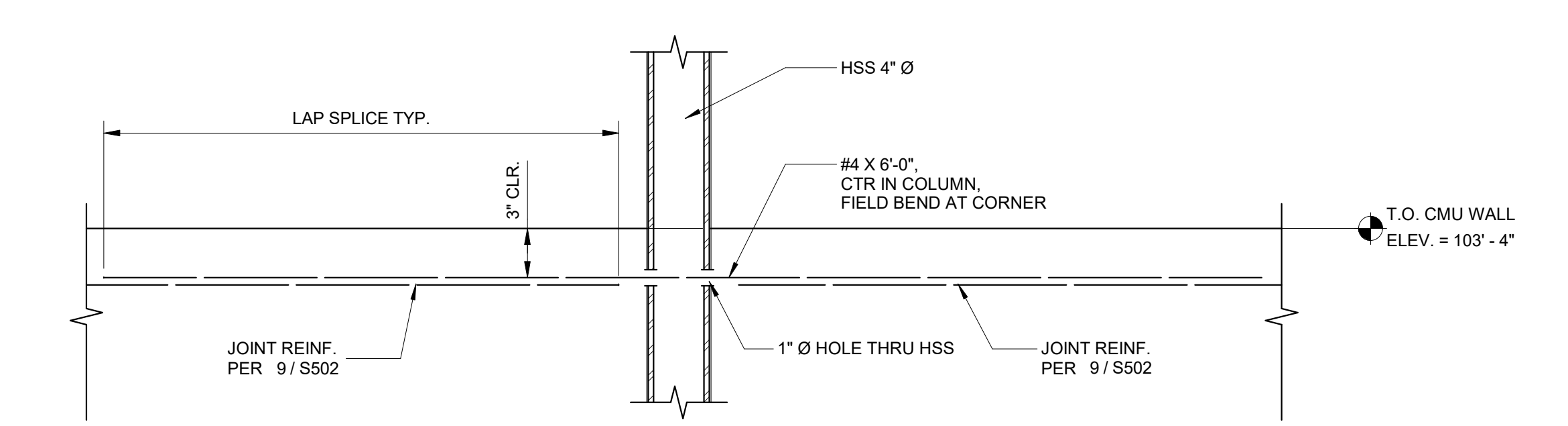
**7 HSS4"X5/16 COLUMN**  
3" = 1'-0"



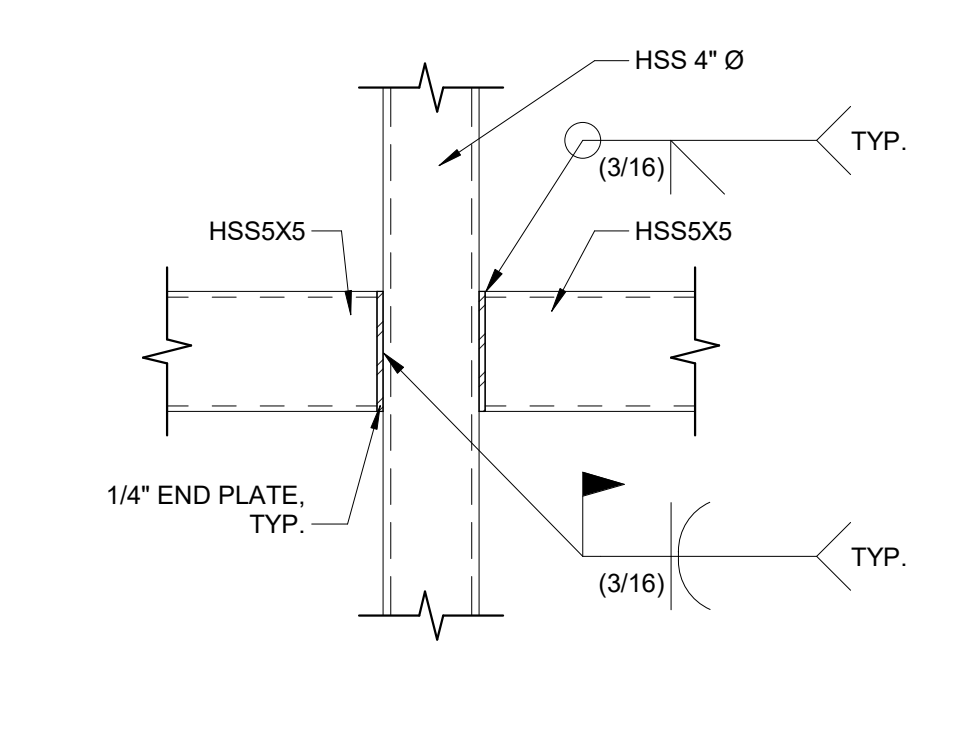
**8 TOP CONNECTION HSS4"Ø**  
3/4" = 1'-0"



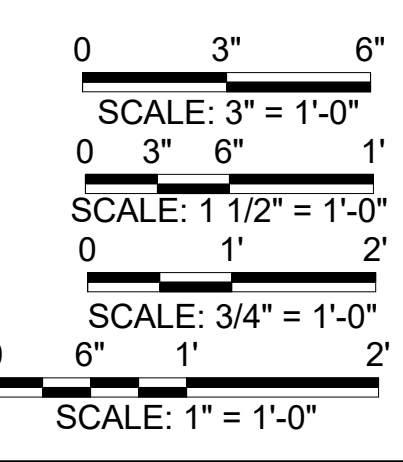
**9 HSS CONNECTION DETAIL**  
1 1/2" = 1'-0"

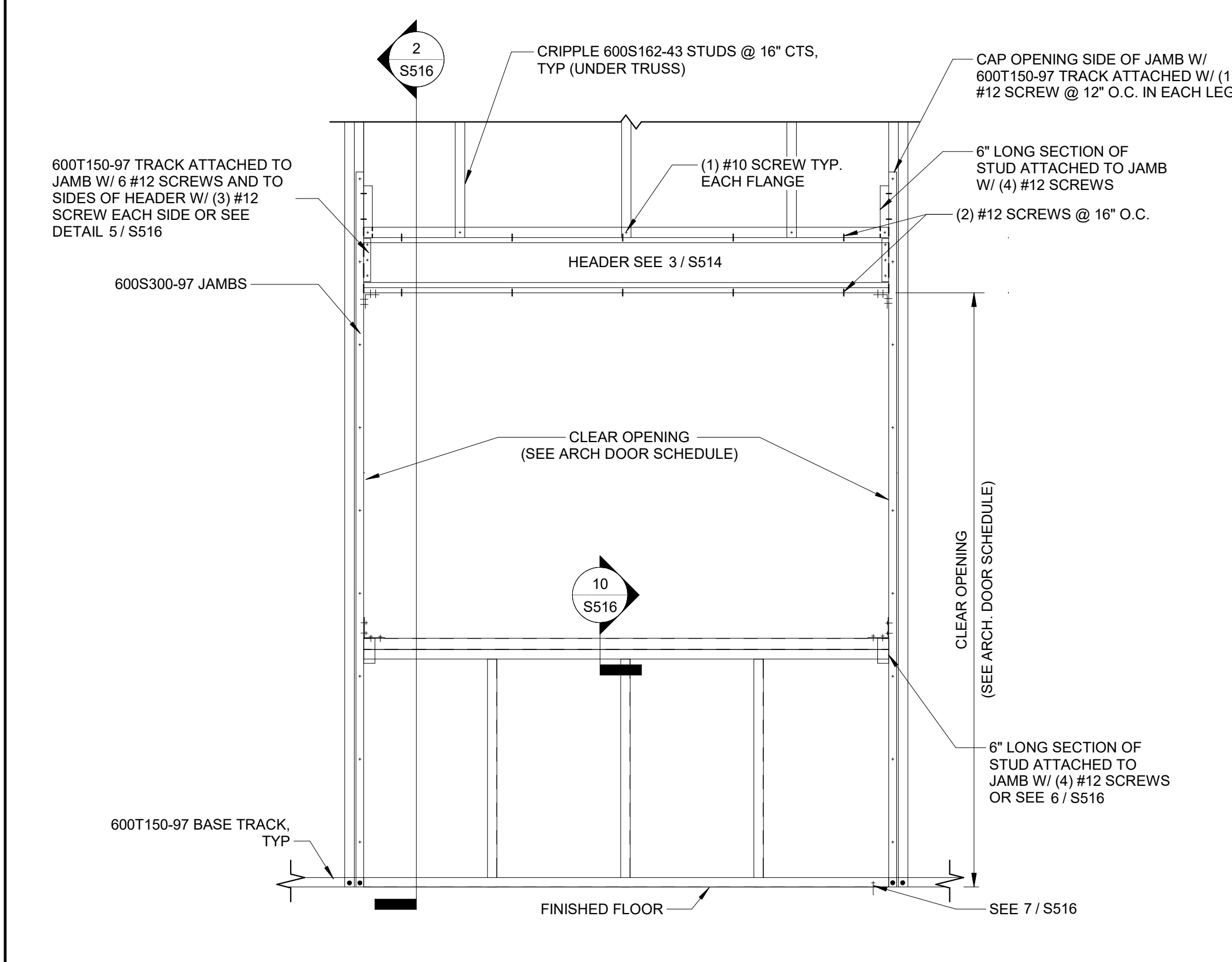


**10 HSS 4" Ø AT KNOCKOUT BOND BEAM SILL**  
1 1/2" = 1'-0"

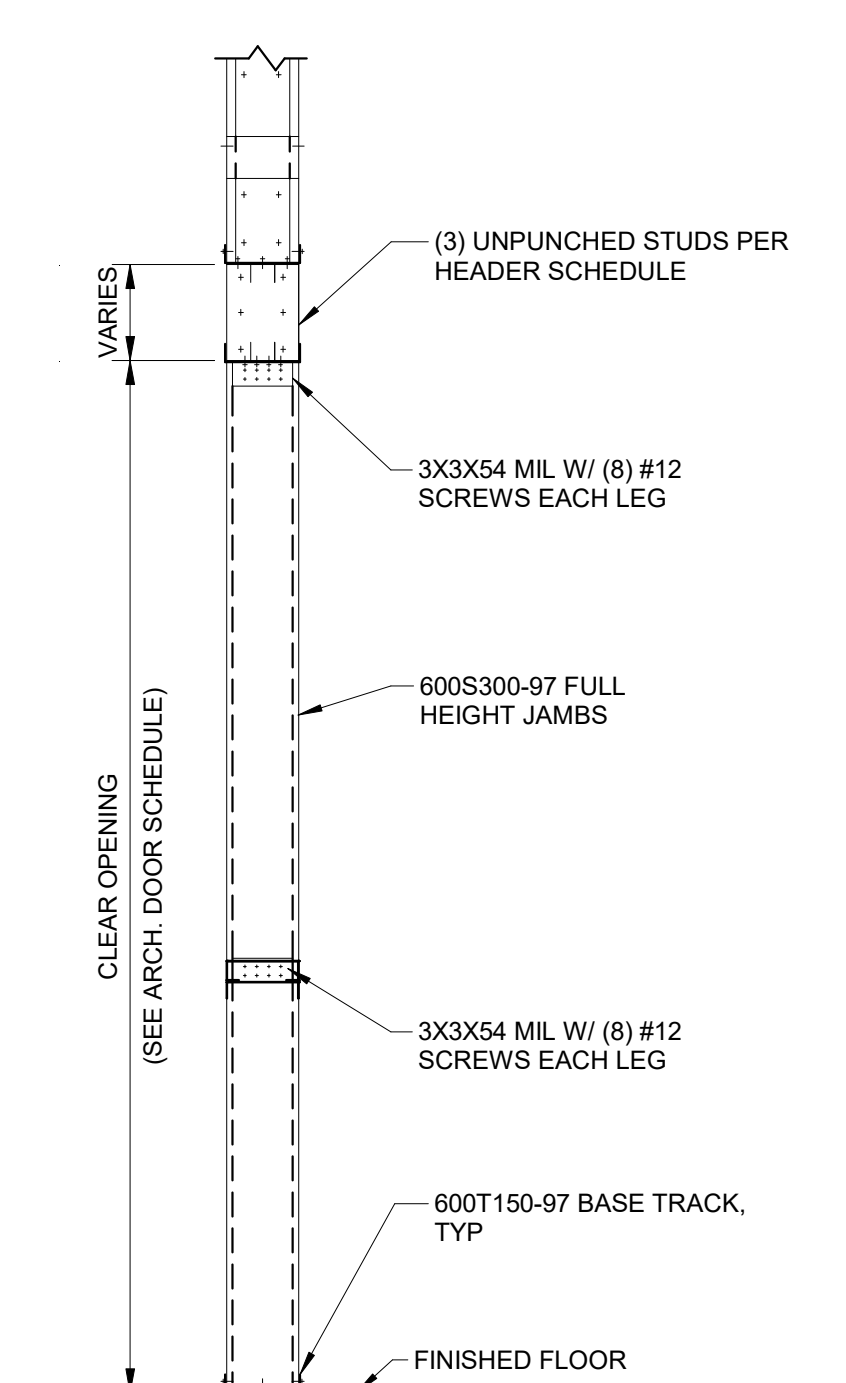


**11 HSS TO HSS CONNECTION**  
1 1/2" = 1'-0"

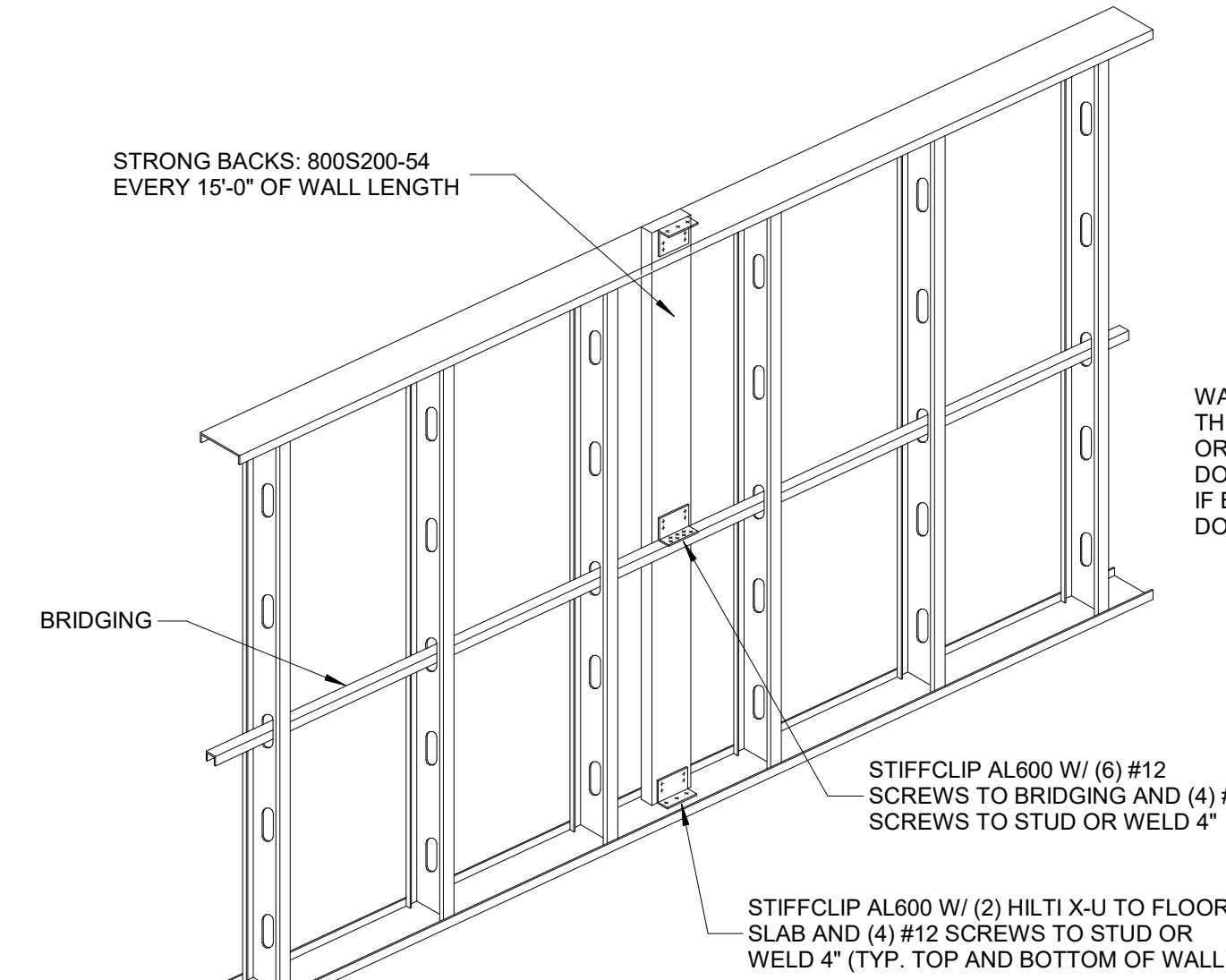




**1** HEADER DETAIL  
3/4" = 1'-0"

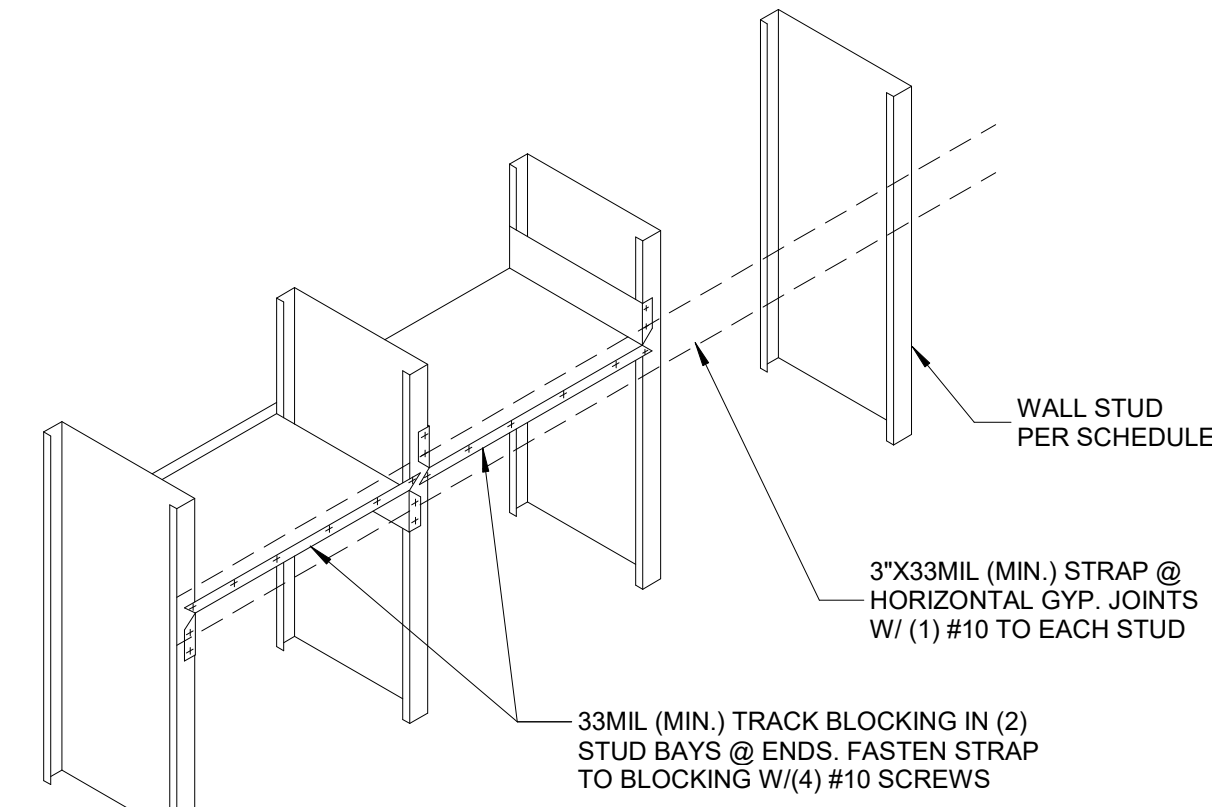


**2** HEADER DETAIL  
3/4" = 1'-0"

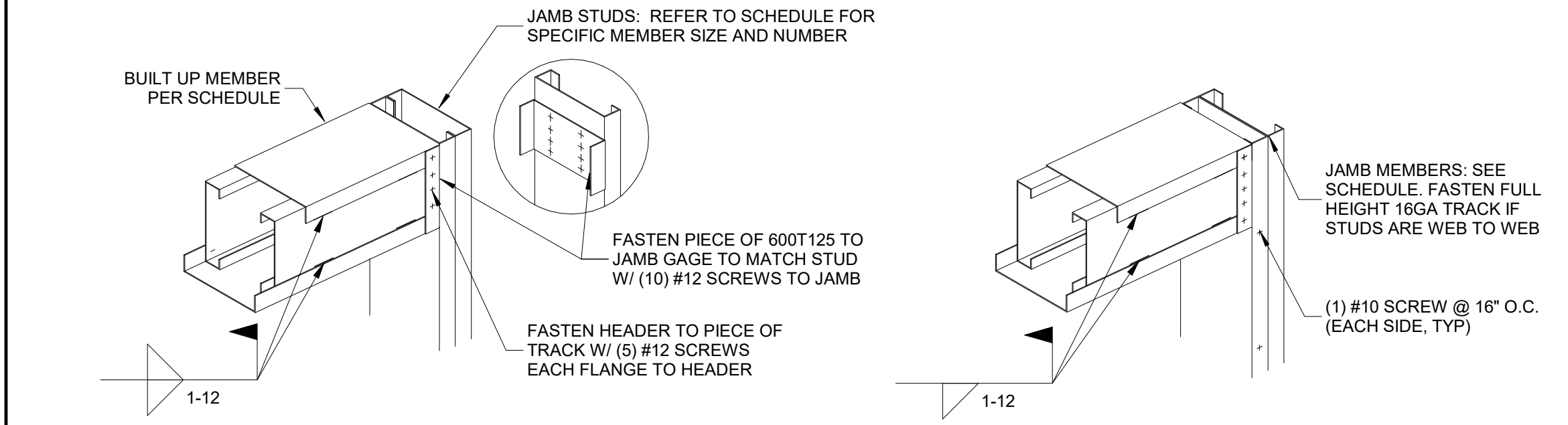


**3** BRIDGING STRONG BACK DETAIL  
1/2" = 1'-0"

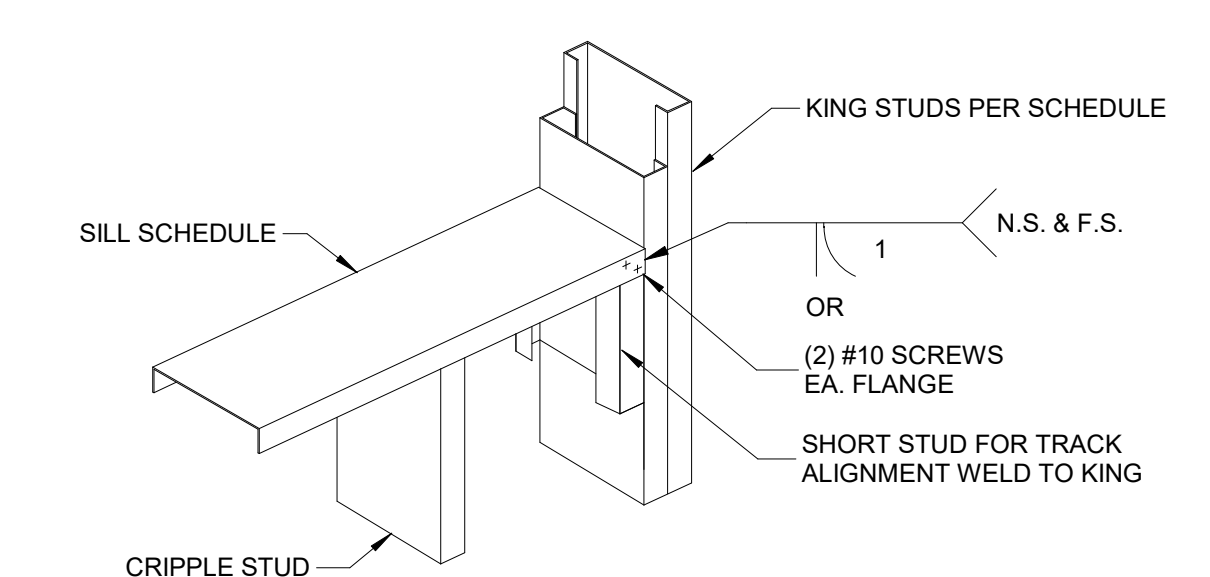
WALLS THAT ARE SHEATHED IN THE FACTORY, OR THERE IS 8'-0" OR LESS BETWEEN KING STUDS DO NOT REQUIRE STRONG BACKS. IF BRIDGING IS ANCHORED INTO DOUBLE STUDS AT EACH END



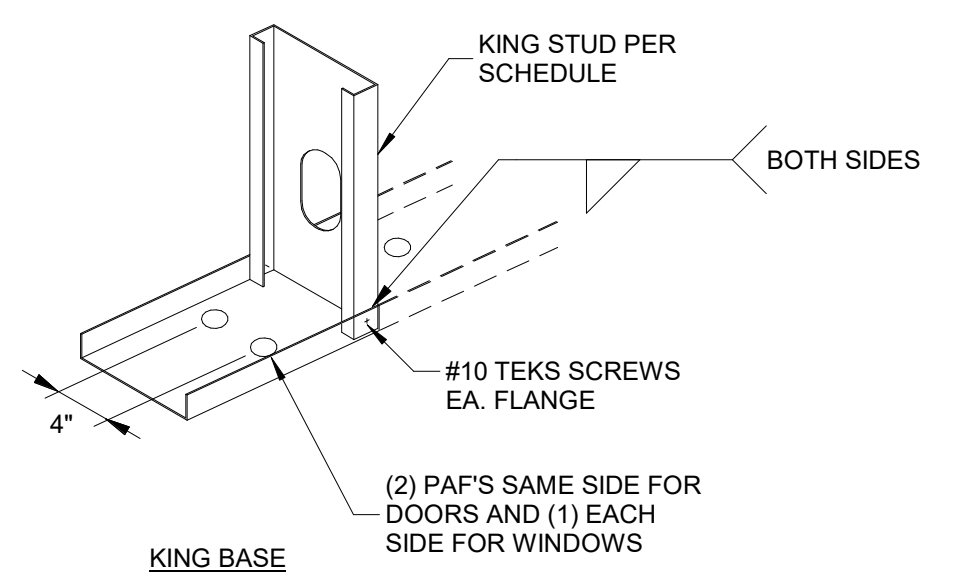
**4** SOLID METAL BLOCKING DETAIL  
1" = 1'-0"



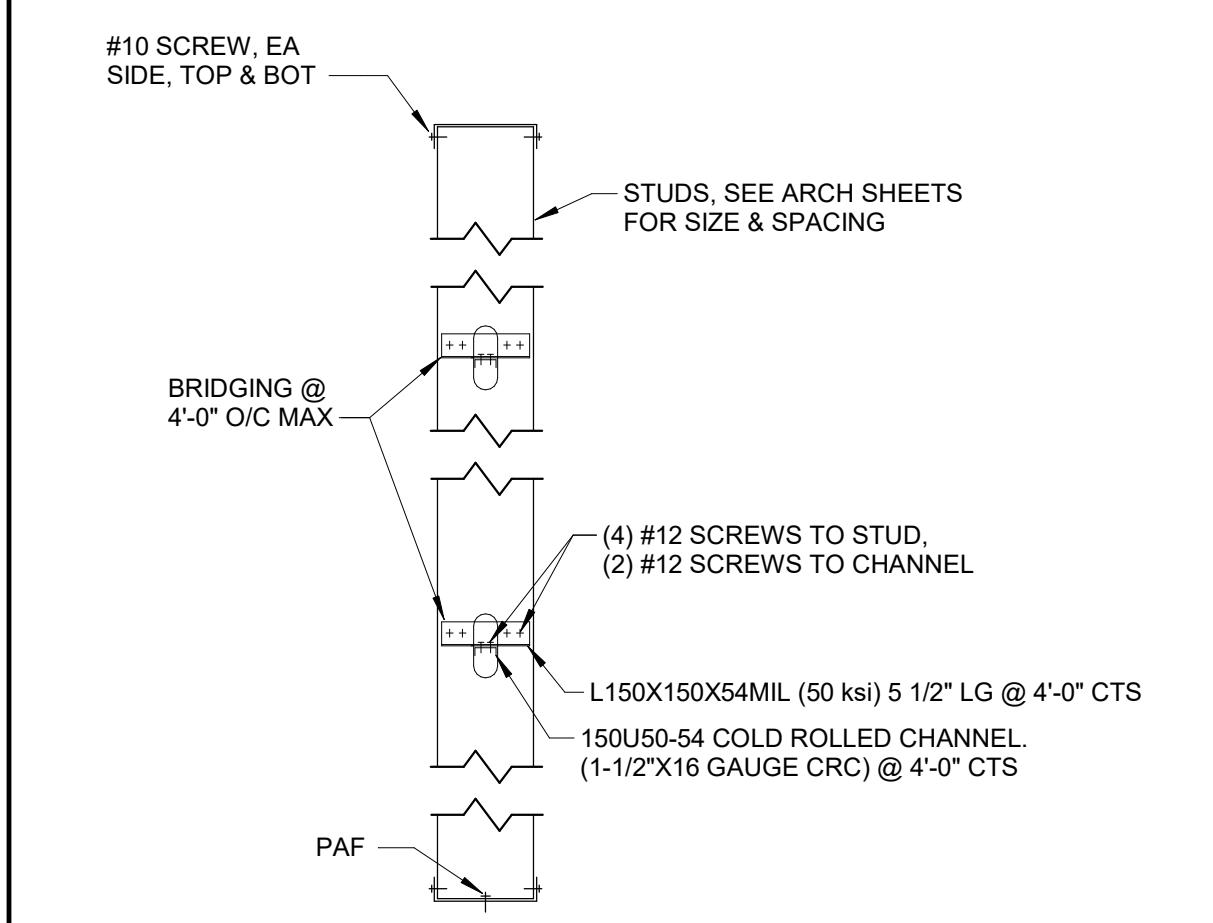
**5** LOOSE HEADER DETAIL  
1" = 1'-0"



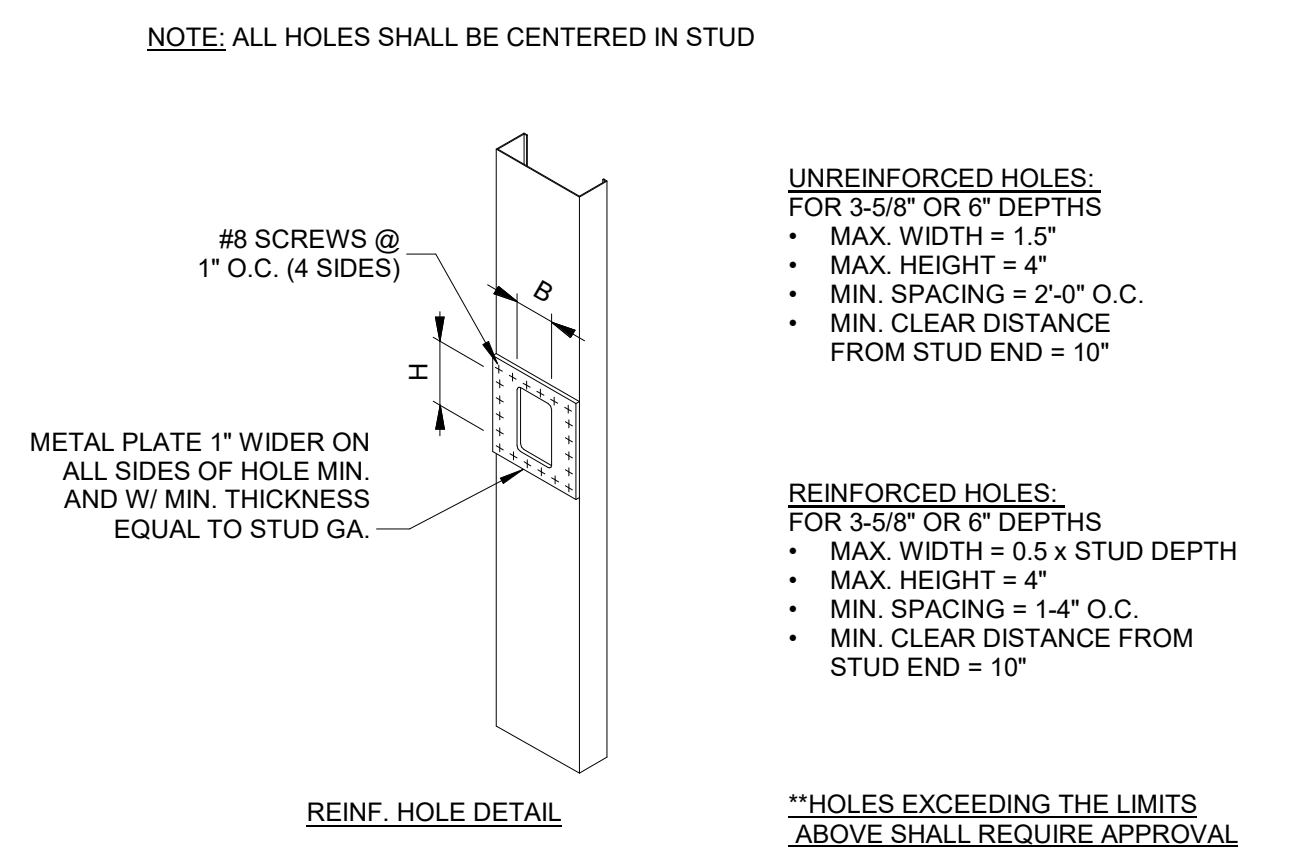
**6** HEADER/SILL/KING CONNECTION  
1" = 1'-0"



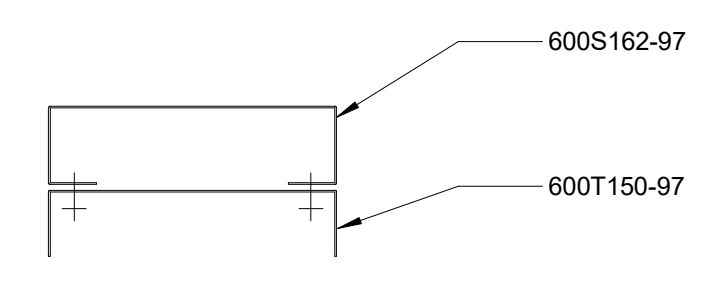
**7** BASE CONNECTION DETAIL  
1" = 1'-0"



**8** BRIDGING DETAIL  
1" = 1'-0"



**9** STUD WEB REINFORCING DETAIL  
1" = 1'-0"



**10** SILL DETAIL  
3" = 1'-0"

